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A MANUAL FOR NORTHERN WOODSMEN



A MANUAL

FOR

NORTHERN WOODSMEN

BY

AUSTIN CARY

Recently Assistant Professor of Forestry in Harvard University



REVISED EDITION

CAMBRIDGE
HARVARD UNIVERSITY PRESS
1918

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By AUSTIN CARY

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PREFACE

The reception accorded this book since it was first issued in 1909, particularly the appreciation expressed by numerous woodsmen, has been gratifying. Letters of commendation have been received from users in all parts of the country. It is significant that the first typographical error discovered (a wrong figure in a logarithmic table) was pointed out by a ranger on the largest tract of unsurveyed timber land in the United States, in Idaho. The second correction was sent in by a Canadian cruiser.

The incidents just mentioned illustrate the wide distribution of the volume and explain the present extension of it. As originally written, the book did not aim at circulation west of the Lake states; but from the first a large part of the demand for it came from Westerners, chiefly those employed in the United States Forest Service. Revisions have been guided largely by this fact, and that is true especially of the present and first considerable revision, for aside from bringing the work up to date as concerns appliances and methods which have come into use since the first edition was written, the new matter and tables which have been introduced are mainly intended for the benefit of western woodsmen. As a result, material additions have been made under the heads Topographic Maps and Timber Estimating.

The book, however, is not materially increased in bulk, nor has there been any change in its chief purpose, which is to serve the men who are carrying the load of actual timber work in this country. To these men, in whatever section they are, and whatever may have been their training, the author extends greeting.

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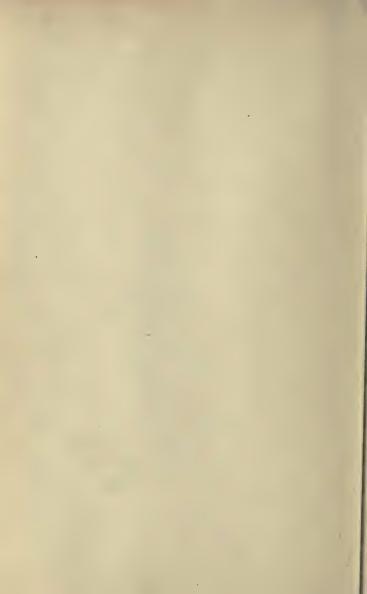
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A MANUAL

FOR NORTHERN WOODSMEN

PART I. LAND SURVEYING

Surveying in forest land as compared with work done in towns and on farms is carried out under unfavorable circumstances. In the first place, timber and brush growth offer an obstruction to sighting; second, the work is often done far from a well supplied base; third, the limits of cost allowed are often the lowest practicable. These conditions have a strong effect upon the methods employed, and they also affect the choice of outfit. Equipment for such work should not usually be expensive, it should be as compact and portable as possible, and it should not be so delicate or so complicated as to be likely to get seriously out of order and so hold up a job.

SECTION I

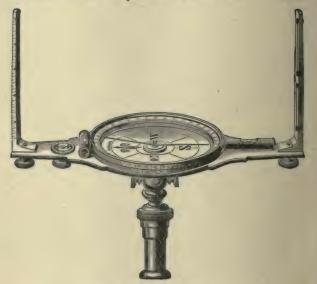
THE SURVEYOR'S COMPASS

Compass and Chain are the instruments that at present are most largely employed in forest land surveying, and there is little doubt that they will continue to be so employed. The compass is one of the mainstays of the practical woodsman. He should thoroughly understand its capacities and limitations, and should have perfect command of all parts of his own particular instrument.

1. THE INSTRUMENT

The essential parts of the surveyor's compass are a magnetic needle for finding a meridian line, a horizontal graduated circle for laying off angles from this meridian, and sights attached for use in prolonging lines on the ground.

The needle in compasses used for surveying purposes is commonly between four and six inches in length. It rests by a jeweled bearing at its center upon a steel pivot screwed into the compass plate, and, turning freely in the horizontal plane, its ends traverse the graduated circle. The plane of the sights passes through the center of the circle, and cuts its circumference at two points marked N and S, known as the north and south points of the instrument. From these points the graduation of the circle runs 90° in each direction to the points marked E and W. These



PLAIN SURVEYOR'S COMPASS

points on the face of the surveyor's compass are reversed from their natural position for convenience in reading bearings.

In using the compass, point the north end of the circle forward along the line and read from the north end of the needle.

A compass bearing is the direction from the observer at

the compass to any given object referred to the meridian. It is read as so many degrees from the N or S direction, up to 90°; as, N 10° W, S 88° 15′ E. The graduations on a surveyor's compass are commonly in half degrees, but it is usual, if necessary, to set by estimation quarter degree, or 15′, courses. A bearing can be set, however, with a surveyor's compass in first-class order, to about 5′.

A compass needle that is in good working order takes some little time to settle, and its condition may be told by the freedom and activity with which it moves. Time can be saved in setting it by checking its motion with the lifting screw. In its final settlement, however, it must be left free. For important bearings, it is well to

let it settle two or more times independently.

A glass plate covers the compass box and two small levels placed at right angles to each other are used to set the instrument in the horizontal plane. It is very desirable that the box of a compass employed for woods work should be as nearly watertight as possible. In general make-up, the instrument is subject to considerable variation.

The plate of the Plain Surveyor's Compass is prolonged in the north and south direction into arms on which the sights are supported at a distance of twelve to sixteen inches apart. The actual sighting is done through fine vertical slits, and round apertures placed at intervals along these are convenient for finding objects and for getting the instrument approximately in line.

The Vernier Compass has the circle and the sights upon separate plates which may be turned on one another for 20° or more. Its advantage consists in the fact that declination, or a change in declination, may be set off, and the courses of an old survey set directly, or lines referred to the true rather than the magnetic meridian.

The Folding-Sight Compass possesses the advantages of light weight and the utmost compactness, and is therefore popular among woodsmen. The sights are set upon the edge of the compass box, and fold down across its face when not in use, the whole instrument with its mountings slipping into a leather case which may readily

be carried in the pack or slung from the shoulder. A folding-sight compass with too small a box and needle of less than full length should not be employed on work of importance, as it is impossible with such an instrument to read bearings and set marks with accuracy.

Compasses are either mounted on a tripod or fitted for attachment to a single staff called a Jacob-staff, which the surveyor may make for himself, when needed, from a straight sapling. The former is the firmer mounting and better adapted to accurate work, but the latter is much more portable, except on bare rocks is more quickly set up, and is generally employed for the ordinary work of the forest surveyor.

2. Adjustments of the Compass

A compass in first-class order will meet the following

- a. The plate must be perpendicular to the axis of the socket.
- b. The plane of the level bubbles must be perpendicular to the same axis.
- e. The point of the pivot must be in the center of the graduated circle.

d. The needle must be straight.

e. The sights must be perpendicular to the plane of the bubbles.

In these tests it is presupposed that the circle is accurately graduated and that the plane of the sights passes through the zero marks. These are matters that belong to the maker of instruments, and in all modern compasses accurate adjustment of them may be assumed.

The general principle of almost all instrumental adjustments is the Principle of Reversion, whereby the error is doubled and at the same time made more apparent. Thorough mastery of this principle will generally enable one to think out the proper method of adjusting all parts of any surveying instrument. In the case of the compass the above-named tests may be applied and the instrument adjusted as follows. The order of the adjustments is essential.

a. The plate is exactly vertical to the spindle in a new compass, but the soft metal of most instruments is liable in use to become bent. If that occurs to any considerable degree, it will be shown by the needle and the bubbles. The instrument should then be sent to the maker for repairs.

b. To make the plane of the level bubbles perpendicular to the axis of the socket, level the instrument, turn it 180°, and, if the bubbles are out, correct one half the movement of each by means of the adjusting-screw at the end of the bubble-case. Now level up again and revolve 180°, when the bubbles should remain in the center. If they do not, adjust for half the movement again and so continue until the bubbles remain in the center of their tubes for all positions of the plate.

c, d. When the pivot is in the center of the circle and the needle is straight, the two ends of the needle will cut the circle exactly 180° apart in whatever position the instrument may be set. If the needle does not so cut, one or both of these conditions is not fulfilled. If the difference between the two end readings is constant for all positions of the needle, then the pivot is in the center of the circle but the needle is bent. If the difference in readings is variable for different parts of the circle, then the pivot is off center and the needle may or may not be straight.

To adjust the pivot, first find the position of the needle which gives the maximum difference of end readings; then, using the small brass wrench commonly supplied with the compass, bend the pivot a little below the point at right angles to the direction of the needle until one half the difference in end readings is corrected. Repeat the test and adjust again if necessary. When the needle cuts opposite degrees, or when it fails to do that by a constant quantity in all parts of the circle, the pivot point is in the correct position.

With the above adjustment attended to, straighten the needle. To do this, set the north end of the needle on some graduation mark and bend the needle until the south end cuts the circle exactly 180° from it.

e. To make the sights perpendicular to the plane of the bubbles, level the instrument carefully, hang a plumb

line some feet away, and then look through the sights upon it. If the plumb line appears to traverse the forward slit exactly, that sight is in adjustment. If not, file off the base of the sight until the adjustment does come. Then revolve the compass 180° and test the other sight in the same manner.

3. KEEPING THE COMPASS IN ORDER

Sharpening Pivot. The pivot or center pin of a compass much in use is liable to become dulled so that the needle does not swing freely. To obviate this the needle should always be raised off the pivot when the compass is being carried. A much blunted pivot should be handed over to a jeweller to be turned down in a lathe, but ordinary sharpening can readily be accomplished by the surveyor himself with the aid of a fine whetstone and the small wrench usually supplied with a compass, or a pair of pliers. The pivot should be removed from the compass box and fixed in the end of a small, split stick; the point may then be sharpened by twirling it gently on the stone at an angle of about 30° with its surface. When the point is made so fine and sharp as to be invisible to the eye, it should be smoothed by rubbing it on the surface of a soft, clean piece of leather.

Remagnetizing Needle. Dulness of the needle may be due to the fact that it has lost its magnetism and needs to be recharged. For this purpose a permanent magnet is required. The north end of the needle should be passed several times along that pole of the magnet which attracts it, and the south end passed similarly over the opposite pole. The passes should be made from center to end of the needle, and a circle described in bringing the two ends successively into contact. In order to prevent the loss of magnetism, the needle of a compass not in use for a considerable time should lie in the north and south direction.

Balancing Needle. The needle is commonly balanced on the pivot by a fine brass wire wound around the south end. If change of latitude is made, the balance will be destroyed, and the wire may be shifted to make adjustment.

Replacing Glass. In case of emergency, a piece of win-

dow glass may be cut down with a diamond and ground on a grindstone to fit its setting. It may then be set in place, with putty if possible, and the binding ring sprung into place over it.

SECTION II

THE MAGNETIC NEEDLE

All compass surveying is based on the tendency of the magnetic needle to point north and south. The direction of the needle, however, is very far from being constant.

Secular Change. There is a belt of country crossing the United States in a general north and south direction through the states of Michigan, Ohio, and South Carolina along which the needle at the present time points due north toward the earth's pole. This belt is called the agonic line, or line of no variation. East of this line the needle points westward of true north; west of this line it points to the eastward of it. The direction from any place toward the pole of the earth's revolution is for that place the true meridian. The direction taken by the needle is the magnetic meridian. The angle between the two is called the declination of the needle, west if the needle points west of true north, east if the needle points east of it. The declination is greater the farther the agonic line is departed from, amounting to more than 20° in the maritime provinces and the Puget Sound country. The agonic line is not stationary but is moving slowly westward, as it seems to have done constantly since the beginning of the last century. The declination of the needle, therefore, is changing from year to year and at a different rate in different parts of the country.

These facts affect the work of the land surveyor importantly, and sections on the bearing of lines and on ascertaining the true meridian are given later on in this volume.

Daily Change. The needle when free and undisturbed swings back and forth each day through an arc amounting commonly in the United States to about 10'. Early in the morning, from four to six o'clock according to the season,

the north end of the needle begins to swing to the east, reaching its maximum position between eight and ten o'clock in the forenoon. It then swings west to a maximum westerly position reached from one to two o'clock P. M. Then it swings slowly east again to a mean position reached between six and eight P. M., at which point it remains practically steady during the night.

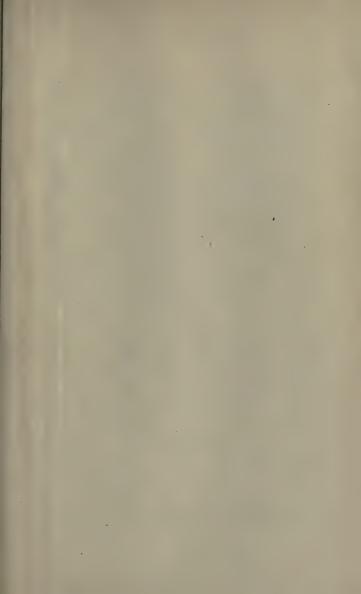
The effect of this variation is such that if a surveyor starts a line in the morning and runs one course all day, he runs, not a straight line, but a long curve. This variation, however, like the slight variation that occurs during the course of the year, is in woods work commonly disregarded.

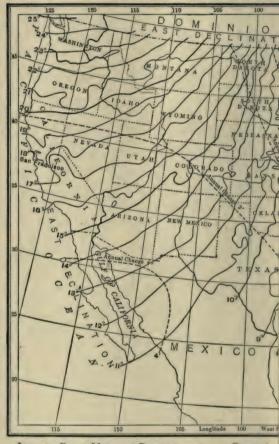
Irregular Changes. The needle is subject occasionally to sudden and irregular changes in direction. They sometimes occur during thunder storms, and at other times are attributed to so-called magnetic storms, related perhaps to the *aurora borealis*. Trouble from this source is not often experienced by the surveyor, but it is a matter which needs to be understood and watched for.

Local Attractions. All users of the compass are on guard against the disturbance caused by iron in its vicinity, in the form, for instance, of chains, axes, and steel rails. In addition, there are in most countries regions of greater or less extent where the needle is subject to irregularities. These are due to iron ore or other magnetic material located in the vicinity, or to unknown causes.

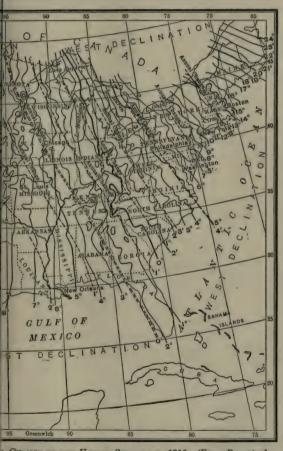
A local disturbance is indicated when the compass does not read the same on the two ends of a line, and in compass running error from this source is guarded against by keeping careful watch of the backsight. Local disturbances vary much in intensity. When very strong, they are readily detected, and if confined in area present little difficulty to the surveyor, who will clear out his line across them with especial care, and either picket 1 through or set the compass by backsight. Slight disturbances are harder to detect. If the area of disturbance is large, particularly if the ground is broken, the compass cannot be depended on to carry a line through with accuracy, and a transit or solar instrument must be used.

¹ See page 21.

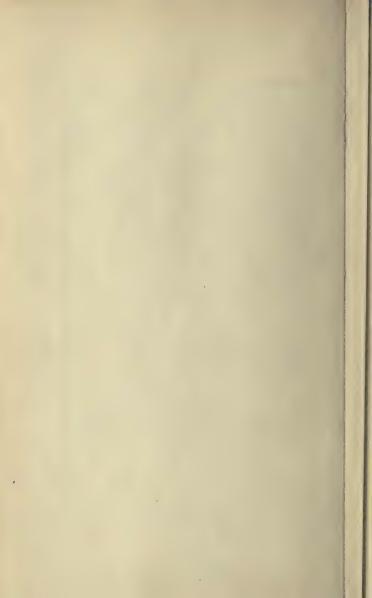




LINES OF EQUAL MAGNETIC DECLINATION AND OF EQUAL ANI United States Coas



t Change in the United States for 1915. (From Report of d Geodetic Survey.)



Electricity. A little caution is necessary in handling the compass in order that the glass cover shall not be electrified by the friction of cloth or the hand, so as to attract the needle to its under surface. If, however, the glass does become electric, the trouble may be removed by breathing upon it, or by touching different parts of its surface with the moistened finger.

Difference in Instruments. It is a well-known fact that different instruments do not always give the same bearing when read on the same marks at the same time. A differ-

ence of 15' is not uncommon.

Summary. The magnetic needle is thus seen to be subject to numerous variations and irregularities, and on that account work with the needle compass cannot be expected to give the most accurate results. The instrument has great advantages, however, and a very large field of legitimate use. It gives an approximately true direction from a detached point. Except on open ground, it furnishes the quickest and cheapest means of turning an angle or prolonging a line. Most authoritative land surveys have been made with the needle compass and their renewal is best accomplished by use of the same instrument. The special advantages of the compass in forest conditions and its most effective use therein are discussed under the head of Surveying Practice.

SECTION III

MEASUREMENT OF DISTANCE

1. THE SURVEYOR'S CHAIN

The word "chain" in connection with land surveying is used to represent two things: a distance of 4 rods or 66 feet, and an instrument for measuring distance. The chain in use for general land surveying is 66 feet long and divided into 100 links, but woodsmen working in rough ground find the 33 foot or half chain with 50 links much more convenient.

A chain for surveying purposes should be made of steel wire, and its links should be brazed to prevent stretching by opening of the joints. Chains have every tenth link marked by a brass tag, and these tags have one, two, three, etc., teeth, so that the number of links may be readily and

accurately counted.

Chains change in length by use. The links may be bent and the chain thus shortened, a matter which can readily be adjusted by hammering; but more commonly a chain increases in length from flattening of the links and wear in the numerous joints. This may be corrected to a limited extent by turning up the nuts which hold the handles. Further effect may be had by taking out one or more of the rings which connect the links, or better still, by hammering each link while it is held in a vise, and so distributing the correction.

The chain is so liable to change in length that provision should be made for testing it frequently. An unused tape, known to be of true length, kept at home or only taken off on long jobs, is the best and most convenient safeguard.

2. THE TAPE

Steel tapes are in wide use for general surveying, but not usually among woodsmen because of their liability to breakage. They have, however, distinct advantages. They are light, so as to be leveled readily when measurement is being made on a slope. They do not stretch. There are no links to get kinked and so cause a false measure. A tape for field use should be made of steel ribbon from \(\frac{1}{8} \) to \(\frac{1}{4} \) inch wide and No. 30 to 32 thick. Wider and thinner tapes are a nuisance in woods conditions.

Tapes are made of any length and graduated to suit the work for which they are designed. One 66 or 33 feet long, graduated to links, will best suit the needs of the timber land surveyor.

Some precaution must be taken with steel tapes. When in use, they should be kept out at full length and never be doubled on themselves, for, if doubled, they are easily kinked and broken. When done up, they should be wiped clean and dry, and so cared for as to prevent rusting. A

broken tape can generally be repaired on the ground if there are at hand a punch, a piece of another tape, and some pins to serve as rivets.

3. MARKING PINS

Woodsmen frequently manufacture their own marking pins of wood or wire. Those bought from dealers are made of heavy iron wire, are some fifteen inches in length, with one end sharpened and a ring turned in the other for convenience in handling. Strips of cloth are tied in the rings, so that they can be readily seen. It is most convenient to use eleven pins in chaining. One of them is stuck at the starting point, the leading man takes ten, and thus there is always one in the ground to start from when the tallies are finished.

4. CHAINING PRACTICE

Chains are standardized in length at about ten pounds pull with their full length supported. In woods work it is generally necessary that the chain should be suspended above the ground and not lie upon its surface. Care must be taken, therefore, in accurate measurement, to give it proper tension. What tension is proper for a suspended chain,—in other words, what sag should be allowed to compensate for the stretch of the chain under the greater tension—may be determined on perfectly smooth and level ground, and this is a valuable exercise for inexperienced chainmen.

In order to get true chainage between points, the chain should be kept straight and free from kinks. It must also be kept in approximately true alignment, though a constant error of 1° in that matter, equivalent to seven inches error in setting pins each two rods of distance, shortens the line by only nine and a half inches in the mile. Similarly, the chain must be levelled so as to give distance in a horizontal line, not following the contour of the ground. In this last connection, that is, in getting distance correctly on slopes and over rough ground, are met the greatest difficulties in practical chaining. What is necessary is first, to determine when the chain is level, and second, to

carry the point occupied by the suspended end of the chain vertically down to or up from the mark on the ground.

The use of plumb lines and plumbing rods for this purpose is well known from standard works on surveying. It is common woods practice to drop a pin from the head end of the chain, and that practice, when a pin loaded near the lower end is used, has been approved for United States land surveys. Only one such pin is required in a set, as after it is stuck in the ground another may be substituted for it. Similarly, for the rear end of the chain, when it has to be held above the ground, an ax held suspended beneath the handle, with the bit turned across the line, enables one to do quick and fairly accurate plumbing. For determining when the chain is level, a hand level or Abney clinometer, such as is shown on page 93, may well be put in the hands of the men. There is a strong tendency on the part of unpracticed chainmen to hold the down-hill end of the chain too low.

It is to be observed that all the above-mentioned sources of error work in one direction, namely, to give too large a valuation to the distance between two points. The young, school-trained man particularly, with his aspiration after exactness, is apt to undervalue these sources of error, and,

in consequence, not give land enough.

In view of all the facts and conditions, particularly because of the pressure for cheapness in this class of work, many practical woods surveyors have concluded that it is best and safest not to strive after too great mechanical exactness, but to make a small constant allowance at the rear end of the chain. On the other hand, the loose practices of some old woodsmen, such as letting the chain run out the length of a man's arm beyond the mark, have nothing to be said in their defense.

The general method of procedure in chaining, to be modified as circumstances may require, is as follows. The two chainmen will be spoken of as head and rear man. Commonly, the rear man is the better and more experienced of the two, and is in general charge.

With one pin set at the starting point, the head man takes his end of the chain or tape and ten pins and steps off in the direction of the line to be measured. Just before the chain is all drawn out the rear man calls out "chain" or "halt," and prepares to hold his end of the chain on the mark. The rear man lines in the other, by the compass ahead, by stakes left, or by the marks and bushing

TABLE SHOWING ERROR CAUSED BY CHAINING ALONG GROUND OF DIFFERENT DEGREES OF SLOPE

SI	ope.	Error.		
In feet per 100.	In degrees.	In feet per mile.	In links per chain.	
2	11	1.0	.02	
種	21/3	4.3	.1	
Б	38	9.5	.2	
В	41/2	16.7	.3	
9	51	21.2	.4	
10	53	26.1	.5	

along the line. Kinks are shaken out, the chain is levelled, and proper tension is applied. When all is ready and the rear man has his handle firmly held on the mark, he calls out "stick" to the leader who sets his pin at once and calls "stuck." When the rear man hears this signal, and not before, he pulls his pin and both men move quickly forward, repeating the operation till the head man has stuck his last pin or has reached the end of the line. When the head man has stuck his last pin he calls "tally." The rear man then drops his end of the chain, counts the pins to make sure that none has been lost, and, going forward, gives them to the head man who counts them again. The tally is marked down and a stake left at the point for reference in case of a lost pin or other cause of debate in the next tally. Pins should be set plumb, and, in general surveying practice, the point held to is the point at which they enter the ground. In the brush and "down stuff" of some woods lines, however, it is sometimes necessary to chain by the top, not the bottom, of the pins. No jerking of the chain should be allowed. The rear man should not stop the head man with a jerk. The head man

must pull steadily on the chain when measuring.

When chaining on slopes which are so steep that the full length of the chain cannot be levelled at once, the head man first draws the chain forward the whole length and in line. He then drops the chain and his marking pins and returns to a point where he can level a part of the chain. This distance is measured and one of the rear man's pins stuck at the point. The rear man then comes forward and, taking the chain at the same point, holds it to the mark while a second section is measured, and so on till the end of the chain is reached, when the head man sticks one of his own pins. It is not usually necessary to note the lengths of the parts of the chain measured. Take care only to measure to and from the same points in the chain and not to lose the count by getting the marking-pins of the two men mixed together.

Accuracy. The requirements of woods chainage vary so widely, its difficulties are sometimes so great, and the expense permissible for the work is often so restricted that only guarded statements can be made as to obtainable accuracy. When chainmen, measuring the same line twice, agree almost exactly, it does not prove that they have given correct chainage, for two other men on the same line may get a result considerably variant. Really correct chainage is to be obtained only by strict attention to the sources of error mentioned above, their amount and nature. In general, it may be said that on smooth and level ground, free from obstructions, chaining may be done with error of a very few feet in the mile. On land as it runs, however, chainage accurate to within a rod in a mile is generally called entirely satisfactory.

Summary. Good chaining consists in keeping the chain of right length, in true alignment, vertical and horizontal, and in proper stretching, marking, and scoring. It is a very important part of all surveying which employs that method of measuring distance, and has been badly neglected in much woods work of the past. It needs and de-

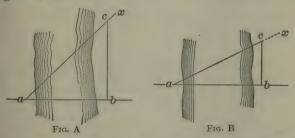
serves good men to carry it on, men who will get down to the ground and take all needed pains in marking, leveling, and alignment. They should be brisk men, moving quickly and doing their work in a prompt and business-like manner. Much, too, depends on system, — on tallying, passing pins, etc., from habit and in regular order. Some men never will make good chainmen because they will not take sufficient pains about details. A few in their strict attention to these are liable to make gross blunders. The man in general charge of surveying work must give careful attention to this part of the business. Chainmen must be trained in good methods and watched till they are perfectly trustworthy, while careful consideration must be given to sources of error and to possible improvements in method.

5. Measuring Inaccessible Lines

Ponds, bogs, and bluffs, over which it is impossible to chain, are met in the practice of nearly every surveyor, and quick and accurate measurement across them constitutes one of the problems which he has frequently to solve. Each problem of that kind has to be solved in the field according to the ground and circumstances. The methods commonly employed in such cases are as follows:

1. Offset. Frequently a short offset squarely to left or

right will clear the obstacle.

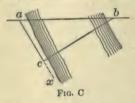


2. Method by 45° Angle. (A) With the compass at a, set a stake in the line at b across the obstruction, and, turning off an angle of 45° , set another stake on that range

as x. Set up at b and, turning off a right angle, set a

stake c in the range ax. Then ab = bc.

3. Method by 26° 34' Angle. (B) Proceed as before, making the angle b a c = 26° 34'; then a b = 2 b c, as may be found in the table of tangents.



4. Method by 30° Angle. (C) With compass at a set a stake in line at b, and, turning off an angle of 60°, set another stake on that range, as x. Set up at b and turn off a b c = 30°, setting a stake c in the range a x. Then a b = 2 a c.

5. Method by Tangents. (D) With the compass at a set a stake at b, also run out a perpendicular line and set a stake at c visible from b at any convenient distance. Measure a c. With the compass at b, take the bearing of c b and thus get the angle a b c. In the table of tangents

look up the tangent of this angle. Then $ab = \frac{ac}{\tan abc}$

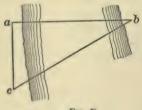


Fig. D

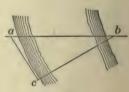


Fig. E

- 6. Method by Oblique Triangle. (E) The stake c may be set at any convenient point visible from both a and b and the angles at a and b measured. Measure also the side a c or b c, whichever is easier. Then a b may be computed as the side of an oblique triangle. For formulas necessary, see pages 212-213.
- 7. Method by Traverse. (F) In the case of a large lake or stream, several courses may be run along its banks, and when the range of the line is again struck, as at e, the dis-

tance a e may be computed by traverse. If a e runs N and S, the distance a e will be the latitude of the traverse, or,

stated in other words, it will be the sum of the products of the cosines of the several courses into their respective distances. The departure of such a traverse should be zero. Thus, if e is not visible from a, or if it is not convenient to take the range a e, e may be set when the sum of the departures figures up 0. This process of surveying a lake or river shore is called "meandering." It is the method pursued in the United States land surveys on considerable bodies of water. The same method may also be employed to get round a precipitous hill or some other inaccessible object.

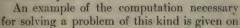




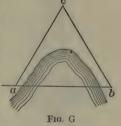
Fig. F

page 33.

8. Method by 60° Angles. (G) A precipitous bluff or impassable swamp may occasionally be passed most read-

ily in the following manner. With the compass at a, lay off a 60° angle and run out a c, carefully chaining. Next, making an angle of 60° at c, run out c b to an equal distance. Then, if the work has been done accurately, b is in the line and a b = a c = b c.

In working by any of these methods it is better, if possible, to set b in range by the compass

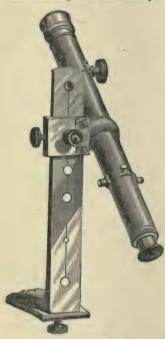


from a rather than to rely for the range on any process of figuring or angulation.

6. STADIA MEASUREMENT

A substitute for chaining, which has to some extent been employed in forest land surveying and which deserves wider use, is stadia measurement, or the measurement of distance by wires placed in the focus of a telescope and the space which they cut off on a graduated rod. The principles of this method are stated on page 77.

For this purpose a light telescope may be fitted to the rear sight of the compass, as shown in the illustra-



A TELESCOPIC SIGHT

tion, a level and vertical circle being added if the instrument is to be used on rough ground. The cost of such an instrument complete is about the same as that of a compass. Its adjustments will readily be understood from its construction and from consideration of the adjustments required for the transit.

The advantages of this instrument in land surveying are as follows: -

1. Sights may be taken on steeper ground, either up or down hill, than can be covered through compass sights.

2. Distances over very steep ground can be measured more accurately and quickly than by use of the chain

3. Distance across

gorges, swamps, and bodies of water can be obtained directly and with ease.

4. It enables the surveyor himself to perform all the particular work on a survey, and this on short jobs, or wherever reliable chainmen cannot be had, may be a very great advantage.

Stadia wires in an instrument used for land surveying

should be so spaced that one foot on the rod will be cut off when it is held at a distance of 66 feet, or, if the wires are fixed, the rod may be graduated to correspond. For occasional use in land surveying, the rod may best be made of painted canvas, which, in case of need, may be tacked on any pole that comes to hand.

The Stadia Hand Level is a simpler form of the instrument, adapted to the measurement of the width of gorges or ponds. It is readily carried in the pack, and, when in use, may be held in the hand or mounted on a staff. The

ready range of this instrument is 200-300 feet.

7. Units of Distance and Area

7.92 inches = 1 link.

25 links = 1 rod.

100 links = 66 feet = 1 chain.

320 rods = 80 chains = 1 mile.

160 square rods = 10 square chains = 1 acre.

640 acres = 1 square mile or section.

The vara, a measure of Spanish origin, prevails in California and in Texas. The California vara is 33 inches. The Texas vara is $33\frac{1}{3}$ inches, and 5645.376 square varas make one acre.

In Louisiana and the Province of Quebec, the *arpent*, an old French unit, is the measure of areas. This is .8449 acre.

The hectare = 10,000 square meters (meter = 39.37 inches) or 2.47 acres. This is also a French measure.

SECTION IV SURVEYING PRACTICE

The starting point of a survey is generally settled for a surveyor by outside controlling circumstances. When this is recognized, the next thing to do may be to find out what course to run by an observation for the true meridian, or by finding the bearing of an old line. With the starting point and course determined, the method of procedure is about as follows.

1. RUNNING A COMPASS LINE

Set up the compass at the point from which the line is to start; level the plate: free the needle, and when it has settled, set the course to be run. It is desirable on starting a line to let the needle settle two or more times independ-

ently.

An assistant, called the rodman or flagman, then goes ahead with a pointed rod or flag, and, following him, go the axemen, clearing out the bushes and other obstructions in such a manner as to secure both a clear line of sight and a path for the chain. The rodman may use an axe. He guides himself at first by the compass sights, later by signals from the compassman or by the range of the line. The axemen guide their work by him.

When the rodman has gone ahead a convenient distance, at signal from the compassman or acting on his own judgment, he selects a spot for a second setting of the compass, attention being paid both to firm setting and clear ground for the instrument, and to facility in getting sight ahead. On uneven ground summits commonly meet best this last

requirement.

When setting the rod, the rodman should face the compass, holding the rod plumb and directly in front of him. He sticks it as directed by the compassman, who assures himself at the time that everything about the instrument is right. Before taking up the compass, the man in charge of it sets a stake near by and in line to be used in backsight. The needle is then lifted, and the compass taken up and carried forward to be set up at the point marked by the rodman. If a Jacob-staff is used instead of a tripod, the compass should be set up ahead of the rod with its center in line, the exact position of the foot of the staff being of no consequence.

The compass is then levelled again with its N mark ahead as before and the sights turned on the object left at the starting point. The needle is then freed, and if, when it settles, the bearing reads the same as before, the surveyor is assured that there is no local disturbance, and may proceed confidently. The rod and axemen soon learn

to range for themselves, and lose no time waiting for the set-up of the instrument. The chainmen keep behind the instrument where they are out of the way. Each man learns his exact duties, and all hands, particularly the com-

passman and rodman, learn to work together.

Running by Backsight. The details of compass surveying vary considerably in accordance with the accuracy required, cost allowed, and the make-up of the party doing the work. If local attraction is suspected or, on short lines, if great accuracy is required, obstructions are cleared completely out of the line, and when an assumed or trial course has been started, it is prolonged by backsight entirely, reference to the needle not necessarily being made. In order to do this, either a rear rodman is employed or a stake is set in line at each station occupied by the compass.

Picketing. The compass after the start, indeed, may not be used at all, but straight stakes, preferably four to five feet high and sharpened at both ends, may be ranged in one after another along the line. This method of running a line is frequently resorted to, and is called picketing.

To clear out in most woods a line open enough for continuous backsighting or picketing is an expensive process, and, further, this method for long distances and uneven ground is not to be relied on. If, in those circumstances, close accuracy of alignment must still be had, resort must be made to another class of instrument, a transit or solar, which may carry the work out of the hands of the woods

surveyor.

Running by the Needle. Usually the compass will do the work reasonably well and satisfactorily to all interested parties, in which case the needle will be used at nearly every setting. In all compass running it is well to carry a light rod ahead, though that is sometimes dispensed with, the compassman going up to a stake or even an axe set up by the head axeman in line. When trees of some size are run into, they are not commonly cut down, but the compassman notes, or has marked, the spot at which his line of sight hits them, and, going forward, sets up beyond them in the same range as nearly as he can. For backsighting it is not a great trouble to set stakes, but, in a

country where local attraction is infrequent it is sufficient precaution to watch the blazes and bushing back along the line. In any case, time is saved by setting up the compass approximately by the backsight before letting the needle go free.

2. TRY-LINES

When two unconnected points are to be joined, it is usual first to run a line without spotting, a try-line so called, and if the desired point is not hit, to measure at right angles the distance between the line run and the point aimed at, figure the angle of error, and rerun the line. The angle required is obtained from a table of tangents.

Thus suppose a try-line to have been run N 4° E 120 rods or 30 chains and to have hit 32 links east of the mark aimed at. Dividing 32 by 3000 (the distance run in links) gives .0107, and the angle of which this is tangent is found in the table of natural tangents to be 37′. The compass may therefore be set N 3° 23′ E, and the line rerun.

Results near enough for most purposes may be had by remembering that the tangent of 1° is .0175 (i. e., $1\frac{3}{4}$ feet in 100, or $1\frac{3}{4}$ links per chain) and that the tangents of small angles are in proportion to the size of the angles. Thus with the case above, the tangent of 1° being .0175 and that of the angle required .0107, .0107 divided by .0175 equals .61 of 1° , or 37'.

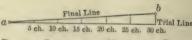


DIAGRAM SHOWING THE METHOD BY OFFSET

Or instead of using the compass to rerun the line, its position may be fixed by offset, that is, by measuring at right angles to the try-line, at different points along it, the distance required to place points in the desired range. For this purpose stakes should be left in the try-line at equal distances apart, say every 5 chains, and the length of each offset may be figured by tangents or as a simple problem in proportion.

Thus with the case in hand. The tangent of the angle between the try-line and the true line has been figured as .0107. This decimal multiplied by five chains or 500 links gives $5\frac{1}{3}$ links, the offset from the 5-chain point. Similarly 10 chains multiplied by .0107 gives 10.7 links, and so on until all the offsets have been computed.

By proportion the problem is even simpler. In the case in hand the offset at the 15-chain mark should evidently be half that at the finish, or 16 links. At the 5-chain mark it is $\frac{1}{6}$ of it, or $5\frac{1}{3}$ links as found before. In the same way offsets for any length of line and any error in closing may be figured. When the points have been put in, the line may be blazed through by eye, or with the aid of the compass.

3. Marking Lines and Corners

Corners. Permanent corner marks are especially valuable in maintaining bounds and protecting property rights; and the desirability of stone monuments, or, failing these, of earth mounds, iron rods, or charcoal, is not to be disputed. Forest land is occasionally subject to great mischances, as from clean cutting, wind, and fire, and marks which can survive these have distinct and peculiar value.

On the other hand, posts of durable wood, and trees that are likely to remain in place a long time are generally handiest, are easy to mark on, and frequently meet, better than more elaborate and expensive marks, the ideas of owners and the customs of the country. Supplemented by blazed and marked witness trees, such markings for corners are now in wide use on forest property and there can be little doubt that their use will continue. Marks on living trees should be placed in most cases on a peeled or blazed surface of the wood, though bark marks, much distorted it is true, have been known to remain legible for a very long time.

Corners in every case should be plainly inscribed so that any interested person may readily identify them. It is usual in woods practice for the surveyor who establishes a corner to leave there his initials, or some mark peculiar to him which will identify it as his work, together with the year in which the survey was made. The same thing may be done by a succeeding surveyor.

Practice in all these matters, however, varies a good deal in different parts of the country. The methods prescribed for use in the United States land surveys will be found on

later pages of this volume.

Lines. A property line in the forests of Germany is kept cleared out several yards wide and blocks of cut stone are deeply set along it near enough together so that one may be seen from another. In addition, the range of a transit line is inscribed upon them. This renders the property limit prominent and durable, and, further, defines it to within a

quarter of an inch.

Such ideal marking is seldom to be looked for in this country, but the ends to be aimed at, which in the foregoing case were attained, should be in the mind of every man who has to do with forest boundaries. A property owner's interests are first, to have his bounds prominent so that he and other parties may know where they are and so that there will be no excuse for trespass; second, to have them durably marked for obvious reasons; and third, to have them so closely defined that all possible causes of dispute may be avoided.

Stone walls, ditches, and fences are the common bounds of property in settled and half-settled countries, and each of these methods of delimitation has its grade of efficiency, considered from the above points of view. In large forest areas blazed trees are the means almost universally employed for the purpose. That system has been reasonably satisfactory in the past. It would have been more so had care and system always been employed in the marking and

more attention paid to renewal.

The directions for marking lines in timbered lands, as contained in the "Manual of Instructions for the Survey of the Public Lands of the United States," are as follows:

All lines on which are to be established the legal corner boundaries will be marked after this method, viz.: Those trees which may be intersected by the line will have two chops or notches cut

on the sides facing the line, without any other marks whatever. These are called sight trees or line trees. A sufficient number of other trees standing within 50 links of the line, on either side of it, will be blazed on two sides diagonally or quartering toward the line, in order to render the line conspicuous, and readily to be traced in either direction, the blazes to be opposite each other, coinciding in direction with the line, where the trees stand very near it, and to approach nearer each other toward the line, the farther the line passes from the blazed trees.

Due care will ever be taken to have the lines so well marked as to be readily followed, and to cut the blazes deep enough to leave recognizable scars as long as the trees stand. This can be attained only by blazing through the bark to the wood. Trees marked less thoroughly will not be considered sufficiently blazed. Where trees two inches or more in diameter occur along a line,

the required blazes will not be omitted.

Lines are also to be marked by cutting away enough of the undergrowth of bushes or other vegetation to facilitate correct

sighting of instruments.

These directions are ample, have been tested by use, and are practically the same as those issued for land survey work in the Dominion of Canada. Plainly, however, they are adapted to sparsely wooded land, for, in real timber growth, blazed trees two rods away from the line would be a source of confusion. In fact, the narrower a line is blazed, so long as it is clear and durable, the better. A good general rule to be applied in timber is to blaze those trees, and only those, which a man can reach with his axe when standing directly in the line.

A line in ordinary woods well blazed according to this method is prominent, and reasonably durable, while the quartering of the spots and special marking of the "line" trees render it reasonably well defined. If decent care is used in maintenance, and if when it has become dim or doubtful it is thoroughly and carefully renewed, there need be no great trouble or expense involved in that process, and no trespass or dispute meanwhile. Certain identification of the "line" trees of a previous authoritative survey is a great help in renewal. In the United States system that is secured by notching those trees; in the province of New Brunswick they are blazed and the blazes hacked three times upward. The same thing might be secured, and in addition the work of the individual surveyor identified,

by a personal mark, such as a stamp cut on the poll of the blazing axe.

4. ORIGINAL SURVEYS AND RESURVEYS

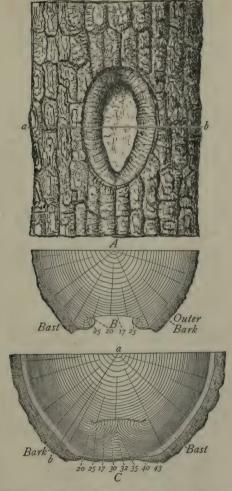
The woods surveyor has two broad classes of work to do,
— the running of new lines, outlining property for sale or
administration, and the work of relocation. The first
class of work constitutes an original survey, which the surveyor must carry out with due regard, on the one hand to
accuracy, on the other to cost. His ordinary duty here
consists of three parts: first, to duly outline and measure
the tract in question; secondly, to mark the bounds of it
in satisfactory fashion; third, to take notes of what he
does for record and the benefit of those who come after.

Resurveys. When a boundary has once been surveyed, marked on the ground, and accepted, it becomes authoritative, and the usual duty of the man who comes after is simply to locate the work of the original surveyor. He uses the compass commonly as the best means of finding the old lines and corners. He may use the chain for the same purpose, or to satisfy himself about area. But his business, so far as the boundary itself is concerned, is to find and remark the old one, not set up a new one according to his notions of propriety. In relocating that boundary the marks of the earlier surveyor are a more reliable guide than his notes: they must, however, be clearly identified and not confused with those of irresponsible parties. On the other hand, where monuments cannot be found, reliable verbal testimony is admitted, while it has further to be recognized that property boundaries may become sanctioned by use or agreement, even though they are crooked and astray from their original location.1

5. Age of Spots or Blazes

A subject of special interest to the forest surveyor is the determination of the age of spots on trees. This means

¹ For both legal and practical guidance in resurvey work, see "Restoration of Lost or Obliterated Corners," by the Land Office, and Hodgman's "Land Surveying."



 $A.\,B.\,$ Blaze Five Years after Cut was Made: A, Front View Showing Rim of Callus; B, Cross Section

C. BLAZE TWENTY-THREE YEARS AFTER CUT WAS MADE

of identifying a surveyor's work is recognized by all the courts. The handling of the problem in the field may be made clearer by the accompanying figures, reproduced from Circular No. 16, Division of Forestry, United States Department of Agriculture.

6. Notes

Notes should be full and exact so as to furnish for the benefit of later comers a complete record of the work done. In the case of resurveys they should be particularly clear as to the old marks found, so that the evidence which governed in the resurvey may be a matter of record. This rule holds especially in regard to starting points and corners.

The date of a survey is an important thing to record clearly, along with the meridian which was used, whether

magnetic, true, or one assumed for the occasion.

Notes should be so plainly and clearly written that any fairly intelligent man can understand them. They should be honest as well, not concealing actual errors. When the lines of a survey do not close in exactly, it may not be worth while to rerun them, but there ought at least to be no dodging of the facts. It is only an incompetent surveyor who will not acknowledge his errors. Errors are normal and to be expected. They grow out of imperfections in method that are imposed on the surveyor by limitations in the matter of expense. Errors are not to be confused with mistakes or blunders.

The notes of a timber land survey should also be full as regards topography. Such notes often give great assistance in the relocation of lines and corners. They are also of value to the owner and operator of such property.

7. PARTY AND COST

The great advantages of compass and chain surveying for woods work are that it is sufficiently accurate for most purposes, and that the cost involved is very moderate. Six

RE	enewal of South line of Twp., 5R.4, Oxford Co., Maine Sept. 25, 1905.
Line of	riginally run by E. Ballard in 1794, has been blazed over some since, but
DOVOE	resurveyed. E.S. Dearborn, rear chain.
HEVE	Have traced down and proved the east line of the township to a line
ofen	ts running west supposed to be its south line. Search along this shows
	20 rods a spruce and a birch with very old blazes which prove as
	s the rings can be counted to be III years old. Ablaze of like
	also found 3 rods to the eastward. No sign seen of the original
Corne.	, noted as being in a birch.
	In range of the spots east and west and in the line coming south
	cedar post and stones. This is in flat spruce land and 9 rods
	Island Pond to the eastward. Marked the post on N.W. T.5 R.4;
	T.4 R.4, On S. T.5 R.3, also "J.J.B. 1905." The witness trees, also marked
J.J.B.	1905, are a cedar standing N 10°E 10 links from the post, another
5.50°E	18 links, a spruce S. 30°W. 20 links & a birch N. 45°W 12 links.
	From the post ran atrial line N. 83°W. at right angles to the N & S line
After	85 rods found another original blaze 20 links to the left. Returned to
post a	nd ran N.83°30'W.
Rods	
80	Marked a birch right of line 4M >>
120	Rising onto the height of a ridge which falls off precipitously
	2 rods to the South. Original timber blown down and rotten here
	and some rods ahead. Found 3 of Ballard's spoks close to the swamp
	ing and some spots by lumbermen often wide of the line.
	Blazed through straight.
160	Marked a spruce right of line. ½ M >> Slope 5.W
210	Down a strong slope S.W. Old spots have been hauling to the right
210	and now one on a birch with III rings over it is 30 links right.
	Offset to it, fill in the line back over the old spots, and continue
240	on same bearing.
240	Set a cedar stake marked ¾M→
256	Water crosses to Southwest
275	Last 40 rods through swamp with mainly young growth and no
	spots to be seen.
	Old blaze probably Ballard's found now on a dead and down cedan
295	Cross Canada hay road.
320	A spot of Ballard's age on a spruce just back 2 rods South are
	Spots of much less age which come into the range a few rods
	further on. Blazed the line through straight Set a post for
	the corner of sections 35 & 36 marked on N.W "S.Nº 35."
	on N.E. "S. Nº 36.", on S. "T. S R.3" Marked it and the witness
	trees "J.J. B. 1905."
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Begin at Southwest corner of lot at junction of stone walls marking recognized by undaries of the lot. Thence— Bearing Dist. N 10°E 847' Along wall to its end 1917' through pine timber both sides with no sign of property (total) line, to a rotten tence running easterly. The deeds calling for a line running "in a northerly direction", I blazed the line through on the range of the wall and set a pound stones at its north end. This is on ledgy ground with a drop off 10 feet west: S 78°35'E. 1054' Along the old fence line Small brook runs N at 680 ft. to S E corner of the lot lying north, as indicated by range of old farm wall run in from the north to this point set a stone block on end and surrounded it with stones set several heaps of stones along the line. N 10°E 350' Onrange of farm wall mentioned and roughly along the bound of the cutting, in swampy land after 200'. Set stakes along the line each 200' and at the end a post with heap of stones. S 80°E 50' At right angles to the range line to Cohasse brook. This distance is the one (3 rods) Called for in the deed and is the only means of fixing the last named corner on the north and south line. S 35°E 176' S 53°E 319' Along Cohasse brook as per call of deed. S 80°E 335' Across brook, then an south border of field in poss sion of owners north, to west side of highway. This point is 716:ff shout 40 rods'' Set post and stone wall, the deed calling for "about 40 rods'' Set post and stone wall, the deed calling for about 40 rods'' Set post and stone wall, the S 22°W 168' Down high way to bridge over Cohasse brook as calls 520°30'E 250' for in deed S 40°30'E 333' In the swamp close to foot of the ridge S 26'W 168' Down high way to bridge over Cohasse brook as calls 520°30'E 250' for in deed S 40°30'E 330'W 1086' Along the wall to place of beginning This survey fallows the ferms of the keality 30 years and familiar with its kind transtead accupance was resident of the keality 30 years and familiar with its kind transtead accupance as a sealest of the kealit			So called Decl. of needle as near as known II. RY. Weston &
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men form a usual party for line work in the northern woods, and from one to three miles a day can commonly be run with it, according to the ground and growth. The usual expense for such work ranges between \$6 and \$10 per mile. A reliable transit line, on the other hand, cannot be cleared out and run for twice those figures.

The work of the forest surveyor may be done for the following purposes, and the party required for each sort of work, outside of maintenance, is noted in connection.

1. New work, for the purpose of sale or administration. Party required: compassman, two chainmen, enough men, commonly three, ahead of the compass, with axes and a

rod, to keep the rest of the party busy.

2. Resurvey, for the sake of reëstablishing lines and corners, also for getting area. Party: same as above; or it may be more economical in some circumstances not to employ chainmen, but for the surveyor himself, with one of his party, to go back and do the chaining.

3. Careful resurvey with the compass of old lines, no

chainage required. Party to correspond.

4. Remarking lines where no great difficulty is expected, but where the lines need freshening. The man in charge and two axemen form an economical party. A small fold-

ing sight compass may be used as needed.

Balance in the party is one element largely influencing cost. The main thing is to have sufficient axemen to give the rest of the party enough to do. Subsistence is an important problem in some circumstances. A chainman can carry a pack on his work, and frequently chainmen are employed on long jobs in the backwoods to carry a portion of the supplies or outfit.

SECTION V

COMPUTATION AND OFFICE WORK

1. Traverse

To "traverse" a line or route is to survey it by any method that ascertains direction and distance. The cir-

cuit of a farm's boundaries by compass and chain is a traverse. So is the survey of a road by usual methods.

When a survey has been made in this fashion the notes are for some purposes best worked up after a method called "computing by traverse," the principles and applications of which are developed in the following paragraphs.

If a course is run out N 30° E 20 chains, a certain distance is made in a northerly direction, also a certain distance in a direction east. The distance made in the former direction is called latitude; in the latter, departure. In this case it is north latitude and easterly departure. These elements may be made evident on a plot by drawing a meridian and base line through the starting point and lines perpendicular to these from the point reached. These distances are also to be obtained from traverse tables.

The same is true of a course run in any direction and for any distance. Any course not run exactly east and west makes northing or southing. The former is reckoned as positive latitude, with the sign (+). The latter is negative or (-) latitude. Similarly, distance made in an easterly direction is (+) departure; that made towards the west (-) departure. If several courses are run in succession, the sum, algebraically reckoned, of their latitudes and their departures gives the position of the point finally attained.

This method of reckoning, using traverse tables for the purpose, has a wide use in connection with land surveying. The traverse table given on pages 214-219 furnishes the elements for 15' courses, those usually employed in compass work. The following is a simple problem illustrating their use.

In running a section line due north, the surveyor comes to a lake shore. Setting there a post, duly marked, he runs round the lake near the shore by the following courses:

> N 50° E 12 chains. N 9° 30′ E 20 " N 40° W 9 " S 80° W 6.81 "

Reckoning up his courses by the traverse table, he finds

that his E and W departures balance, hence he should be in line. The difference between northing and southing gives him the distance. He may then set a second post, add the distance to his previous chainage, and proceed with his survey.

COMPUTED TRAVERSE

Field N	Fro	m Trave	erse Table	3.	
Bearing.	Distance.	Latitude.		Depa	rture.
		N.	S.	E.	w.
N. 50° E.	12.0 chains	7.71		9.19	
N. 9° 30′ E.	20.0 "	19.73		3.30	
N. 40° W.	9.0 "	6.89			5.78
S. 80° W.	6.81 "		1.18		6.71
		34.33	1.18	12.49	12.49
		1.18			
Distance due north		33.15 с	hains	Bala	ance

When a closed survey is made, that is to say, when a surveyor starts and finishes at the same point, it is evident that its (+) and (-) departures should be equal, also its (+) and (-) latitudes. Owing to the errors unavoidable in survey work it is very seldom that they do so reckon up exactly. The amount by which the two ends fail to meet, whether plotted or reckoned, is the error of closure, and the percentage of error is the ratio of this distance to the total length of the survey. A certain percentage of this error, say 1 in 500 or 1 in 300, may be allowable in an ordinary woods survey. For plotting and for area, however, it may be desirable to distribute the error through the different courses, and this, when the traverse has been reckoned out. is readily done. The error in both latitude and departure is usually distributed to the different courses in proportion to the length of each, but if any course was more difficult of chainage than the others, it may be given extra weight in

the distribution. In any case the correction is applied so as to help close the survey and not the reverse. This process is called Balancing a Survey.

The field notes of a closed survey, the latitudes and departures as they reckon out, and the same balanced, are given herewith. The reckoning is also given, and all is in convenient arrangement. The latitudes and departures

COMPUTING LATITUDES AND DEPARTURES

	Course. A — B	Course. B — C	Course.	Course. D — E	Course. E — A
log sin =	9.9386	9.7604	9.5340	9.9555	9.5163
log dist. =	1.3010	1.1790	1.0910	1.2109	1.3444
log dep. =	1.2396	0.9394	0.6250	1.1664	0.8607
Departure =	17.36	8.70	4.22	14.67	7.26
log cos =	9.6957	9.9125	9.9730	9.6340	9.9752
log dist. =	1.3010	1.1790	1.0910	1.2109	1.3444
log lat. =	0.9967	1.0915	1.0640	0.8449	1.3196
Latitude =	9.92	12.35	11.59	7.00	20.87

in this case have been reckoned out not from the traverse table, but from the table of logarithmic sines and cosines. A little consideration shows that the latitude of a course is the cosine of its bearing multiplied by its distance, while the departure is the product of the sine multiplied by the distance. Now a table of sines and cosines gives values to single minutes instead of for 15' bearings. Logarithmic computation, too, shortens the process. This is, therefore, the more convenient way of reckoning for transit work, or for accurate compass surveying.

When all but the final course has been run, it is in some circumstances desirable to ascertain what course to set in order to hit the starting point. This, too, may readily be done by means of the figured latitudes and departures.

Thus, suppose that four courses of the above survey have

BALANCING A CLOSED SURVEY

		1							
	W.	:		4.21	14.65	7.24	26.10		
Balunced.	Ē	17.38	8.72	:	:	:	26.10		
Balm	vi	9.95		:	:	20.91	30.80		
	ż	:	12.32	11.57	6.97	:	30.86		
ture.	W.	1 :	:	4.22	14.67	7.26	26.15	26.06	60.
Departure.	Ei.	17.36	8.70		:	*	26.06		
Latitude.	vi	9.92	:	:	:	20.87	30.79		
Latit	ż	:	12.35	11.59	7.00	:	30.94	30.79	.15
ses.	Distance.	20.00 chains	15.10 "	12.33 "	16.25 "	22.10 "	200	3	
Courses.	· Bearing.	S. 60° 15′ E	N. 35° 10′ E	N. 20° W	N. 64° 30′ W	S. 19° 10′ W			
Station.		A	В	D .	D	国			

Error in latitude .15 ch. = 1 link per each 5.7 ch. of perineter, to be added to southings and subtracted from northings. Error in departure .09 ch. = 1 link per each 94 ch. of perimeter, to be added to eastings and subtracted from westings. $\frac{17.5}{8578} = 1 \text{ in 490.}$ Error of closure = $\sqrt{15^2+9^2} = 17.5 \text{ links}$.

been run out and the latitude and departure computed, as given. The result shows that the point reached is north

FIGURED LATITUDES AND DEPARTURES

	Lati	tude.	Departure.		
	N. S.		E.	w.	
A — B		9.92	17.36		
В-С	12.35		8.70		
C-D	11.59			4.22	
D-E	7.00			14.67	
	30.94	9.92	26.06	18.89	
	9.92		18.89		
	21.02		7.17		

departure.

Now to find the bearing of E A we have

tan.
$$A E X = \frac{A X}{E X} = \frac{7.17}{21.02} = .3411.$$

A E X from the table of tangents = 18° 50'. S 18° 50' W is therefore the bearing required.

The length of $E \Lambda$ may also be found, since it is the hypothenuse of a right angled tri-

angle whose base and altitude are the latitude and departure given.

$$\sqrt{21.02^2 + 7.17^2} = 22.21$$

the distance required. That this value and that for the angle differ somewhat from the true ones is due to the errors of compass surveying.

In a similar way the course and distance of an inaccessible line may be computed or omissions supplied in notes.

That is a very undesirable thing to do, however, as it infringes on the tests which serve to verify the work.

2. AREA

Rectangles. The woodsman in his land work has most frequently to do with rectangular figures, and computation of area is simple. If the average of the chained east and west sides of a rectangular piece of land is 201 rods or 50.25 chains, and the north and south dimension 40 chains, the area equals $50.25 \times 40 \div 10$ (the number of square chains in an acre), or 201 acres. So with a rectangular piece of any dimensions.

Area by Triangles. The area of a triangle of known base and altitude is half the product of these dimensions, and an irregular figure when plotted may be cut into triangles, the dimensions of each measured, and the areas computed. The same process in case of necessity may

be performed on the ground.

When, as is frequently the case, it is easier to obtain the three sides of a triangle than the base and altitude, the area may be obtained from the formula

Area =
$$\sqrt{s(s-a)(s-b)(s-c)}$$
,

where a, b, and c are the three sides and s is half their sum.

Or, lastly, an irregular figure when plotted may be reduced graphically to the triangular form and the area obtained at one computation by either of the methods just given.

The relations between units of distance and of area are

given on page 19.

By Offsets. In surveying around the borders of a body of water, and in some cases when the exact border of a property presents great difficulties, it is customary to run as near the border as is practicable and to take rectangular offsets to it at selected intervals along the line. These offsets should be measured to angles in the border, or placed near enough together so that the border between offsets may be considered a straight line. The area of the figure between each two offsets may then be computed by multiplying the distance along the base by half the sum of the two offsets.

Another way is to take the offsets at regular distances along the base, 10 rods apart for instance. In that case the rule for the area is:—Add together all the intermediate offsets and half the end offsets, and multiply the sum by the constant interval between them.

By Cross Sectioning. The method of ruling off an area on a map into squares of equal and known size is very convenient, especially for irregular areas like bodies of water. The whole squares can be counted up and the fractions of squares estimated. In such cases it may be best to do the ruling not on the map itself but on a detached piece of tracing cloth or of paper. If the map is opaque, the ruled tracing cloth may be laid over it and held firmly till the work is done. If it is transparent, the ruled sheet may be laid underneath.

By Planimeter. The area of any surface may be quickly and accurately ascertained by an instrument called the planimeter. That instrument is not, however, in the

hands of most woodsmen.

From Traverse. The area enclosed by a balanced survey may be accurately computed from the latitude and departure of its courses. The general scheme will be grasped at once from the figure, in which A B C D E represents the survey whose notes are given on page 35, e b is a meridian through its most westerly point, b B, c C, dD, and e E are lines drawn vertical to it from the angles, and B m, D n, and E o are parallel to it or vertical to c C and d D. In this figure it is evident in the first place that

the area of the figure b B C D E e minus the area of the two triangles A E e and A B b equals the area of A B C D E, and secondly that the figure b B C D E e is made up of

the three trapezoids b B C c, c C D d, and d D E e. The area of these trapezoids and triangles is easily computed from their dimensions. All that is necessary is to express those dimensions clearly in terms of latitude and

departure.

The bases of these triangles and trapezoids are clearly related to departure. b B is the departure of the course A B, and A $\hat{b} \times b$ B = twice the area of A B b. b B + c C, the two bases of the trapezoid b B C c, = twice the departure of AB + the departure of BC, cC + dD= the same expression as the last + the departure of B C+ the departure of C D, which last, however, being westerly, is reckoned negatively. Now a general expression for these values is double meridian distance, meridian distance being perpendicular distance from the meridian. The D. M. D. of a course is the sum of the meridian distances of its two ends. For a course starting on the meridian it equals the departure of the course. For any succeeding course it equals the D. M. D. of the preceding course plus the departure of that course plus the departure of the new course, easterly departures being reckoned as positive and westerly departures as negative.

A check on the reckoning of the D. M. D.'s is in the last one, which should be numerically equal to the de-

parture of the last course.

These elements for convenient working out of the area surrounded by a closed survey are embodied in the following rule: — Twice the area of the figure enclosed by a survey is equal to the algebraic sum of the products of the D. M. D.'s of the several courses multiplied by the corresponding latitudes, north latitudes being reckoned positively and south latitudes negatively. If the tract is kept on the right in the course of the survey, the result comes out with a minus sign.

An operation of this kind, starting with the balanced latitudes and departures, may be conveniently arranged

as follows:

Course.	Lat.	Dep.	D. M. D.	+ Area.	Area.
A - B	- 9.95	+ 17.38	17.38		172.93
B-C	+ 12.32	+ 8.72	43.48	535.67	
C-D	+ 11.57	- 4.21	47.99	555.24	*
D-E	+ 6.97	- 14.65	29.13	203.04	• • •
E-A	-20.91	- 7.24	7.24		151.39
				4000 0#	1

324.32 2)969.63 484.81 sq. ch. Area = 48.48 acres.

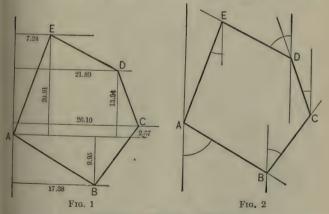
3. PLOTTING

The computation of traverse, if it aids in testing the accuracy of a survey, gives also data for plotting it with ease and accuracy. Taking the initial point of the survey as the starting point for a meridian and a base line vertical to it, the position of the second point of the survey may be fixed by measuring off its latitude on the vertical line, its departure on the horizontal, and from these points drawing lines parallel to the base and the meridian until they intersect. The latitude of the second course may then be added to that of the first and the two departures also added to gether, when the third point of the survey may be fixed in the same way as before, and so on until the survey is finished. The points thus fixed may then be joined by lines representing the courses. The position of the points in the above survey as taken from the balanced figures on

page 35 is given in the table, and below is a diagram showing the method of plotting.

Point.	N.	S.	E.	w.
A B		9.95	17.38	
C	2.37		26.10	
D	13.94		21.89	
E	20.91		7.24	

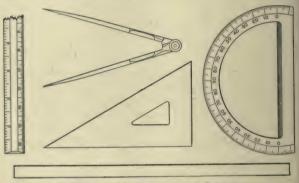
It is not, however, the most common practice to plot a survey after this fashion. The more usual way is to plot the angles and distances directly from the notes. To do this select a point on the paper for the initial point of the survey and draw a meridian through it in pencil. Then by means of a protractor mark the bearing of the first



METHODS OF PLOTTING A SURVEY.
FIG. 1 BY LATITUDES AND DEPARTURES. FIG. 2 BY COURSES AND DISTANCES.

course and draw a line of indefinite length through it. On this line lay off to scale the length of the course, thus establishing the second corner. Through this draw another meridian in pencil and proceed as before. If the survey and the plotting are both perfect, the last course should hit the initial point. If it does not so hit, there is error in one or the other.

To plot one course from another by means of the figured angles between them is not good practice, because by that method errors accumulate.



THE ESSENTIAL INSTRUMENTS FOR PLOTTING

A straight edge, a scale, a protractor, a pair of dividers, and a parallel ruler or a pair of triangles are the essentials for ordinary plotting.

The lettering on a woodsman's map ought to be plain. The size of the letters should be varied according to the importance of the object designated. It is a good rule to use erect letters in general, and slant capitals and italics in connection with water.

The usual practice is to represent waters and swamps with blue ink, contours with brown, and all other objects with black. Common brown and blue inks, however, do not blueprint well, so black is ordinarily used for tracings.

Various systems have been devised for representing the character and density of timber growth. A system of that kind, if one is required, is best devised for each forest region or property.

Maps may be rendered plainer by the judicious use of

topographic symbols. A number that are in common use and generally agreed upon are given herewith.

Railroad
Highway
Wood Road
Trail
Stone Wall
Fence
Telephone Line
Field or Prairieant. allie, all
Open Swamp
Dam

TOPOGRAPHIC SYMBOLS

SECTION VI ON THE BEARING OF LINES

The surveying work of the woodsman of the present day is mostly of the nature of resurveys, or the subdivision of tracts whose boundary lines are on the ground. To ascertain correctly the present bearing of old lines is therefore a problem of great importance and one very frequently met with.

1. Bearing Directly Observed. The best and surest way to find that direction is the direct one of running a piece of the line. For example, suppose a section of land was run out in 1845 with lines stated to run north, east, south, and west by the true meridian. The surveyor coming on to retrace it in 1915 may pay no attention to the north star or reference meridians, but finding the southwest corner of the tract plain and running northerly find by trial

that N 4° 20′ E runs through the old spots. He figures now that the courses he will have to run in order to reproduce the lines of the square are N 4° 20′ E, S 85° 40′ E, S 4° 20′ W, and N 85° 40′ W. He may run them so or turn the vernier of his compass 4° 20′, so as to read N, E, S, and W, like the compass of the original surveyor. In any case he will not be able to reproduce the old line all around exactly. Even if no errors are made in either survey the daily variation of the needle will be pretty sure to cause some divergence. In remarking the line he will follow as closely as possible the marks of the old surveyor.

2. By Reference Meridian. The change in bearing of old lines may often be ascertained by reading on a reference meridian. If the compass in use be so tested and if the compass which did the work to be reviewed was tested on the same marks at the time of the original survey, then the difference in the two bearings will hold closely for a

considerable region around.

Example: On a county meridian in Pennsylvania in 1850 a surveyor's compass read N 2° 30′ E and in the neighborhood a line was run bearing S 55° E. In 1905 another compass on the meridian reads N 6° 20′ E, showing a change of 3° 50′ in the time elapsed. Then S 51° 10′

E ought to reproduce the line.

3. By Tables. The following tables, derived from publications of the United States Coast and Geodetic Survey, are very convenient for determining change in declination. They give for many localities well distributed throughout the United States declination at tenyear intervals as far back as it has been recorded. The change found to have taken place at a given locality between any two dates may then be applied through a considerable region around it. It should be understood, however, that this means of determination does not obviate the chances of error due to difference between instruments. It is well known that two compasses on the same line at the same time may not read exactly alike.

Example: A land line in the Adirondacks was run out in 1800 on the magnetic meridian. What course should

be set in 1910 to reproduce it?

TABLE GIVING SECULAR CHANGE OF THE MAGNETIC DEC-LINATION IN THE UNITED STATES

(From U. S. Coast and Geodetic Survey Reports)

Year	Maine	Maine	New	Ver-	Mass.	Mass.
(Jan. 1)	N'theast	S'thwest	Hamp.	mont	East	West
1750 1760	° ' 12 05W 11 53	8 34W 8 15	8 02W 7 28	7 43W 7 09	7 46W 7 19	6 21W 5 52
1770	11 53	8 10	7 03	6 44	7 00	5 31
1780	12 05	8 10	6 47	6 28	6 50	5 19
1790	12 26	8 15	6 42	6 23	6 50	5 17
1800	12 58	8 34	6 49	6 30	7 01	5 25
1810	13 38	9 02	7 06	6 47	7 20	5 54
1820	14 23	9 38	7 32	7 13	7 47	6 08
1830	15 12	10 18	8 11	7 48	8 22	6 41
1840	16 02	10 57	8 56	8 29	9 04	7 21
1850	16 58	11 38	9 46	9 13	9 48	8 05
1860	17 43	12 18	10 31	9 59	10 28	8 43
1870	18 13	12 48	11 08	10 39	11 01	9 17
1880	18 34	13 22	11 38	11 14	11 32	9 58
1890	18 44	13 51	12 03	11 39	12 02	10 25
1900	19 02	14 21	12 31	12 08	12 34	10 59
1910	19 45W	15 06W	13 16W	12 57W	13 21W	11 42W

Year	Rhode	Conn.	N. Y.	N. Y.	Penn.	Penn.
(Jan. 1)	Island		East	West	East	West
	0 /	0 /	0 /	0 /	0 /	0 /
1750 1760 1770 1780 1790	7 04W 6 37 6 18 6 08 6 08	5 47W 5 18 4 57 4 45 4 43	7 35W 6 53 6 17 5 50 5 34	4 40W 3 57 3 18 2 46 2 24	4 47W 4 01 3 19 2 44 2 21	1 16W 0 52
1800	6 19	4 51	5 28	2 13	2 08	0 37
1810	6 38	5 08	5 34	2 13	2 09	0 31
1820	7 05	5 34	5 50	2 24	2 22	0 37
1830	7 40	6 07	6 17	2 46	2 47	0 52
1840	8 22	6 47	6 53	3 18	3 21	1 16
1850	9 06	7 31	7 39	3 57	4 04	1 48
1860	9 46	8 09	8 25	4 46	4 46	2 26
1870	10 19	8 43	9 04	5 23	5 32	3 06
1880	10 50	9 24	9 51	6 16	6 16	3 50
1890	11 20	9 51	10 14	6 57	6 50	4 28
1900	11 52	10 25	10 48	7 37	7 25	5 07
1910	12 40W	11 11W	11 31W	8 12W	8 07W	5 45W

TABLE GIVING SECULAR CHANGE OF THE MAGNETIC DEC-LINATION IN THE UNITED STATES

(From U. S. Coast and Geodetic Survey Reports)

Year (Jan. 1)	New Jersey	Ohio	Indiana	Illinois	Iowa	Mich. North
1750 1760 1770 1770 1780 1800 1810 1820 1830 1840 1850 1860 1870 1880 1890	0 / 4 43W 4 04 3 51 3 06 3 30 3 31 4 04 4 43 5 5 22 6 001 6 41 7 7 49	3 13E 3 22 3 22 3 23 2 53 2 24 1 50 1 14 0 37E 0 02W 0 42	4 44E 4 59 5 04 4 44 4 21 3 50 3 13 2 35 1 57 1 24	5 54E 6 18 6 33 6 37 6 33 6 18 5 54 5 26 4 44 4 05 3 36	10 09E 10 24 10 30 10 24 10 09 9 44 9 06 8 21 7 52	6 42E 6 42 6 28 6 28 6 02 5 25 4 38 3 47 2 58 2 20 2 05E

Year	Michigan	Wisconsin	Minnesota	Minnesota	
(Jan. 1)	South		North	South	
1750 1760 1770 1780 1790 1800 1810 1820 1830 1840 1850 1860 1870 1880 1890	4 10E 4 04 3 46 3 20 2 46 2 04 1 17 0 32E 0 02W 0 27W	8 34 E 8 40 8 34 8 16 7 49 7 14 6 25 5 36 5 01 4 51E	10 27E 10 44 10 50 10 44 10 27 9 59 9 17 8 33 7 58 8 03E	.11 20E 11 36 11 42 11 36 11 20 10 54 10 22 9 32 8 57 9 00E	

ABLE GIVING SECULAR CHANGE OF THE MAGNETIC DECLINATION IN THE UNITED STATES

(From U. S. Coast and Geodetic Survey Reports)

Year (Jan. 1)	Washington D. C.	Maryland (Baltimore)	Virginia East (Richmond)	Virginia West (Lynchburg)	West Virginia (Charleston)	North Caro- lina East (Newbern)	North Carolina West (Salisbury)
1750 1760 1770 1780 1790 1800 1810 1820 1830 1840 1850 1860 1870 1880 1890 1900	1 41W 1 02 0 28 0 01W 0 19E 0 28 0 28 0 29 0 19E 0 01W 0 28 1 02 1 41 2 21 4 11 4 51W	3 05W 2 26 1 52 1 25 1 05 0 56 0 56 1 05 1 25 1 52 2 26 3 05 3 45 4 24 4 24 5 00 5 35 6 15W	1 13W 0 37 0 05W 0 20E 0 38 0 47 0 47 0 38 0 20E 0 05W 0 36 1 151 2 29 3 06 3 40 4 13W	0 08E 0 42 1 11 1 33 1 46 1 51 1 46 1 33 1 11 0 45 0 10E 0 29W 1 09 1 46 2 22 2 53W	2 00E 2 15 2 20 2 15 2 00 1 37 1 05 0 30E 0 12W 0 51 1 28 2 06 2 39W	0 18W 0 18E 0 50 1 17 1 35 1 44 1 44 1 35 1 16 0 50 0 17E 0 58 1 35 2 2 51 3 25W	1 3/1E 2 08 2 42 3 12 3 34 3 48 3 52 3 48 3 352 3 48 3 310 2 406 1 29 0 51 0 13E 0 23W 0 47W

Year (Jan. 1)	South Carolina (Columbia)	Georgia (Macon)	Florida East (Jack- sonville)	Florida West (Pensacola)	Florida South (Tampa)	Alabama (Montgom- ery)	Mississippi (Jackson)
1750	2 04E	3 16E	2 27E	5 00E	5 00E	2 52E	0 /
1760	2 41	3 53	3 04	5 37	5 30	3 28	
1770	3 15	4 29	3 40	6 13	5 55	4 03	
1780	3 44	5 01	4 12	6 44	6 15	4 34	
1790	4 06	5 26	4 37	7 11	6 26	5 02	
1800	4 19	5 44	4 55	7 32	6 30	5 24	7 54E
1810	4 24	5 53	5 04	7 45	6 26	5 39	8 13
1820	4 19	5 53	5 04	7 50	6 15	5 47	8 24
1830	4 06	5 44	4 55	7 45	5 55	5 46	8 28
1840	3 44	5 26	4 37	7 31	5 30	5 38	8 24
1850	3 15	5 01	4 12	7 12	5 00	5 22	8 13
1860	2 41	4 29	3 40	6 45	4 28	5 00	7 57
1870	2 03	3 53	3 04	6 13	3 53	4 32	7 31
1880	1 25	3 14	2 25	5 34	3 16	3 54	6 55
1890	0 47	2 39	1 50	4 57	2 48	3 15	6 21
1900	0 11E	2 08	1 19	4 29	2 19	2 49	5 58
1910	0 12W	1 52E	1 05E	4 22E	2 06E	2 45E	6 08E

TABLE GIVING SECULAR CHANGE OF THE MAGNETIC DECLINATION IN THE UNITED STATES

(From U. S. Coast and Geodetic Survey Reports)

(Liberty Control of the Control of t							
Year (Jan. 1)	Tennessee East (Chat-tanooga)	Tennessee West (Bun- tingdon)	Kentucky East (Lexington)	Kentucky West (Princeton)	Louisiana (Alexandria)	Texas East (Houston)	Texas Middle (San Antonio)
1750 1760 1770 1780 1790 1800 1810 1820 1830 1840 1850 1860 1870 1880 1900 1910	5 07E 5 16 5 16 5 07 4 49 4 24 3 52 3 16 2 36 2 201 1 30 1 12E	7 24E 7 24 7 16 6 59 6 35 6 05 5 29 4 53 4 18E	4 22E 4 31 4 31 4 31 4 22 4 04 3 39 3 07 2 31 1 53 1 15 0 41 0 19E	6 32E 6 50 6 59 6 59 6 50 6 32 6 07 7 4 57 4 57 4 20 3 36E	8 04E 8 25 8 41 8 49 8 48 8 40 8 24 8 02 7 26 6 53 6 53 6 50E	8 55E 9 10 9 19 9 19 9 12 8 56 8 29 7 56 7 54 8 05E	9 37E 9 48 9 53 9 48 9 37 9 19 8 52 8 43 9 09E

Year (Jan. 1)	Texas West (Pecos)	Arkansas (Little Rock)	Oklahoma (Okmulgee)	Missouri (Sedalia)	Kansas East (Emporia)	Kansas West (Ness City)	Nebraska East (Hastings)
1750 1760 1770 1780 1800 1810 1820 1830 1840 1850 1870 1890 1900 1910	10 46E 11 00 11 08 11 07 11 07 11 02 10 18 10 50E	8 13E 8 36 8 51 9 00 8 59 8 51 8 34 8 14 7 38 7 01 6 38 6 49E	10 15E 10 06 9 51 9 33 9 07 8 42 8 55E	10 03E 10 13 10 13 10 04 9 46 9 24 8 44 8 02 7 38 7 46E	11 34E 11 28 11 12 10 07 9 50 10 08E	12 24E 12 23 12 12 11 54 11 108 11 27E	11 39E 11 57 12 07 12 07 11 59 11 11 10 10 31 10 14 10 281

'ABLE GIVING SECULAR CHANGE OF THE MAGNETIC DECLINATION IN THE UNITED STATES

(From U. S. Coast and Geodetic Survey Reports)

Year (Jan. 1)	Nebraska West (Alliance)	South Da- kota East (Huron)	South Da- kota West (Rapid City)	North Da- kota East (Jamestown)	North Da- kota West (Dickinson)	Montana East (Forsyth)	Montana West (Helena)
1750 1760 1770 1780 1790 1800 1810 1820 1830 1840		13 06E	0 /	0 /	0 /	° ,	18 53E
1850 1860 1870 1880 1890 1900 1910	15 27E 15 27 15 18 14 50 14 20 14 10 14 31E	13 06 12 57 12 39 12 07 11 25 11 07 11 28E	16 26E 16 26 16 16 15 50 15 17 15 07 15 27E	14 31E 14 21 14 02 13 31 12 43 12 24 12 44E	17 37E 17 37 17 27 17 00 16 21 16 10 16 36E	18 27 18 36 18 36 18 21 17 53 17 50 18 17E	19 18 19 36 19 45 19 34 19 23 19 31 20 02E

Year (Jan. 1)	Wyoming East (Douglas)	Wyoming West (GreenRiver)	Idaho (Boise)	Washington East (Wilson Creek)	Washington West (Seattle)	Oregon East (Sumpter)	Oregon West (Detroit)
1750 1760 1770 1770 1780 1790 1800 1810 1820 1830 1840					17 19E 17 52 18 27 19 04 19 41 20 16 20 49	• /	16 05E 16 43 17 22 18 01 18 38
1850 1860 1870 1880 1890 1900 1910	15 51E 15 59 15 59 15 47 15 24 15 19 15 43E	16 45E 16 58 17 02 16 54 16 36 16 37 17 08E	18 00E 18 30 18 45 18 45 18 39 18 51 19 31E	21 16E 21 37 21 52 21 56 22 06 22 22 23 00E	21 19 21 45 22 06 22 19 22 38 22 58 23 40E	19 15E 19 40 19 58 20 09 20 11 20 26 21 07E	19 12 19 41 20 06 20 24 20 32 20 50 21 33E

TABLE GIVING SECULAR CHANGE OF THE MAGNETIC DECLINATION IN THE UNITED STATES

(From U. S. Coast and Geodetic Survey Reports)

	manus abstraction trans-					
Year (Jan. 1)	California South (Los Angeles)	California Middle (San Jose)	California North (Redding)	Nevada East (Elko)	Nevada West (Haw- thorne)	Utah (Salt Lake)
1750 1760 1770 1770 1780 1790 1800 1810 1820 1830 1840	10 24E 10 58 11 32 12 07 12 39 13 09 13 36	13 37E 14 03 14 32 15 01 15 30 15 57 16 22	14 07E 14 35 15 04 15 34 16 04 16 33 17 01	0 ,	0 ,	0 /
1850 1860 1870 1880 1890 1900 1910	13 57 14 13 14 24 14 33 14 36 14 52 15 35E	16 45 17 05 17 20 17 28 17 32 17 51 18 32E	17 26 17 47 18 06 18 15 18 20 13 39 19 22E	17 20E 17 36 17 41 17 44 17 38 17 49 18 27E	16 16E 16 37 16 52 17 00 17 02 17 17 17 58E	16 25E 16 36 16 40 16 30 16 20 16 28 17 03E

Year (Jan. 1)	Colorado East (Pueblo)	Colorado West (Glen- wood Springs)	New Mexico East (Santa Rosa)	New Mexico West (Laguna)	Arizona East (Holbrook)	Arizona West (Prescott)
1750 1760 1770 1780 1790 1800 1810 1820 1830 1840		0 /	0 /	0 /	0 /	0 /
1850 1860 1870 1880 1890	13 47E 13 50 13 46 13 31 13 00 12 53	16 07E 16 15 16 16 16 04 15 40 15 39	12 43E 12 47 12 43 12 29 12 03 11 59	13 26E 13 33 13 34 13 22 13 02 13 02 13 36E	13 33E 13 44 13 47 13 40 13 25 13 29	13 19E 12 33 13 40 13 36 13 32 13 44
1910	13 19E	16 10E	12 29E	13 36E	14 05E	14 25E

From the table for change of declination, and for the locality eastern New York, the values 5° 28' and 11° 31' are obtained, showing that the needle in the 110 years swung 6° 03' to the westward. The desired bearing therefore should prove to be N 6° E nearly.

SECTION VII

ON OBTAINING THE MERIDIAN

When for any reason it is necessary to determine a true meridian, that is best obtained from the north star. This star, easily identified by the range of the "pointers," is not exactly at the pole of the heavens, but in 1908 was 1° 11′ 4″ from it. This angle is called the "polar distance" of the star. It is decreasing at the rate of about one third of a minute yearly.

The north star, like other stars, is thus circling around the pole once in about 24 hours. When directly over or under the pole it is said to be in culmination, upper or lower as the case may be. The star is then in the meridian, and bringing it down with plumb line or transit gives the

meridian directly.

When the north star is farthest from the meridian it is said to be in elongation, east when the star is east of the meridian, west when on the opposite side. A plane through the observer, the zenith, and the north star when at elongation, prolonged downward to the horizon, makes an angle with the meridian which is called the azimuth of the star at that time. This angle may be obtained for any time and position from tables, and setting off the angle, the true meridian is found. Upon this meridian the needle can be read or marks can be left for reference at any future time.

The operation of bringing down the star may be performed either with the plumb line or, more accurately and conveniently, with a well-adjusted transit. When the transit is used it is necessary to illuminate the cross wires. This may often be done by holding a lantern or candle in front of the transit tube and a little to one side, when the field should appear light with the cross hairs show-

ing as dark lines. If light enough is not so obtained, a tin reflector may be made of the design shown, or a



REFLECTOR

piece of tracing cloth or greased paper with a hole cut in it may be bound bellshape over the front of the instrument with a string or rubber band.

Directions for obtaining the true meridian which involve an accurate knowledge of time are not adapted to the use of the woodsman. The following directions do not impose that very difficult requirement.

(From United States "Manual of Instructions for Survey of the Public Lands.")

TO OBTAIN A MERIDIAN AT CULMINATION OF POLARIS

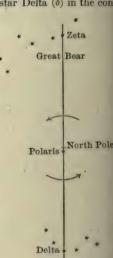
A very close approximation to a meridian may be had by remembering that Polaris very nearly reaches the meridian when it is in the same vertical plane with the star Delta (δ) in the con-

The vertical stellation Cassiopeia. wire of the transit should be fixed upon Polaris, and occasionally brought down to the star Delta, to observe its approach to the same vertical line. When both stars are seen upon the wire, Polaris is very near the meridian. A small interval of time (as 6 min. in 1908) will then be allowed to pass, while Delta moves rapidly east and Polaris slightly east to the actual meridian. At that moment the cross wire should be placed upon Polaris, and the meridian firmly marked by stakes and tack-heads.

This method is practicable only when the star Delta is below the pole during the night; when it passes the meridian above the pole, it is too near the zenith to be of service, in which case the star Zeta (ζ) , the last star but one in the tail of the Great Bear, may be used instead.

Delta (δ) Cassiopeiæ is on the meridian below Polaris and the pole, at midnight about April 10, and is, there-

fore, the proper star to use at that date and for some two or three months before and after.



Cassio peis

Six months later the star Zeta (5), in the tail of the Great Bear, will supply its place, and will be used in precisely the same manner.

The diagram, drawn to scale, exhibits the principal stars of the constellations Cassiopeia and Great Bear, with Delta (δ) Cassiopeiæ, Zeta (ζ) Ursæ Majoris (also called Mizar), and Polaris on the meridian, represented by the straight line; Polaris being at lower culmination.

In the above process, the interval of waiting time may be found for the proper year from the following data:

*	For Zeta Urs. Maj.	$\begin{cases} 1910 \\ 1920 \\ 1930 \end{cases}$		•	6.5 10.6 14.7	min.	{	annual increase .41 min.
	For Delta Cass.	$\left\{ \begin{array}{c} 1910 \\ 1920 \\ 1930 \end{array} \right.$	 		$7.1 \\ 11.0 \\ 14.9$	min.	{	annual increase .39 min.

* Data furnished by Prof. Robt. W. Willson.

Instead of the transit the plumb line may be used for this observation in much the manner described later on.

At certain times of year it is inconvenient to observe Polaris at culmination, and for other reasons as well it is more usual to observe the star at elongation. The Land Office instructions follow, and the table for azimuths of the star and for time of elongation which are required.

TO ESTABLISH A MERIDIAN AT ELONGATION BY TELESCOPIC INSTRUMENT

Set a stone, or drive a wooden peg, firmly in the ground, and

upon the top thereof make a small, distinct mark.

About thirty minutes before the time of the eastern or western elongation of Polaris, obtained from the table, set up the transit firmly, with its vertical axis exactly over the mark, and carefully level the instrument.

Illuminate the cross wires by the light from a suitable lantern, the rays being directed into the object end of the telescope by an assistant; while great care will be taken, by perfect leveling, to insure that the line of collimation describe a truly vertical plane.

Place the vertical wire upon the star, which, if it has not reached its elongation, will move to the right for eastern, or to the left for

western elongation.

While the star moves toward its point of elongation, by means of the tangent screw of the vernier plate it will be repeatedly covered by the vertical wire, until a point is reached where it will appear to remain on the wire for some time, then leave it in a direction contrary to its former motion; thus indicating the time of elongation.

Then while the star appears to thread the vertical wire, depress

the telescope to a horizontal position; five chains north of the place of observation set a stone or drive a firm peg, upon which by a strongly illuminated pencil or other slender object, exactly coincident with the vertical wire, mark a point and drive a tack in the line of sight thus determined; then, to eliminate possible errors of collimation or imperfect verticality of the motion of the telescope, quickly revolve the vernier plate 180°, direct the glass at Polaris and repeat the observation; if it gives a different result find and mark the middle point between the two results. This middle point, with the point marked by the plumb bob of the transit, will define the trace of the vertical plane through Polaris at its eastern or western elongation, as the case may be.

By daylight lay off to the east or west, as the case may require, the proper azimuth taken from the following table (page 56); the instrument will then define the meridian. The needle may be read then, giving the magnetic declination, east or west as the case may be. Or the line may be permanently marked for reference

at another time or with another instrument.

TO DETERMINE A MERIDIAN WITHOUT A TELESCOPE

Attach a plumb line to a support situated as far above the ground as practicable, such as the limb of a tree, a piece of board nailed or otherwise fastened to a telegraph pole, a house, barn, or other building, affording a clear view north and south.

The plumb bob may consist of some weighty material, such as a brick, a piece of iron or stone, weighing four to five pounds, which will hold the plumb line vertical, fully as well as one of

finished metal.

Strongly illuminate the plumb line just below its support by a lamp or candle, care being taken to obscure the source of light

from the view of the observer by a screen.

For a peep sight, cut a slot about one-sixteenth of an inch wide in a thin piece of board, or nail two strips of tin, with straight edges, to a square block of wood, so arranged that they will stand vertical when the block is placed flat on its base upon a smooth horizontal rest, which will be placed at a convenient height south of the plumb line and firmly secured in an east and west direction, in such a position that, when viewed through the peep sight, Polaris will appear about a foot below the support of the plumb line.

The position may be practically determined by trial the night

preceding that set for the observation.

About thirty minutes before the time of elongation, as obtained from the table, bring the peep sight into the same line of sight with

the plumb line and Polaris.

To reach elongation, the star will move off the plumb line to the east for eastern elongation, or to the west for western elongation; therefore by moving the peep sight in the proper direction, east or west, as the case may be, keep the star on the plumb line until it appears to remain stationary, thus indicating that it has reached its point of elongation. The peep sight will now be secured in place by a clamp or weight with its exact position marked on the rest, and all further operations will be deferred until the next morning.

By daylight, place a slender rod at a distance of two or three hundred feet from the peep sight, and exactly in range with it and

the plumb line; carefully measure this distance.

Take from the table on page 56 the azimuth of Polaris corresponding to the latitude of the station and year of observation; find the natural tangent of said azimuth and multiply it by the distance from the peep sight to the rod; the product will express the distance to be laid off from the rod exactly at right angles to the direction already determined (to the west for eastern elongation or to the east for western elongation), to a point, which with the peep sight, will define the direction of the meridian with sufficient accuracy for the needs of local surveyors.

Example: Sept. 10, 1915, in latitude 45° N, longitude 71° W, it is desired to obtain the declination of the needle.

From the table giving times of elongation it is found that Polaris is at eastern elongation on Sept. 1st at 53.2 minutes past 8 p. m.

Correction A is not required in this case.

Correction B, for the 9 days elapsed since Sept. 1st, is 35.3 min., to be subtracted.

Correction C, for 71° longitude, is 16 min., to be subtracted. Correction D, for 45° latitude, is 0.85 min., to be added.

Correction E is 0.2 min., to be added.

8 hrs. 53.2 min. -35.3 min. -16 min. +.85 min. +.2 min. = 8 hrs. 3 min., time of elongation by the watch.

The star having been observed at the time indicated and brought down to the horizon, its azimuth is ascertained from the table of azimuths. For 1915 and latitude 45°, this value is 1° 37.4′ and there is no appreciable correction for apparent place. The meridian then is that much to the west of the line determined. In this case, with the instrument on the azimuth line the needle was allowed to settle and a reading of N 17° 50′ E obtained. 17° 50′ - 1° 37.4′ = 16° 12.6′. 16° 12.6′ is therefore the magnetic declination for the place and time, or 16° 15′ as near as a needle can be read.

In practice corrections D and E may usually be neglected. Using the table for time of elongation with corrections A, B, and C applied to it, the surveyor will ascertain when to be on hand for the observation. Then, watching the star, when satisfied by its motion that it has reached elongation he will bring his instrument down without regard to time. In fact, Polaris traverses less than 4' of azimuth in the hour before and the hour after elongation.

N.

AND 1909 BETWEEN YEAR FOR ANY ELONGATION WHEN POLARIS OF AZIMUTH

2.2.2.4 2.5.2.2 2.5.5.2 14.9 16.5 17.4 18.3 19.2 20.2 21.3 22.4 23.5 100010 31.8 335.2 38.9 1928 328.7.28 19.6 20.6 22.8 23.9 25.1 26.4 27.8 29.1 30.6 0000000 15.3 16.9 17.8 18.6 32.2 33.9 35.6 39.4 39.4 927 - C C C C 4 0 _ 15.6 17.2 19.0 223.0 24.3 24.3 5000 m 32.6 34.3 36.1 39.9 926 टाटांक मं मं 0 600000 000000 16.0 17.6 18.5 19.4 20.4 222.4 23.5 24.7 1925 0,00040 33. 386. 40. 30.52 42. 0 -Survey) 13.0 4.0.8.0.8.0 9.8.0 9.8.0 33.5 35.2 38.8 40.8 12.9 924 26. 29. 30. 31. 0 United States Coast and Geodetic 13.3 16.7 17.5 18.3 19.2 22.1.1 22.2.2 24.3 25.5 28.0 29.4 30.8 32.4 34.0 35.6 37:4 39.3 13.4 1923 0 13.6 14.2 14.9 15.6 16.3 17.0 17.9 18.7 19.6 20.5 10100100 34.4 36.1 37.9 39.8 41.7 1.25.25.25 23.5 32.5 32.5 32.5 32.5 32.5 13. 15.9 15.9 16.6 21.8 22.9 24.0 25.1 26.3 17.4 18.2 19.1 20.9 1000 100 00 CM 00000000 38. 38. 40. 42. 0 -14.3 14.9 15.6 17.0 17.8 18.6 19.4 20.3 40000 27.9 29.1 30.6 33.6 35.3 37.0 40.7 920 222222 From 0 -15.9 18.1 18.9 19.8 20.7 21.6 10766 1919 45. 53.0. 0 18.5 19.3 20.1 22.0 15.0 15.0 17.0 17.7 23.0 24.0 25.1 27.5 28.7 30.0 31.5 34.5 36.1 37.8 39.7 41.6 918 45.7 0 -18.8 119.7 220.5 22.4 22.4 36.6 38.3 40.1 44.1 1917 8.76.65 225.5.4.23 0 Latitude 2827.00 33500 38000 001886 200

The table on the preceding page was computed with mean declination of Polaris for each year. A more accurate result will be had by applying to the tabular values the following correction, which depends on the difference of the mean and the apparent place of the star. The deduced azimuth will in general be correct within 0.3'.

For Middle of	Correction	For Middle of	Correction
January February March April May June	$ \begin{array}{c} -0.5 \\ -0.4 \\ -0.3 \\ -0.0 \\ +0.1 \\ +0.2 \end{array} $	July August September October November December	+0.2 +0.1 -0.1 -0.4 -0.6 -0.8

LOCAL CIVIL (NOT STANDARD) TIME OF THE ELONGATIONS OF POLARIS IN THE YEAR 1915. (COMPUTED FOR LATI-TUDE 40° NORTH AND LONGITUDE 90° OR 6h WEST OF GREENWICH)

(From United States Coast and Geodetic Survey)

Date	Eastern Elongation	Western Elongation
1915	h. m.	h. m.
January 1 January 15 February 15 February 15 March 1 March 15 April 15 May 15 June 1 June 15 July 1 July 15 August 15 September 1 September 15 October 15 November 1 November 15 December 15	0 51.7 P. M. 11 56.4 A. M. 10 49.2 A. M. 9 54.0 A. M. 8 58.7 A. M. 6 56.6 A. M. 6 1.6 A. M. 4 58.7 A. M. 2 57.2 A. M. 2 57.2 A. M. 2 57.2 A. M. 0 59.8 A. M. 0 59.8 P. M. 8 53.2 P. M. 8 53.2 P. M. 6 55.5 P. M. 6 00.6 P. M. 3 58.6 P. M. 3 58.6 P. M.	0 46.0 P. M. 11 46.8 P. M. 10 39.7 P. M. 9 44.4 P. M. 8 49.2 P. M. 7 54.0 P. M. 6 47.1 P. M. 5 52.0 P. M. 4 49.2 P. M. 3 54.2 P. M. 1 52.8 P. M. 1 52.8 P. M. 1 55.4 A. M. 10 48.8 A. M. 9 54.1 A. M. 8 47.5 A. M. 7 52.6 A. M. 6 49.8 A. M. 5 54.9 A. M. 5 54.9 A. M. 2 49.9 A. M.

A. To refer the above tabular quantities to years subsequent to 1915:

For year	1917	subtract	0.7	minute	
	1918	add	0.9	minute	
	1919	add	2.5	minutes	
	1	add	4.0	minutes	up to March 1
	1920 {	add	0.1		on and after March 1
	1921	add		minutes	
	1922	add		minutes	
	1923	add		minutes	
	(add		minutes	up to March 1
	1924 {	add		minutes	on and after March 1
	1925	add		minutes	on and after March 1
			0.0	mnutes	
	1926	add	4.6	minutes	
	1927	add	5.9	minutes	
	1000	add	7.2	minutes	up to March 1
	1928 {	add		minutes	on and after March 1

B. To refer to any calendar day other than the first and fifteenth of each month, subtract the quantities below from the tabular quantity for the preceding date.

Day of Month	Minutes	No. of Days Elapsed
2 or 16	3.9	1
3 or 17	7.8	2
4 or 18	11.8	3
5 or 19	15.7	4
6 or 20	19.6	5
7 or 21	23.5	6
8 or 22	27.4	7
9 or 23	31.4	8
10 or 24	35.3	9
11 or 25	39.2	10
12 or 26	43.1	11
13 or 27	47.0	12
14 or 28	51.0	13
29	54.9	14
30	58.8	15
31	62.7	16

For the tabular year, two eastern elongations occur on January 14, and two western elongations on July 13.

C. To refer the table to standard time: Add to the tabular quantities four minutes for every degree of longitude the place is west of the standard meridian and subtract when the place is east of the standard meridian.

D. To refer to any other than the tabular latitude between the limits of 25° and 50° North: Add to the time of west elongation 0.10 min. for every degree south of 40° and subtract from the time of west elongation 0.16 min. for every degree north of 40°. For eastern elongations subtract 0.10 min. for every degree south of 40°, and add 0.16 min. for every degree north of 40°.

E. To refer to any other than the tabular longitude: Add 0.16 min.for each 15° east of the ninetieth meridian and subtract 0.16 min. for each 15° west of the ninetieth meridian.

The deduced time of elongation will seldom be in error ·

more than 0.3 min.

For Evening Observation. Study of the tables will show that at certain times of the year a choice of methods is offered. Since, however, evening observation is usually most convenient, the following directions have been arranged with that in view. The time limits for these observations, it will be understood, vary somewhat with the latitude.

On the tenth of January observe western elongation at midnight and for each fifteen days thereafter earlier by one hour. This may be done until late March.

From late March to early June, use lower culmination with the help of Delta of Cassiopeia. On April 1st the culmination occurs at 12.37 and after that for each fifteen days earlier by one hour.

From early June to early October use eastern elonga-

tion. On June 15th it occurs at 2 A. M.

From early October to middle January use upper culmination with Zeta of the Great Bear.

SECTION VIII

THE UNITED STATES PUBLIC LAND SURVEYS

In the original States there is a great variety of system, or lack of system, in the division of land for ownership. Land which has ever been a part of the Public Domain of the United States—and that embraces in general the territory north of the Ohio River and from the Mississippi River west to the Pacific coast—has been surveyed, with small exceptions, under a common system, the so-called "System of Rectangular Surveying." An account of this, so far as it concerns the woodsman, follows.

Chapter III of the Public Land Laws contains the following sections:

SEC. 99. The public lands shall be divided by north and south lines run according to the true meridian, and by others crossing them at right angles, so as to form townships of six miles square, unless where the line of an Indian reservation, or of tracts of land heretofore surveyed or patented, or the course of navigable rivers, may render this impracticable; and in that case this rule must be departed from no further than such particular circumstances require.

Second. The corners of the townships must be marked with progressive numbers from the beginning; each distance of a mile between such corners must be also distinctly marked with marks

different from those of the corners.

Third. The township shall be subdivided into sections, containing, as nearly as may be, six hundred and forty acres each, by running through the same, each way, parallel lines at the end of every two miles; and by making a corner on each of such lines at the end of every mile. The sections shall be numbered, respectively, beginning with the number one in the northeast section, and proceeding west and east alternately through the township with progressive numbers till the thirty-six be completed.

Fourth. The deputy surveyors, respectively, shall cause to be marked on a tree near each corner established in the manner described, and within the section, the number of such section and over it the number of the township within which such section

may be.

Fifth. Where the exterior lines of the townships which may be subdivided into sections or half-sections exceed or do not extend six miles, the excess or deficiency shall be specially noted and added to or deducted from the western and northern ranges of sections or half-sections in such townships, according as the error may be in running the lines from east to west, or from north to south; the sections and half-sections bounded on the northern and western lines of such townships shall be sold as containing only the quantity expressed in the returns and plats, respectively, and all others as containing the complete legal quantity.

Sixth. All lines shall be plainly marked upon trees, and measured with chains, containing two perches of sixteen and one-half feet each, subdivided into twenty-five equal links; and the chain shall be adjusted to a standard to be kept for that purpose.

Sec. 100. The boundaries and contents of the several sections, half-sections, and quarter-sections of the public lands shall be as-

certained in conformity with the following principles:

First. All the corners marked in the surveys returned by the surveyor-general shall be established as the proper corners of sections, or subdivisions of sections, which they were intended to designate, and the corners of half and quarter-sections, not marked on the surveys, shall be placed as nearly as possible equidistant

from two corners which stand on the same line.

Second. The boundary lines, actually run and marked in the surveys returned by the surveyor-general, shall be established as the proper boundary lines of the sections or subdivisions for which they were intended, and the length of such lines as returned shall be held and considered as the true length thereof. And the boundary lines which have not been actually run and marked shall be ascertained by running straight lines from the established corners to the opposite corresponding corners; but in those portions of the fractional townships, where no such opposite corresponding corners have been or can be fixed, the boundary lines shall be ascertained by running from the established corners due north and south or east and west lines, as the case may be, to the water-course, Indian boundary line, or other external boundary of such fractional township.

Third. Each section or subdivision of section, the contents whereof have been returned by the surveyor-general, shall be held and considered as containing the exact quantity expressed in such return; and the half-sections and quarter-sections, the contents whereof shall not have been thus returned, shall be held and considered as containing the one-half or the one-fourth part, respectively, of the returned contents of the section of which they

may make part. (Act of Feb. 11, 1805, and R. S., 2396.)

Sec. 101. In every case of the division of a quarter-section the line for the division thereof shall run north and south, and the corners and contents of half-quarter-sections which may thereafter be sold shall be ascertained in the manner and on the principles directed and prescribed by the section preceding. In elaboration of the law are the following rules laid down by the Federal Land Office:

24. Existing law requires that in general the public lands of the United States "shall be divided by north and south lines run according to the true meridian, and by others crossing them at right angles so as to form townships six miles square," and that the corners of the townships thus surveyed "must be marked with progressive numbers from the beginning."

Also, that the townships shall be subdivided into thirty-six sections, each of which shall contain 640 acres, as nearly as may be, by a system of two sets of parallel lines, one governed by true meridians and the other by parallels of latitude, the latter inter-

secting the former at right angles, at intervals of a mile.

25. In the execution of the public surveys under existing law, it is apparent that the requirements that the lines of survey shall conform to true meridians, and that the townships shall be six miles square, taken together, involve a mathematical impossibility due to the convergency of the meridians.

Therefore, to conform the meridional township lines to the true meridians produces townships of a trapezoidal form which do not contain the precise area of 23,040 acres required by law, and which discrepancy increases with the increase in the convergency of the meridians as the surveys attain the higher latitudes.

26. In view of these facts, and under the provisions of Section 2 of the Act of May 18, 1796, that sections of a mile square shall contain 640 acres, as nearly as may be, and also under those of Section 3 of the Act of May 10, 1800, that "in all cases where the exterior lines of the townships, thus to be subdivided into sections and half-sections, shall exceed, or shall not extend six miles, the excess or deficiency shall be specially noted, and added to or deducted from the western or northern ranges of sections or halfsections in such township, according as the error may be in running lines from east to west, or from south to north; the sections and half-sections bounded on the northern and western lines of such townships shall be sold as containing only the quantity expressed in the returns and plats, respectively, and all others as containing the complete legal quantity," the public lands of the United States shall be surveyed under the methods of the system of rectangular surveying, which harmonizes the incompatibilities of the requirements of law and practice, as follows:

First. The establishment of a principal meridian conforming to the true meridian, and, at right angles to it, a base line conform-

ing to a parallel of latitude.

Second. The establishment of standard parallels conforming to parallels of latitude, initiated from the principal meridian at intervals of 24 miles and extended east and west of the same.

Third. The establishment of guide meridians conforming to true meridians, initiated upon the base line and successive standard

parallels at intervals of twenty-four miles, resulting in tracts of land twenty-four miles square, as nearly as may be, which shall be subsequently divided into tracts of land six miles square by two sets of lines, one conforming to true meridians, crossed by others conforming to parallels of latitude at intervals of six miles, containing 23,040 acres, as nearly as may be, and designated townships.

Such townships shall be subdivided into thirty-six tracts, called sections, each of which shall contain 640 acres, as nearly as may be, by two sets of parallel lines, one set parallel to a true meridian and the other conforming to parallels of latitude, mutually intersecting at intervals of one mile and at right angles, as nearly as

may be.

27. Any series of contiguous townships or sections situated north and south of each other constitutes a RANGE, while such a series situated in an east and west direction constitutes a TIER.

28. By the terms of the original law and by general practice, section lines were surveyed from south to north and from east to west, in order to uniformly place excess or deficiency of measurement on the north and west sides of the townships. But under modern conditions many cases arise in which a departure from this method is necessary. Where the west or the north boundary is sufficiently correct as to course, to serve as a basis for rectangular subdivision, and the opposite line is defective, the section lines should be run by a reversed method.

For convenience the well-surveyed lines on which subdivisions are to be based will be called governing boundaries of the

township.

29. The tiers of townships will be numbered, to the north or south commencing with No. 1, at the base line; and the ranges of the townships, to the east or west, beginning with No. 1, at the

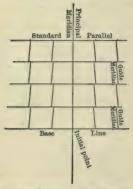
principal meridian of the system.

30. The thirty-six sections into which a township is subdivided are numbered, commencing with No. 1 at the north-east angle of the township, and proceeding west to number six, and thence proceeding east to number twelve, and so on, alternately, to number thirty-six in the southeast angle. In all cases of surveys of fractional townships, the sections will bear the same numbers they would have if the township was full; and where doubt arises as to which section numbers should be omitted, the proper section numbers will be used on the side or sides which are governing boundaries, leaving any deficiency to fall on the opposite sides.

31. Standard parallels (formerly called correction lines) shall be established at intervals of twenty-four miles, north and south of the base line, and guide meridians at intervals of twenty-four miles, east and west of the principal meridian; thus confining the errors resulting from convergence of meridians and inaccuracies in meas-

urement within comparatively small areas.

In pursuit of this system, during the course of the public land surveys twenty-four initial points have been established, a principal meridian has been run due north and south from each of these, and a base line east and west. Each twenty-four miles north and south of the initial point standard parallels or correction lines have been started on which, as they were run east and west, marks have been left each six miles for the starting of township lines. These are run due north to the next standard parallel; each fourth one being run first and



1	St	andard	Parall	el I	L
Principal Meridian	First Range	Second Range	Third Range	Fourth Range	Guide Meridian
	Ste	indard	Parafle	ol .	-

FIRST SUBDIVISION OF LAND

Division into Townships

most accurately as a guide meridian. On the north and south lines township corners are fixed each six miles by measurement, and each pair of corners is later connected. A township corner is common to four townships except on a standard parallel. There, owing to convergence of meridians, the corners of the townships north are farther from the principal meridian than those of the townships south; farther east or west, as the case may be. The ranges of townships connected with any given initial point are numbered east and west from the principal meridian, and the townships themselves are numbered north and south from the base line. Thus the sixth township north of a base line in the fourth range east of a principal meridian is designated as township 6 north, range 4 east. Each township contains

thirty-six square miles or 23,040 acres, neglecting the narrowing effect of the convergence of the meridians. These relations are indicated clearly in the diagrams.

As the township lines are run, corner marks are left each mile, and the township is divided into thirty-six sections by beginning on the south side at each mile mark and running north, marking each mile or section corner, also each half mile or quarter-section corner. At the north end these lines are made to close on the mile marks left in surveying the north line of the township, with the exception of those on a standard parallel. Here the section lines are run straight out to the parallel, which thus serves as a "correction-line" for the sections as well as for the townships.

			I	N						
	6	5	4	3	2	1				
	7	8 9		10	11	12				
317	18	17	16	15	14	13	-			
W	19	20	21	22	23	24	E			
	30	29	28	27	26	25				
1	31	32	35	36						
	S									

SECTIONS	IN	A	To	WN	SHIP

N. W. 1/4		N. E. 14	
W. 1/2 of 8.W.	E. 1/2 of S.W.	N.W. ¼ of S.E. ¼	40 acres
34 80 acres	1/4 80 acres	40 acres	8.E. ¼ of 8.E. ¼

SUBDIVISION OF A SECTION

The east and west section lines are run between corresponding corners on the north and south lines, always marking the half-mile or quarter-section point. The effect on area of convergence of meridians is localized in the case of sections, in the first place by chaining the latitudinal township lines always from the east end, thus confining any deficiency of width to the westerly board of sections; in the second place by running the north and south lines not due north exactly, but with a westerly bearing sufficient at one, two, three, four, and five miles from the east line to keep them at equal distances apart throughout their length. Short area is thus confined to

the westerly board of sections in each township when surveys are accurately made. For the same purpose, reduction in the number of irregular units, quarter corners for the north and west tiers of sections are placed exactly forty chains from the interior corners, not at the middle

point of the section lines.

The Land Office instructions to surveyors contain several articles on the marking of lines, of which those of interest to the woodsman are quoted on page 24 of this work. Instructions for establishing corners and erecting monuments are also given, but are far too elaborate to be here quoted in full. Corner monuments consist of an object marking the corner itself and its accessories. They are to be set up at the intersection of all the lines noted in the instructions quoted above and at some other points to be mentioned hereafter. Several approved forms of corner monuments are described below. Any one may be used for a township, a section, or a quarter-section corner, the marks upon it indicating what the corner is.

- 1. Stone with pits and mound of earth.
- 2. Stone with mound of stone.
- 3. Stone with bearing trees.
- 4. Post with pits and mound of earth.
- 5. Post with bearing trees.
- Mound of earth, with marked stone or charcoal deposited inside, and stake in pit.
 - 7. Tree with pit and mound of stone.
 - 8. Tree with bearing trees.

Posts of wood and stone and bearing trees have been employed largely as corner monuments in timbered country. The post is set not to exceed one foot out of the ground. At a standard, closing, or quarter corner it is set facing cardinal directions, diagonally at a corner common to four townships or sections. Plain figures and initial letters inscribed on the faces give the location, and this in the case of section corners is also indicated by notches cut in the edges or by grooves on faces. These notches, on account of their durability, are of much service in identi-

fication of section corners. They are placed on the south and east angles of the posts, one for each mile to the township boundary in the given direction. Quarter corners are not notched; township corners are cut six times on each face or angle.

Equally serviceable are the bearing trees. These are blazed rather close to the ground so that the stump can be identified if the tree is cut down. The blazes face the corner, and that on each tree at township or section corners is plainly scribed with the township number and range and that of the section in which it stands. Thus, T 10 S R 6 E S 24 B T (B T for bearing tree).

There are several exceptions to the system of rectangular surveying and the regular scheme of monuments resulting therefrom, which it is necessary for the woodsman to understand.

1. Township and Section Corners on Standard Parallels.

It will be noted after careful reading of the above that township or section corners are common to four townships or sections, with the exception of those on the standard parallels which are four townships apart. Here the corners for the townships north of the parallel are not the same as for those south, but are further from the principal mcridian. The former are called "standard corners" and are marked S C in addition to other marks placed on them for their identification. In a similar way the corners relating to land subdivisions lying south of the parallel are marked C C, "closing corner." This last term is also applied in other connections, as when a rectangular survey closes on the boundary of a state, a reservation, or a previous land claim, while occasions for its application have often been found in connection with errors or departures from instructions in the system of surveying.

2. Meander Lines and Corners.

Ownership of considerable streams or lakes, with the exception of certain "riparian rights," is not conveyed with a land title, the legal limit being high-water mark, or the line at which continuous vegetation ends and the sandy

or muddy shore begins. This line is surveyed in connection with a United States land survey, the process being called "meandering."

At every point where a standard, township, or section line intersects the bank of a navigable stream or other meanderable body of water, corners are established at the time of running these lines. These are called "meander corners." They are always marked M C in addition to any other marks left for their identification.

In the same way, when a line subdividing a section runs into a considerable body of water, a "special meander corner" is established and marked in the same way.

3. Witness Corners and Witness Points.

A key to the location and meaning of these will be found in the following sections from the "Instructions."

49. Under circumstances where the survey of a township or section line is obstructed by an impassable obstacle, such as a pond, swamp, or marsh (not meanderable), the line will be prolonged across such obstruction by making the necessary right-angle offsets; or, if such proceeding be impracticable, a traverse line will be run, or some proper trigonometrical operation employed to locate the line on the opposite side of the obstruction; and in case the line, either meridional or latitudinal, thus regained, is recovered beyond the intervening obstacle, said line will be surveyed back to the margin of the obstruction.

50. As a guide in alignment and measurement, at each point where the line intersects the margin of an obstacle a witness point will be established, except when such point is less than twenty chains distant from the true point for a legal corner which falls in the obstruction, in which case a witness corner will be established

at the intersection.

51. In a case where all the points of intersection with the obstacle to measurement fall more than twenty chains from the proper place for a legal corner in the obstruction, and a witness corner can be placed on the offset line within twenty chains of the inaccessible corner point, such witness corner will be established.

97. The point for a corner falling on a railroad, street, or wagon road, will be perpetuated by a marked stone (charred stake or quart of charcoal), deposited twenty-four inches in the ground, and witnessed by two witness corners, one of which will be established.

lished on each limiting line of the highway.

In case the point for any regular corner falls at the intersection of two or more streets or roads, it will be perpetuated by a marked stone (charred stake or quart of charcoal), deposited twenty-four inches in the ground, and witnessed by two witness corners established on opposite sides of the corner point, and at the mutual intersections of the lines limiting the roads or streets, as the case

may be.

94. When the true point for any corner described in these instructions falls where prevailing conditions would insure its destruction by natural causes, a witness corner will be established in a secure position, on a surveyed line if possible, and within twenty chains of the corner point thus witnessed.

95. A witness corner will bear the same marks that would be placed upon the corner for which it is a witness, and in addition, will have the letters W C (for witness corner) conspicuously displayed above the regular markings on the NE. face when witnessing in township or section corner; such witness corners will be established, in all other respects, like a regular corner, marking bearing trees with the proper numbers for the sections in which they stand.

W C will also be cut into the wood of each bearing tree above

the other markings.

98. Witness points will be perpetuated by corners similar to those described for quarter-section corners, with the marking WP (for witness point), in place of 1, or 1 S, as the case may be.

If bearing trees are available as accessories to witness points,

each tree will be marked W P B T.

4. Fractional Sections, Lots, etc.

A section or quarter-section made of less than full size by water is called "fractional," and in some cases is subdivided according to special rules laid down by the Land Office. The sections on the westerly board of a township, into which, under the plan of survey, shrinkage of area due to convergence of township lines toward the north is crowded. are called fractional as well. Within these sections again, the westerly quarters and forties will be fractional for the same reason. The final subdivisions of irregular area the system is followed next the north as well as the west line of the townships - are called "lots." In a regular township there are four to each section, numbered from 1 to 4 for each, beginning with the east or north, with seven lots for Section 6. In timbered country, however, they are seldom run out on the ground.

While the above are usual features of the public land surveys, numerous exceptions were made, as for instance in case of a defective east or south boundary in a township.

when subdivision was begun from the opposite side. Somewhat different rules also were in force during the very early surveys. Then in addition irregularities due to the errors of surveying, and these sometimes of an extreme nature, are sometimes found.

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PART II. FOREST MAPS

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PART II. FOREST MAPS

SECTION I

THE TRANSIT

The transit in general engineering work is the most useful and most frequently employed of surveying instruments. It is commonly used to measure horizontal and vertical angles, but, having a magnetic needle, it may be used to take bearings, and, when provided with stadia wires, to measure distances. It may also be used as a levelling instrument. A cut of a transit is shown herewith, also a sectional view through the axis of the same instrument.

The essential parts of an engineer's transit are described below. The telescope is attached by means of a horizontal axis and standards to the upper of two circular plates. The two plates move freely on one another, the lower being graduated, while the upper has a vernier which allows readings to be made with accuracy. A compass circle is also attached to the upper plate. A clamp fixes the upper to the lower plate, and a tangent screw secures a slow adjusting movement between the two. A similar arrangement is placed between the lower plate and the head of the instrument.

The whole instrument is supported on a **tripod**; **levelling screws** serve with the aid of **cross levels** to fix the plates in a horizontal position; and a finely turned **spindle and socket** arrangement guides the plates in their movement on one another. By means of a **plumb line** attached to the lower end of the spindle the instrument may be set with its axis

exactly over any desired point.

1. Adjustments of the Transit

The object of these adjustments is to cause (1) the instrument to revolve in a horizontal plane; (2) the line of sight to generate a vertical plane when the telescope is

revolved on its axis; (3) the axis of the telescope bubble to be parallel to the line of sight, thus enabling the instrument to be used as a level; (4) the vernier on the vertical

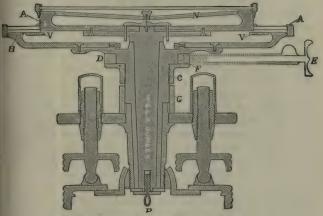


THE TRANSIT

circle to be so adjusted as to give the true altitude of the line of sight. These results may be secured as follows:

a. To adjust the plate levels so that each is in a plane

perpendicular to the vertical axis of the instrument. Set up the transit and bring the bubbles to the center of their respective tubes. Turn the plate 180° about its vertical axis, and see if the bubbles remain in the center. If they move from the center, turn the capstan-headed screws on the bubble tube until the bubble moves half-way back to the center, or as nearly so as this can be estimated. Each bubble must be adjusted independently. The adjustment should be tested again by relevelling and reversing as before, and the process continued until the bubbles remain in the center when reversed. When both levels are adjusted, the bubbles should remain in the center during the entire revolution about the vertical axis.



CROSS-SECTION OF THE TRANSIT HEAD

b. To make the line of sight perpendicular to the horizontal axis so that the telescope when revolved will generate a plane. To do this choose open and nearly level ground. Set up the transit carefully over a point A, sight accurately at a point B at about the same level and 200 or 300 feet away, and clamp both plates. Revolve the telescope and set C in line with the vertical cross-hair at about the same distance and elevation. B, A, and C should then be in a straight line. To test this, turn the instrument

about the vertical axis until B is again sighted. Clamp the plate, revolve the telescope, and observe if point C is in line. If not, set a third point D in the new line. Then, to adjust, the cross-hair ring must be moved until the vertical hair appears to have moved to the point E, one-fourth the distance from D toward C, since, in this case, a double reversal has been made.

The cross-hair ring is moved by loosening one of the screws which hold it in the telescope tube and tightening the opposite screw. The process of reversal should be repeated until no further adjustment is required. When finally adjusted, the screws should hold the ring firmly but without straining it.

c. To make the horizontal axis of the telescope perpendicular to the vertical axis of the instrument, so that the telescope in its revolution will generate a vertical plane. Set up the instrument and level it carefully. Suspend a fine, smooth plumb line twenty or thirty feet long some twenty feet away from the instrument with a weight on the lower end hanging freely in a pail of water. Set the line of sight carefully on the cord at its upper end. Clamp both plates and bring the telescope down until it reads on the lower end of the cord. If the line of sight does not cut the cord, raise or lower the adjustable end of the horizontal axis until the line of sight does revolve in a vertical plane. Constant attention must be given to the plate bubbles to see that they do not indicate an inclined vertical axis.

If more convenient two points in a vertical line may be used, as points on a building. Set on the top point and turn down to the bottom one, marking it carefully. Revolve both plate and telescope 180° and set again on the bottom point. Raise the telescope again and read on the top point. The second pointing at the top point should correspond with the first. If it does not, adjust as above for half the difference.

d. To make the telescope bubble parallel to the line of sight. This adjustment is performed in the same way as for a level, as explained on pages 89 and 90.

e. To make the vernier of the vertical circle read zero

when the line of sight is horizontal. Having made the axis of the telescope bubble parallel to the line of sight, bring the bubble into the center of the tube and adjust the vernier of the vertical circle until it reads zero on the limb. If the vernier is not adjustable, the reading in this position is its index error, to be applied to all readings.

2. CARE OF THE TRANSIT

The transit should be protected from wet and dust as much as possible, a waterproof bag to cover it being useful for that purpose. The tripod legs should move freely, but not too freely; there should be no lost motion about their shoes or elsewhere. Dust or water should be removed from the glasses by a camel's hair brush or the gentle use of a clean handkerchief; grease may be removed by alcohol. Care should be taken not to strain the parts of the instrument by too great pressure on the screws when using or adjusting it. Before the transit is picked up, the levelling screws should be brought approximately to their mid position, the telescope should be turned vertically and lightly clamped, and the clamp of the lower plate should be loosened. Then, if the instrument strikes anything while being carried from point to point, some part will move easily and severe shock will be avoided.

3. STADIA MEASUREMENT

Measurement of distance by stadia is secured by simply sighting with a transit at a graduated rod held on any desired point and noting the space on the rod included between two special cross-hairs set in the focus of the instrument. This is a very rapid method of measurement, being especially handy and effective over broken land; it gives a degree of accuracy sufficient for very many purposes; it allows the computation of the difference in elevation between two points. Thus for many purposes it is the most effective method of survey, and it is coming into general use.

The Instrument. A transit intended for stadia work is

provided with two additional horizontal hairs, usually fastened to the same diaphragm as the ordinary cross-hairs, and placed at a known distance apart. The space between these two extra hairs is preferably fixed, but in some transits the diaphragm is so arranged that it can be adjusted. The instrument must also be provided with a level on the telescope and a circle or arc for measuring vertical angles, since the telescope is seldom level when measurements are taken.

Stadia rods are usually 10 or 12 feet long. They are plainly painted in such a design as to be read at long distances. Engineers generally use rods graduated to feet and tenths, the hairs cutting off one foot on the rod at a distance of 100 feet. Hundredths of a foot are generally estimated. For use in connection with a land survey it may be more convenient to graduate the rod or adjust the hairs so that one unit will be cut off at a distance of 66 feet or one chain.

Inclined Sights. The distance between instrument and rod is measured directly if the sight is taken horizontally, and a vertical angle between them of 5° or less does not so affect the sight as to matter particularly in many kinds of work. If, however, a sight of greater inclination is taken, a reading is obtained that represents a greater distance than the horizontal one between instrument and rod. If for an inclined reading the rod is also inclined, so as to be perpendicular to the line of sight, the reading represents the inclined distance, and the horizontal distance is the cosine of the angle of inclination multiplied by the inclined distance. Similarly, the difference in elevation is the inclined distance multiplied by the sine of the angle.

It is usual, however, and better, to hold the rod plumb, and here the computation of horizontal and vertical elements is not so simple. Tables, however, have been computed which give these elements, horizontal distance and difference of elevation, directly. A compact stadia table will be found on page 211 of this work and an example showing the method of its use is given on page 80.

What has been written above needs, however, one qualification. Stadia wires to read truly at all distances

must cut off the unit distance on the rod not at a distance of 100 or of 66 feet, but at a greater distance equal to the distance from the center of the instrument to the objective lens + the distance from the cross-wires to the same lens when focused on a distant object. This correction, (f+c) as it is called, is about 1 foot in common transits.

In testing the instrument on measured bases, therefore, these should be measured out from the plumb line or center of instrument to the required distance + the constant above described, and for accurate determination of distance the constant should be added to the distance observed. In working out inclined sights from the table this constant may be added to the rod reading before the reductions for horizontal distance and elevation are made.

In the practice of woodsmen, however, work will generally be accurate enough if this constant is neglected, all the more so since this error tends to be compensated by that arising from neglect of the small vertical angles noted above. There are, indeed, a few transits so constructed that no such constant correction as that above stated has to be considered.

Accuracy. The accuracy of stadia measurement depends largely on the state of the atmosphere. If that is hazy, or unsteady from the effects of heat, long shots cannot be taken and measurements on shorter distances cannot be accurately obtained. There is furthermore the possibility that the line of sight by the lower hair when passing over very hot ground may be refracted more than the other and thereby give too small a reading. Otherwise than here and above stated the only sources of inaccuracy are due to errors in rod readings which for small errors are as apt to be + as — and so mainly balance one another. Thus while on single shots stadia measurement may be appreciably inaccurate, the relative error decreases with the length of the line run.

In general it may be said that stadia measurement gives satisfactory results for very many purposes, and that it has great advantages in the way of rapidity and cheapness. With good instruments and clear air it can be employed

on distances from one quarter to one third of a mile, giving results which are accurate to within a few feet.

Example and Reduction of Readings. . 1' on rod cut off at distance of 100'. In computation, correction made for 1' instrumental constant. True horizontal distance and difference of elevation between points both worked out. Height of instrument over station obtained at each setting and center hair for vertical angle read at same height on rod.

Observed		Computed			
Bearing	Rod Reading	Vert. Angle	Distance	Diff. Elev.	Elev.
N. 5° E. N. 5° E. N. 5° E. N. 5° E.	2.00′ 1.80′ 1.05′ 1.50′	+ 1° 30′ + 4° 10′ + 8° - 30′	200,86′ 179.84′ 103.94′ 150.98′	+ 5.27' + 13.12' + 14.61' - 1.31'	5.27' 18.39' 33.00' 31.69'
			635.62′		31.69'

Computation. First shot, with v. a. of 1° 30′, rod reading 2.00′. Add .01′ for instrument constant, making 2.01′, for corrected rod reading. From table the horizontal distance for 1′ rod reading is found to be 99.93′ the difference of elevation 2.62′. For 2.01′ rod reading the elements are 99.93×2.01 and 2.62×2.01 or 200.86′ and 5.27′, as above.

Second shot, 1.80 + .01, = 1.81, corrected rod reading.

For v. a. 4° 10' and rod reading 1', horizontal distance 99.47 and diff. elev. 7.25 are found in the tables. 99.47 \times 1.81 and 7.25 \times 1.81 = 179.84 and 13.12.

Similarly for succeeding shots.

4. Uses of the Transit

To Take the Bearing of a Line. Set up over the first point, level the instrument, free the needle, and turn the telescope toward the other point. Read the bearing in the same way as with a compass.

When set up on the forward one of two points, exactly the same bearing may be read as if the instrument were set up on the rear point, if the telescope is revolved before

the pointing is made and the bearing taken.

To Measure a Horizontal Angle. Set up the instrument, center it by means of the plumb line over the vertex of the angle required, set the zeros of the two plates together, clamp them, and turn the telescope toward one of the points, making the final adjustment by means of the lower tangent screw. Then loosen the upper clamp, turn toward the other point, clamp again, and set finally by the upper tangent screw. Read the angle turned by means of the vernier. If the instrument has two verniers, both may be read and the average taken.

Measurement by Repetition. A more accurate measurement may be had by turning the angle several times, taking the final reading, and dividing it by the number of times the angle has been turned. If the final reading is about 360°, possible errors in the graduation of the instrument will have no effect on the angle read, and if later the telescope is inverted and the angle turned in the opposite direction from the first turning, other sources of error will have been eliminated. The exact program for an observation of this kind is as follows:

a. Telescope direct.1

- 1. Clamp plates on zeros, and set on left station. Clamp below.
 - 2. Unclamp above and set on right station.
 - 3. Unclamp below and set on left station.
 - 4. Unclamp above and set on right station.

Continue until the desired number of turnings have been made, when the final reading may be taken.

- b. Telescope inverted.
- 1. Clamp plates on zeros and set on right station. Clamp below.
 - 2. Unclamp above and set on left station.
 - 3. Unclamp below and set on right station.
 - 4. Unclamp above and set on left station.

Continue for the same number of turnings as before

¹ That is, with the level tube underneath the telescope.

and read the final angle. If the instrument has two verniers both should be read. It is customary to record the reading after turning the angle once, as a check on the repeated reading. The true reading is the average of the values obtained for the angle with telescope direct and telescope inverted.

To Prolong a Straight Line. Set up the instrument over the forward point and sight the telescope on the rear one. Set both clamps, revolve the telescope on its axis, and set a

new point as far ahead as convenient or desired.

More Accurately. With the telescope in its natural position, turn on the rear point, clamp, revolve the telescope as above, and set a stake and tack at the forward pointing. Then, leaving the telescope inverted as it is, swing the plates around half a circle and set on the rear point again. Revolve the telescope, and again sight at the forward point. If the two pointings ahead do not coincide, set a tack half-way between the two and it will be in the line desired.

To Measure a Vertical Angle. For this purpose the vertical circle must be adjusted so as to read zero when the telescope is level, or, if it is not adjustable, the error of its reading must be obtained, as explained under adjustments of the transit. Then the angle of elevation or depression to any point may be measured by sighting the telescope upon it and reading the vertical angle by means of the vertical circle and its vernier.

To Survey a Piece of Ground with the Transit. Set up on the initial point of the survey, turn to the second point, read the bearing of the line, recording it for a check on later angles, and measure the line. Set up over the second point, set the two plates to read zero, and clamp them together; then turn the telescope at a rod held vertical and carefully centered over the first point. Set the lower clamp and loosen the upper one, swing the telescope with the upper plate around until the third point is sighted, and read the angle so turned. Read the bearing for a check, and measure the line. Proceed in this way until all the angles have been turned and all the sides measured. Interior angles should always be read, though

they may be more than 180°. The magnetic bearings may be used to figure out the angles as a check on measurement; they also help to locate an error if one exists, but a more accurate check is the sum of all the angles which should equal twice as many right angles less four as the figure has sides.

Computed bearings are worked out by applying the angle measurements to the bearing of the first line. Computed, not observed, bearings should be used, for plotting or for computing traverse. Notes may be kept as follows:

Notes of Survey of Field					
Sta.	Int. Angle	Observed Bearing	Computed Bearing	Distance	
0		N 81°E	N81°E	518.63 ft.	
1	269° 19′	N8° 15'W	N8°19'W	48.19 "	
2	95° 52′	N75°45'E	N75°49'E	300.53"	
3	85° 12'	59° 30′E	59°23'E	183.60 "	
4	91°28′	579°15'W	S79°9'W	819.96"	
5	86°56′	N7°45'W	N7°47'W	134.85 "	
0	91°13′	N8IE			



Instead of interior angles, deflection angles may be read, a deflection angle being the angle which any course makes with the prolongation of the one preceding. To get this, after the instrument has been turned on the rear point, revolve the telescope on its axis and turn to the point ahead. The deflection must be recorded as right or left,

along with the amount of the deflection. Notes may be kept as follows:

Instr.	Deflection Angle	Observed Bearing	Computed Bearing	Distance
0		N. 81° E.	N. 81° E.	518.63 ft.
1	89° 19′ L.	N. 8° 15′ W.	N. 8° 19′ W.	48.19 ft.
2	84° 8′ R.	N. 75° 45′ E.	N. 75° 49′ E.	300.53 ft.

In any case, a sketch kept on the right-hand page of the note book will be an aid to clearness. The whole survey, indeed, may be recorded in that form.

A Survey or Traverse by Azimuths. Azimuth is the angle which a line forms with the meridian, or with any other line which is selected as a basis. It is similar to bearing, but is measured in one direction, commonly from south around through west, north, and east up to 360°, and transits are commonly graduated so as to be read directly in this way. The method of work is as follows:

Set up on the initial point of the survey, set the zeros of the two plates together, clamp them, and turn until the telescope points south, as shown by the needle. Clamp below, loosen above, and point the telescope at the second point of the survey, recording the angular reading, and the bearing for a check upon it. Clamp above and loosen below. Measure the line.

Set up over the second point, revolve the telescope, and turn on the first point, making sure not to start the upper clamp at any time during the process. Clamp below; then revolve the telescope into its natural position, loosen above, and turn on the third point of the survey. The azimuth of this line may now be read off the plate and bearing by the needle for a check. Measure the second line. Proceed in this way until the survey is completed. If the survey is a closed one, when the transit is finally set up again at the initial point, the azimuth of the first line should be the same as it was at the beginning.

Notes	man	ha	kont	00	fol	Oure.
Notes	may	be	кері	as	IOL	lows:

Azimuth	Bearing	Distance
162° 12′ 30″	N. 17° 45′ W.	6.40 ch.
223° 30′	N. 43° 30′ E.	7.25 ch.
280° 25′	S. 79° 30′ E.	4.92 ch.
5° 43′ 30″	S. 5° 45′ W.	6.10 ch.
	162° 12′ 30″ 223° 30′ 280° 25′	162° 12′ 30″ N. 17° 45′ W. 223° 30′ N. 43° 30′ E. 280° 25′ S. 79° 30′ E.

Caution. In transit surveying, where angles are read, each line is referred to the one that goes before, and in consequence an error in reading one angle is perpetuated throughout the survey. Further than that, some of the errors arising from lack of adjustment of the instrument are multiplying errors, increasing as the work proceeds, and unless every precaution is taken they may, though individually small, mount up to a very considerable size in the course of a survey.

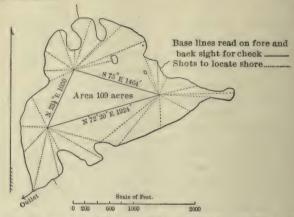
With compass surveying, on the other hand, though bearings cannot be read with great exactness and single angles are not so accurately determined as with the transit, yet errors have not the same opportunity to accumulate because each course in the survey is referred anew to the meridian.

The man who is not in constant practice, therefore, will be likely to find that he attains better results with the needle than by turning angles, and in that case, unless the telescope is wanted for stadia measurements, the compass is the instrument to use. The matter of cost is, in woods conditions, strongly on the side of the compass, for it is usually expensive to cut away for the long, clear sights requisite to the running of a reliable transit line.

Typical examples of stadia surveys such as the woodsman may have occasion to perform are as follows:

Stadia Survey of a Pond as carried out on the ice. The needle was relied on in this case, but it will readily be understood that angles might be read instead of bearings and the survey so rendered independent of the magnetic needle. If the survey were to be made in summer, points

and islands would have to be used for observing stations, and it might be necessary to do a good deal of traversing of the shore.



Stadia Survey of Road. 1 foot on rod cut off at distance of one chain. Instrument set up at alternate stations only, except where a check on local attraction of the needle is desired. Vertical angles of less than 5° neglected as having no material effect on horizontal distance.

				Jan. 10, 1907 W.W.Cook, Inst. H.O.Colton Rod.	
Line as read	Line Bearing Reading V.A. Dist				Remarks
0-1	NOOE	2.30		2.30ch	
1-0	N805'E	2.30	Head fo	edle.	56.5 chains So. on it as Shown by
1-2	578°30'E	6.16		6.16 ch	Survey of boundary
3-2	N89 15 E	5.25		5.25 11	These courses along South slope
3-4	N41°E	1.10	+5%	1.09 "	onto shoulder coming fron NE
5-4	N25 15 E	6.52	-2°	6.52 "	
5-6	NIOE	5.30		5.30 "	
7-6	N	6.10		6.10 "	
7-8	N10°15'E	8.15		8.15"	2.45 on this course brook crosses
9-8	N35 30 E	3.50		3.50"	to East.
9-10	N10°30'W	9.50		9.50 "	
10-9	N1030W				Test of needle
1	•	1	1		

5. STIMMARY

The transit of late years has gained a considerable field of use among working foresters for map making and other purposes. The instrument has for woods work great advantages over the plane table in that it is more portable, is less liable to accident, and is not so easily driven off the field by bad weather.

The uses for it, present and prospective, are as follows:

(1) It is the instrument for land surveys when great accuracy is required or the needle is seriously disturbed. When it is so employed the stadia wires in some cases afford the most effective means of distance measurement.

(2) It may be used as a level in dam and road building

or for topographic purposes.

(3) Two men using transit and stadia can traverse roads, streams, or lake shores very rapidly, using the needle and, except for a check on local attraction, setting up the instrument on alternate points only.

(4) Uses (2) and (3) may be combined, allowing a traverse and a profile to be run at the same time by the

same party.

(5) A skeleton of accurately run lines, embracing both horizontal and vertical angles, may be made the basis of topographic surveys, and the method is in fact highly serviceable in some kinds of country.

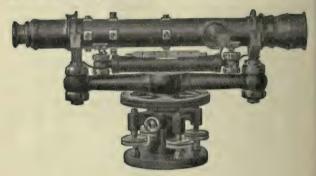
(6) With its various capacities again utilized, the transit is sometimes employed to work out the detail

of small tracts requiring great accuracy.

SECTION II

THE LEVEL

The engineer's level consists of a telescopic line of sight joined to a spirit level, the whole properly supported, and revolving on a vertical axis. The outside parts of the frame which support the telescope are called the wyes, and the corresponding bearings on the telescope tube, the pivot rings. The telescope can be lifted out of the wyes by lifting up the clips over the rings. The attached bubble enables the line of sight in the telescope to be brought into a horizontal position.



THE LEVEL

1. Adjustments of the Level

(a.) Make the line of sight coincide with the axis of the pivot rings. Pull out the pins which hold the clips on the telescope and turn the clips back so that the telescope is free to turn in the wyes. Sight the intersection of the cross-hairs at some well-defined point. Then rotate the telescope 180° in the wyes, so that the bubble tube is above the telescope. The intersection of the cross-hairs should still be on the point. If not, move the horizontal cross-hair half-way back to its first position by means of the upper and lower adjusting screws of the cross-hair ring. Then move the vertical cross-hair half-way back to its first position by the other pair of screws. Repeat the test until the adjustment is perfect.

(b.) Place the line of sight and the bubble in the same vertical plane. Bring the bubble to the center of the tube. Revolve the telescope a few degrees in the wyes and note the action of the bubble. If it runs to one end, bring the tube under the axis of the telescope by means of the lateral

adjusting screws. When the two axes are in the same plane, the bubble will remain in the center while the telescope is revolving.

(c.) Make the level tube parallel to the line of sight. This may be done in two ways. The first or indirect

method is as follows:

Clamp the instrument over a pair of levelling screws; then bring the bubble to the center of the tube, lift the telescope out of the wyes, turn it end for end, and set it down in the wyes again. The eye end now is where the objective was originally. This operation must be performed with the greatest care, as the slightest jar of the instrument will vitiate the result. If the bubble returns to the center of the tube the axis of the tube is in the correct position. If it does not return to the center, the end of the tube provided with the vertical adjustment should be moved until the bubble moves half-way back to the center. This test must be repeated to make sure that the movement is due to defective adjustment and not to the jarring of the instrument.

For the second, the direct or peg adjustment, select the points A and B, say 200 feet apart. The distance need not be measured. Set up the level close to A so that when the rod is held upon it the eyepiece of the telescope will swing within about half an inch of its face. Bring the bubble to the middle of the tube and looking through the telescope wrong end to, put a pencil mark on the rod at the center of the small field of view. Note the rod reading thus obtained. Then turn the telescope toward B and take a rod reading in the usual way, making sure that the bubble is in the middle of the tube. The difference between these two rod readings is the difference in elevation of the two points + or - the error of adjustment. Next take the level to B and repeat the above operation. The result here gained is the difference in elevation - or + the error of adjustment, and the mean of the two results is the difference of elevation between points A and B. Now, knowing the difference between A and B and the height of the instrument above B, the rod reading at Λ which will bring the target on the same level as the instrument may be computed. With the horizontal cross-hair on the target, the adjustable end of the level tube is raised or lowered by means of the adjusting screws until the bubble is in the middle. The adjustment should then be correct, but it will be well to test it.

EXAMPLE

Instrument at A

Rod reading on A = 4.062Rod reading on B Diff. elev. of A and B = 5.129= 1.067

Instrument at B

Rod reading on B = 5.076

Rod reading on A=4.127Diff. elev. of B and A=0.949Mean of the two results = 1.067 + 0.949 = 1.008, true diff. in elev.

Instrument is now 5.076 above B.

Rod reading at A should be 5.076 - 1.008 = 4.068 to give a level sight.

This method of adjustment may be used for the transit with this difference — that instead of adjusting the level tube to the line of sight, the level tube is first made horizontal and then the line of sight is made parallel with it by adjusting the cross-hair. The same is true of a dumpylevel.

(d.) Make the axis of the level tube perpendicular to the vertical axis of the instrument.

Bring the two clips down over the telescope and fasten them. Level the instrument, bring the bubble precisely to the middle of the tube over one set of levelling screws, and then turn the telescope 180° about the vertical axis. If the bubble moves from the center, bring it half-way back by means of the adjusting screws at the foot of one of the wye supports.

Since the bubble is brought to the center of the tube each time a rod reading is taken, this last adjustment in no way affects the accuracy of levelling work, but it is a con-

venience and a saving of time.

2. Use of the Level

Levelling is employed to get the difference in elevation between points. With the level set up and the rod held on a point whose elevation is known or assumed, the reading that is obtained is called a (+) or backsight. Similarly, a reading on a point ahead or unknown is called a (-) or foresight. A point occupied by the rod in this way, but not recorded or used further, is called a turning-point. When two points have been connected by a series of readings of this kind, the sum of the backsights minus the sum of the foresights gives the difference in elevation. If the backsights are greater, the second point is the higher of the two. If the foresights are greater, it is the lower. A brief set of notes is given and worked out illustrating this matter. Work of this kind is called differential levelling.

B.S.	F.S.	Remarks
9.52' 10.12'	4.45′ 3.27′	B.S. onto B.M. of previous survey.
8.56′	1.01′	
7.40′	5.71'	
3.65′	8.62′	F.S. to pond level required.
39.25′ 23.06′	23.06′	
16.19′		Pond is above B. M.

When levelling is employed to get the elevation of a large number of points in a region, several or many foresights may be taken from one position of the instrument. It is customary then to note the height of instrument, and the elevation of any point observed will be that height less the foresight to the point.

A benchmark is a point whose elevation has been determined and which is marked and left for reference. It is

noted B. M. in level notes.

The following set of notes illustrates those commonly kept in running profiles of a road or railway. The form may be easily modified for any other class of work.

Summary. Levelling is comparatively simple work. Even though a level is somewhat out of adjustment, accurate results may nevertheless be had by taking backward and forward sights of equal length, and this check it is easy

Profile of Bull Band						Cartin 1997 SGould T
Profile of Park Road					Sept. 10, 1907 {Gould The Martin, Rod	
Sta	B.S.	H.I.	FS.	Elev.	B.M.&.	Description
BM3	12.23	34.98			22.75	
0			9.8	25.2		
1			6.6	28.4		
2			3.0	32.0		
T.P.			1.43		33.55	On stump
"	11.18	44.73				
3			6.1	38.6		
+65			2.7	42.0		
4			3.7	410		
5			5.2	39.5		
6			112	33.5		
TP.2			5.62		39.//	Boulder
"	3.48	42.59				
7			102	32.4		
1						

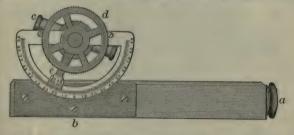
to secure by pacing. It is important that the rod should be held plumb during the levelling operation. This position is secured by careful attention on the part of the rodman and by waving the rod slightly. The length of sight varies with the instrument, the condition of the air, and the accuracy desired. About 300 feet is stated to be in general the best length on the score of accuracy, but speed will often require that much longer shots be taken. In accurate work, it should be remembered that error may be introduced by the slightest causes, such as disturbance of the tripod.

Levelling is employed by woodsmen in constructing dams and ascertaining the area of flowage, in laying out roads and railroads, and for the basis of topographic work. For these uses a light and cheap form of the level, sometimes called the architect's level, costing about half as much as one adapted to railway work, is commonly sufficient.

SECTION III

COMBINED HAND LEVEL AND CLINOMETER

A pocket instrument capable of a great variety of uses is shown in the accompanying figure. The eye is placed at a peep hole at the right end (a) of the main tube. The cross-wire is over (b) in the figure, and beside it, occupying half the orifice of the tube, is a mirror set at



an angle of 45° . Directly over the wire and mirror is a spirit tube (e), shown inclined in the figure. It is fixed to the milled wheel (d) which turns it, and the graduated arm (e), which serves to set the bubble parallel to the line of sight of the instrument, or to read the angle of inclination between them. When the bubble is in the center of the tube, the mirror below reflects it side by side with the cross-wire back through the peep hole.

This instrument is largely used by northwestern lumbermen in laying out roads, locating dams, etc., and it ought to be in the outfit of every woodsman. To use it as a hand level the zeros of the graduated arm and the scale must first be set together. The observer then sights an object through the tube, which he brings to a level by the bubble reflected in the mirror. He may then place himself on a level with the object by sighting at it directly,

or, if difference in elevation is required, a pole or level rod may be used to measure the amount.

The instrument may be used to find the difference in elevation between any two points without the use of a level rod. To do this the observer begins at the lower point, and, after levelling the instrument, sights in the desired direction and notes the point on the ground ahead intersected by the cross-wire. He then advances to that point and repeats the operation, and so moves on up the grade until the upper point is reached. As between every two observations he has advanced to a height equal to the distance from the ground to his eye, the height of the hill will be the product of that distance by the number of sights taken.

The instrument may also be used as a clinometer to measure slope. To do this the observer sights along the slope parallel to the ground, and then uses the hand wheel to turn the level tube until the bubble shows it is level. The measuring arm, turning with the wheel and the level, sweeps the scale and indicates the slope in degrees, or in per cents, according as the instrument is graduated.

In the same way, and with the aid of a table of tangents, one may use the instrument to obtain the height of a tree or a hill. This process is explained and illustrated on page 166.

For an improved form and more complicated use of the instrument, see pages 130-131.

SECTION IV

COMPASS AND PACING

The staff compass, with folding sights, cross levels, and a needle from $2\frac{1}{2}$ to 4 inches long, is familiar to most woodsmen. It is a very compact and practical instrument, has long been employed for retracing lines, and of late years, as forest lands have come to be handled more systematically, has attained a great extent and variety of uses. It has also been constructed in a variety of forms, combined with other instruments in some cases. The form

shown in illustration is the pattern of the U. S. Forest Service. The base is flat so that the instrument may be used to orient a plane table — it is square also and graduated on its edges with a protractor and two scales for drafting purposes; declination can be set off by means of a vernier; inside the box a pendulum is fitted and the staff mountings permit of turning the instrument and holding it edgewise while employed as a level or clinometer.



STAFF COMPASS

A main use for the staff compass in topographical and timber work is for making foot traverses, a purpose for which it is thoroughly adapted. The common **pocket** compass with needle $1\frac{1}{2}$ to 2 inches long, indeed, may be used for the same purpose, and when it enables a man to travel a mile with only 1° or 2° of angular swing, as it will do if carefully used, it deserves to be called a surveying instrument.

Pacing. The pace has been long used as a check on short distances, but the real capacity of pacing as a method of measurement has only recently been developed. It is of special value to woodsmen who must travel their country over in any case, and who by a little extra pains taken in this direction can bring out much valuable infor-

mation. As against chaining, pacing has the advantage of cheapness, it can be done by one man alone, and its accuracy is frequently quite sufficient.

The natural gait of the woodsman should be tested on measured lines and in pacing for distance he should always walk at his natural gait, not try to take a three-foot stride. The slope of the ground, if it is considerable, affects the length of step; the step is shortened whether one goes up or down hill.

This matter has been investigated accurately and the results of one extensive test are given in the table below,

INFLUENCE OF SLOPE ON LENGTH OF PACE AS TESTED ON MOUNTAIN TRAILS

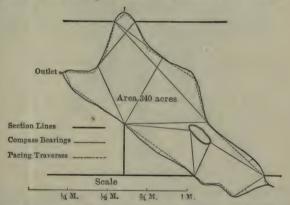
Slope	Length of step ascending	Length of step descending
0°	2.53	2.53
5°	2.30	2.43
10°	2.03	2.36
15°	1.84	2.30
20°	1.64	2.20
25°	1.48	1.97
30°	1.25	1.64

but for practical work it is better for each man to train himself on measured distances and learn to discount on slopes by experience and the sense that he develops. Similarly, rough bottom and bushes have an effect on the pace. This is best dealt with in the same way.

Harder perhaps to allow for, are the errors arising from a man's own condition. A man steps shorter when travelling slowly than when going at a good rate; he steps shorter when tired unless he forces himself to the work; he is not sure of himself in the morning or after a longer rest until he gets "into his gait"; he has his "off times" when nothing seems to go right. Keeping the count also is a source of frequent error. Woods travel is too uneven

as a rule to allow a pedometer to be employed. Some men register double paces. Others count up to a hundred in the head and take down the hundreds on a "clicker." in a note book, or by breaking an elbow in a tough twig carried in the teeth or hand.

Accuracy. With all its limitations, pacing is a very serviceable means of measurement and a man who has duly trained himself can get very good results. "Surveying" says, that when a man's gait has been standardized and on the work he walks at a constant rate, "distances can be determined by pedometer or by counting the paces to within 2 per cent of the truth." That refers, without doubt, to open land. In woods work too there



POND SURVEYED FROM SECTION LINES BY CROSS BEARINGS AND THE COMPASS AND PACING METHOD

are many men who can be depended on for results as close as that, but errors up to 5 per cent in a straight mile on uneven land is for the writer the usual standard of work. This is not serious. When the error is distributed over the mile by plotting, the utmost probable error in the location of any point is not over 25 yards.

Uses of the Method. (1) The staff compass is largely used in retracing old lines. Pacing may well be employed with it as a means of finding blind marks and corners, for this purpose replacing the chain.

- (2) In timber estimating, the area of waste lands, heavy bodies of timber, etc., can often be obtained quickly and with a fair degree of accuracy by this method, and these facts often furnish very great help in securing a close estimate.
- (3) The compass and pacing method is the cheapest for mapping roads, streams, ponds, and other topographic details in wooded country. For a real map, however, this method of survey should not cover too long distances, but should tie into more accurate work.
- (4) Compass and pacing may be used to get a reconnoissance map of a region of any size, using a road or any other avenue of travel that passes through it. Not only the line of travel may be mapped, but the hills and other features of the country that can be seen. Cross bearings with the compass will locate them in the horizontal position, and the clinometer will serve to get their height.

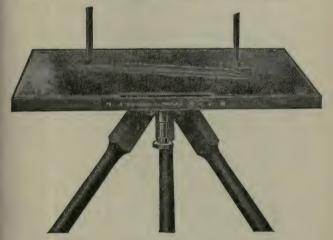
Specimen notes illustrating this method of work combined with the use of the aneroid barometer for determining height, and a diagram showing how it is made to contribute to the production of a topographic map will be found on pages 130-132.

SECTION V

THE TRAVERSE BOARD

The plane table in its simplest form is called a traverse board, and consists of a square board without levels mounted on a tripod. On this board a sheet of paper is pinned, and the map is developed in the field. A compass needle set into the edge of the board serves to "orient" it, or, in other words, to fix one edge always in the north and south position. A brass ruler with vertical sights attached serves both to sight with and to draw lines and scale off distances on the map. It is called an alidade.

A simple use for the board is to traverse a road, a stream, or the shore of a pond. Suppose, for instance, it is desired to survey a stream on the ice in winter, and a point on it is known by the crossing of a section line. The instrument should be set up at the known point, with one edge of the board set north and south as shown by the needle. A point is then chosen on the sheet to represent the one occupied on the ground, the edge of the ruler is swung about it until the sights range toward the second point to be occupied, say the next turn of the stream, and



TRAVERSE BOARD

a line is drawn in its direction. The distance between the two points is then chained or paced, and when this has been scaled off a second point on the map is obtained. The board must then be set up at the new point and oriented as before, when, the ruler being swung about the new point, a ray may be drawn from it to a third, and so on. Little difficulty will be experienced by one who understands compass surveying in working this instrument. A point on the sheet always represents the point occupied, and that is always the point to work from. The map is carried to completion right in the field and that, as regards both cost and accuracy, constitutes the advantage of the method.

Another method of working is by intersections. For this, it is necessary to have two known points or a measured base. The instrument is set up at one of the known points, and, the alidade being pointed at the other, a line



is drawn and the known distance scaled off upon it. Then, from that end of the base line representing the point occupied, rays are drawn in the direction of other well-defined objects on the shore which it will be desirable to locate. Flags may be used to define them, but natural objects will often suffice. The instrument is then

taken to the other known point, and set up by the range back to the first. Then swinging the ruler about the second point located on the sheet, the surveyor draws rays from this to the same objects as before. The intersection of pairs of rays directed toward the same object in the field fixes that point upon the map. This is done directly and graphically, no computation or reduction being required.

More complicated forms of the instrument, telescopic alidades, the application of the vertical angle, etc., need not be here discussed, as they are hardly likely to be employed by other than specialists. It seems likely, however, that among a large class of foresters and woodsmen this simple form of the plane table will find general use.

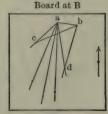
The following survey of a small lake made with the traverse board involves a somewhat more complicated use of the instrument than that described above. This particular piece of work took the time of two men for two days, but on the ice it could have been done more quickly. The steps in making the survey were as follows:

The steps in making the survey were as follows:

1. Base line A B measured, the longest straight line that could be had on the shore and in wading depth of water. Flags set up at its ends and at C, D, E, F, and G, prominent points on the shore visible from both ends of the base line.

2. Plane table set up at A as oriented by the needle. Point a selected on the paper, line drawn from it in direction of B and a b measured to scale. Rays a c, a d, a e, a f, a g drawn in direction of C, D, E, F, and G.

Board at A

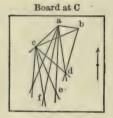


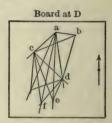
3. Table set up at B, oriented by ranging ba at A and checked by the needle. Rays drawn from b toward C and

D. These where they intersect corresponding rays from a fix points c and d. Rays also drawn toward E, F, and G, but the angles made with the corresponding rays from a are so small that these points are not given a good location.

4. Board taken to C and oriented by A and B. Check ray drawn to d. Rays toward E, F, and G, intersecting

similar rays from a, fix e, f, and g.



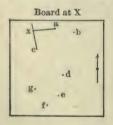


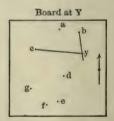
5. Board taken to D and similar process performed for a check. E, F, and G may also be checked with one another.

6. Fix other points on the shore such as prominent rocks or trees.

(a) By intersecting rays from any two of the primary points in the same manner as these were fixed.

(b) By drawing a ray from one of the primary points as c toward any object as X, setting up at X, using c x to orient by, and then fixing x by a ray brought back in the range A a until it cuts c x.





(e) By setting up the board on any desired point on the shore as Y, oriented by the needle, and ranging back from

any two flags or fixed points, through the corresponding points on paper, to an intersection which will fix the point occupied.

7. Fill in the shore line as the other work progresses, whatever at the time is nearest the instrument, by traverses,

sketching, etc.

SECTION VI

THE ANEROID BAROMETER

The aneroid barometer is a cheap and handy instrument which, when carried from one point to another, will tell approximately their difference in height. This it does by measuring the pressure of the air, varying as that does

when one goes up or down hill.

The essential parts of an aneroid barometer are out of sight. The instrument consists of a vacuum box with one very flexible and sensitive side, which works in and out with varying pressure of the air. This slight movement is multiplied, and converted into the circular motion of the pointing hand seen on the face of the instrument. At sea level the hand points to one part of the dial. As the instru-



ANEROID BAROMETER

ment is carried up a hill or mountain the hand, worked by expansion of the box within, turns round to the left. The

face is graduated to correspond with the height of column of a mercurial barometer, 30, 29, 28, etc., inches, these even

inches being divided into fractional parts.

This change in pressure corresponds with definite change in altitude. One inch on the scale means roughly 900 feet in altitude; a half inch means 450 feet, and so on. As a matter of fact, there is a foot scale on most aneroids outside the inch scale, movable and graduated from zero up to the capacity of the instrument. Thus, if one knows how high he is above sea level, he may turn the foot scale of his instrument until the registering hand points to that height, and, going either up or down hill, read directly the elevation of any station which he may occupy.

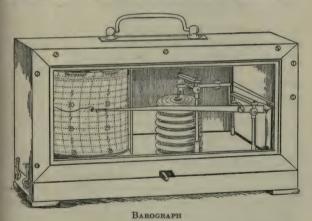
Just this process answers many purposes, but when best results are sought for, the operation is not quite so simple. First, there is the Correction for the Temperature of the Air. An inch difference in pressure at a temperature of 32°, for instance, converted into height, means one thing; at 70° it means a good deal more. In order to get accurate results, therefore, on considerable elevations, it is necessary to read the inner or inch scale of the instrument, take the temperature of the air at the two points, and obtain the elevation from tables. Such tables will be found on pages 111 and 112 and full directions for their

use accompany them.

Correction for Weather Change. The other liability to error arises from the fact that the air pressure is frequently changing with the weather. This does not hamper work seriously in the western country where the weather and pressure remain steady for long periods at a time, but difficulty does arise from this source throughout the East. With an approaching storm the air grows lighter, and the reverse in clearing weather. This effect is best seen on a stationary barometer, but it has a like effect on one that is in motion. Thus, if an explorer starts at a lake of known elevation and takes two hours in going to the top of a hill, the air pressure meanwhile may have changed so as to throw his height readings off materially.

There are three ways of obviating this, outside the evident one of working only in steady weather. One is to

return to the lake and take a second reading, using the average of the two to compare with that observed at the summit. A second, often available in cruising timber, is to read on the same point two or more times during the day and so ascertain the course of the barometer. The third method of correction is by means of another instrument which is left at the base station or some other convenient point, and read by another person every hour or half hour while the observer is in the field. Since in ordinary weather the air changes are the same over large areas, this arrangement tells what the field barometer would have read on the base station at any hour during the day. Better than this, however, is a self-recording barometer, or barograph, which makes a continuous record of pressure. The explorer compares his pocket instru-



ment with this as he starts out on his work, and again when he comes in. If these comparisons are satisfactory, he has the means of telling what his field instrument would have read on the base station at any time while he was

gone, and so obtains the correct figure for comparison with any given field observation. This arrangement enables him to stay away from known elevations half a day or a day at a time and still make fairly satisfactory height determinations.

This is all good in theory, but it must be said that in practice it does not always work out to one's entire satisfaction. The air, in the first place, is not the homogeneous fluid that it has been considered, but varies more or less from point to point. Then aneroids are not sure in their workings. Different instruments of the same make and cost vary greatly in reliability, and the observer needs to watch the best of them to see that they do not get out of order or play some kind of a trick. Barographs, again, are not thoroughly reliable. In particular, some of them do not follow the changes in pressure as fast as the portable instrument. Nevertheless, trial has shown that by the methods outlined sufficiently accurate results for many purposes can be obtained. In general it may be said of aneroid work that, while it cannot be counted on for refined accuracy, there is a large field open to it of good, useful work which no other instrument, on account of considerations of cost, can do. It is particularly serviceable in a timbered country where it is difficult to see from point to point, having there the same sort of advantage that the compass possesses in the same field.

Aneroids for ordinary work should be 2½ to 3 inches in diameter, graduated to the equivalent of 20 feet, and have as open a scale as may be. Such instruments cost from \$20 to \$35. For the finer class of work it may be advisable to employ a larger and more delicate instrument furnished with a vernier. A barograph costs from \$40 to \$50. Thermometers suitable for the work, in a nickel or rubber case about the size of a lead pencil, can be had for \$.50 to \$1

each

The following Working Rules have grown out of the experience of the writer and others:

1. Each instrument should be tested not only under the air pump but for general behavior in the field.

2. The best place to carry an aneroid while at woods work is in a leather case hung on the belt. The case serves to protect it from damage, also from extreme heat and rapid changes of temperature.

3. Any considerable blow is likely to throw the instrument out of order for the time being, if not permanently. Two instruments carried are a considerable insurance.

4. The aneroid should always be held in the same position when read, and be given a little time to adjust itself. By gentle tapping on the face the observer should assure himself that its various parts are all free and in working order.

5. In starting out for work it is well to carry the instrument a while, so as to get it into its regular field working

order, before reading on the base station.

6. One should check on points of known elevation as often as possible, and, if there is a choice of readings to refer to, he should depend on that which is nearer, time and elevation both considered.

7. A general caution may be needed that the proper use of the instrument is to obtain relative elevation of points by means of readings on the two. One must not expect by one reading to obtain his height above sea level.

REDUCTION OF ANEROID READINGS BY USE OF THE TABLES AND WITH CORRECTION FOR TEMPERATURE AND WEATHER CHANGES

(See tables on pages 111 and 112)

PROBLEM I. — Given barometric readings on two stations and temperature at each, to find the difference in elevation

of the two points.

Rule. — Enter the first column of Table I with the readings of the barometer on the two stations, and take out the corresponding numbers from column 2 (column 3 is for help in interpolating). Take the difference between these two figures. Call this result for the present a.

Add the two temperatures together (or if the temperatures of the two stations do not differ materially, multiply that of the region by two). With this enter Table II, that for temperature correction, and find in column 1 the nearest number of degrees given. Take out of column 2 the number corresponding, noting the + or - sign, and

multiply a above by this percentage. Let us call this b. If b has a plus sign, add it to a; if a minus sign, subtract from a. The result will be the desired elevation.

Example. — The barometric reading on a lake of known elevation is 29.500 inches, and the temperature there 72° F. Shortly after, the reading on a hill not far away is found to be 28.760 and the temperature 63°. How high is the hilltop above the lake?

From Table I we have

Barometric elevation of hill
Barometric elevation of lake
Difference (a above)

1150 feet
458 feet
692 feet

From Table II we have for $t + t' = 135^{\circ}$, C = + .042. b therefore $= 692 \times .042$, is = 29 feet. This must be added to a, since the sign of the factor is +, and the result (692 + 29 = 721) gives 721 feet as the required answer.

A short cut to the same result, which is accurate enough and which will save much labor in reducing a number of readings referred to the same base station, is as follows: Between 29.500 and 28.760 inches the difference of elevation corresponding to .1 inch pressure is 94 feet. This is obtained instantly by inspection of column 3 of Table I. Stated another way, the difference of elevation in feet is 6 per cent less than the difference between barometric readings expressed in thousandths of an inch. But the temperature correction for the conditions is +4 per cent, leaving a net loss of 2 per cent on the difference in the barometric readings.

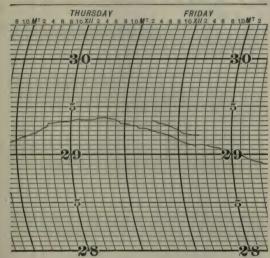
Now 29.500 - 28.760 = .740, and 740 - 2 per cent =

725. Answer, 725 feet.

PROBLEM II. — To correct for changes of pressure due to the weather, as shown by regular readings on a station

barometer or the record of a barograph.

The barograph sheet reproduced herewith shows for the working hours of that Friday a steady fall of pressure. At 6.30 in the morning when the party left camp the indicated pressure was 29.250 inches. When they got in at 5 P.M. it was 29.100. That difference in pressure corresponds to nearly 150 feet in elevation, and height observations made during the day would be uncertain to very wide limits if the change could not be allowed for.



The possibility of correction rests in two suppositions: (1) that at any moment of time the air pressure is constant over a considerable horizontal area, and (2) that the field barometer and the station barometer work together, and that they both follow exactly and quickly the change of air pressure. The latter point may be expressed in this way — that the field barometer, if left at the base station, would have followed the same course as did the instrument which in fact was left there.

The field barometer may not read the same as the barograph when they are brought together, but that "index error," as it is called, does not matter if the difference between the two remains constant. In this case the field barometer at camp in the morning read 29.350 and at night 29.200, .1 inch higher than the barograph. One may, therefore, when he gets to computing, draw on the

barograph sheet a curve through these two new points and parallel to the one made by the barograph pen. From this curve he may take off the reading for any hour in the day to compare with a field reading taken at the same time. Such a supplementing curve is shown on the sheet illustrated.

Example. — At 11 A. M. on the day in question at a point two miles away from camp the field barometer read 29.270. What was the elevation relative to the base station?

The field reading can not be compared with the morning reading at camp because the barometric pressure is known to have been changing. Neither can it be compared with the night reading, for the same reason. The short curve on the sheet, however, does tell what the field instrument would presumably have read at camp at any hour in the day. The curve at 11 A. M. is at 29.270, and the two points, therefore, are of equal elevation.

In view of the low accuracy of aneroid work, different users of the instrument have devised schemes for shortening or obviating the labor of computation. One that is serviceable where temperature at different seasons shows wide variation is as follows:

On the foot scale of most instruments 1000 feet at the higher elevations will be found to occupy a smaller sector on the scale than 1000 feet at low elevations — as 5000–6000 as against 0–1000. This can be tested by comparing against identical marks on the inner scale.

Now, being at a known or assumed elevation, set the corresponding graduation against the movable hand and observe where the thousand-foot marks above and below cut the inner or inch scale; next, take the values so obtained and compute difference of elevation accurately, correcting for temperature. If the result obtained varies seriously from 1000 feet, shift the foot scale by even thousands until a portion is found so graduated that it does correspond. With a constant correction of even thousands, elevations may now be had directly. Correction is not thus made for weather changes, however.

TABLES FOR REDUCING READINGS OF THE ANEROID BAROMETER 1

I - Barometric Elevation

1— Datometric Enevation							
Reading Inches	Elevation Feet	Difference for .01 inch Feet	Reading Inches	Elevation Feet	Difference for .01 inch Feet		
Reading Inches		for .01 inch Feet -13.6 -13.5 -13.4 -13.3 -13.3 -13.2 -13.1 -13.1 -13.0 -12.9 -12.8 -12.8 -12.7 -12.6 -12.6 -12.5 -12.5 -12.5 -12.4 -12.3 -12.2 -12.2 -12.2 -12.2 -12.0 -11.9 -11.9			for .01 inch Feet		
23.1 23.2 23.3	7121 7004 6887	-11.8 -11.7 -11.7	26.5 26.6 26.7	3380 3277 3175	-10.3 -10.2 -10.2		

 $^{^{\}rm t}$ Taken from Johnson's "Surveying " and Report of U. S. Coast and Geodetic Survey for 1881.

I - Barometer Elevation - continued.

Reading Inches	Elevation Feet	Difference for .01 inch Feet	Reading Inches	Elevation Feet	Difference for .01 inch Feet
26.8 26.9 27.0 27.1 27.2 27.3 27.4 27.5 27.6 27.7 27.8 27.9 28.0 28.1 28.2 28.3 28.4 28.5 28.6	3073 2972 2871 2770 2670 2570 2470 2371 2272 2173 2075 1977 1880 1783 1686 1589 1493 1397	-10.1 -10.1 -10.1 -10.0 -10.0 -10.0 -9.9 -9.9 -9.8 -9.8 -9.7 -9.7 -9.7 -9.7 -9.6 -9.6 -9.5 -9.5	28.7 28.8 28.9 29.0 29.1 29.2 29.3 29.4 29.5 29.6 29.7 29.8 29.9 30.0 30.1 30.2 30.3 30.4 30.5	1207 1112 1018 924 830 736 643 550 458 366 274 182 91 00 91 181 271 361451	-9.5 -9.4 -9.4 -9.4 -9.3 -9.2 -9.2 -9.2 -9.1 -9.1 -9.0 -9.0 -9.0

${\bf II-Correction\ for\ Temperature\ in\ Degrees\ Fahrenheit}$

The Correction for Temperature in Degrees Fameunes							
t+t'	C.	t+t'	C.	t+t'	C.		
0°	-0.1025	60	-0.0380	120	+0.0262		
5°	-0.0970	65	-0.0326	125	+0.0315		
10°	-0.0915	70	-0.0273	130	+0.0368		
15°	-0.0860	75	-0.0220	135	+0.0420		
20°	-0.0806	80	-0.0166	140	+0.0472		
25°	-0.0752	85	-0.0112	145	+0.0524		
30°	-0.0698	90	-0.0058	150	+0.0575		
35°	-0.0645	95	-0.0004	155	+0.0626		
40°	-0.0592	100	+0.0049	160	+0.0677		
45°	-0.0539	105	+0.0102	165	+0.0728		
50°	-0.0486	110	+0.0156	170	+0.0779		
55°	-0.0433	115	+0.0209	175	+0.0829		
60°	-0.0380	120	+0.0262	180	+0.0879		

SECTION VII

METHODS OF MAP MAKING

1. Introductory

There is a well defined call at the present time for good maps of small forest areas — maps which show topographic features and record essential facts about timber stand. With the consolidation of large forest properties and their more careful and foresighted management, the need is felt for good maps of these as well, and it is certain that this demand will increase.

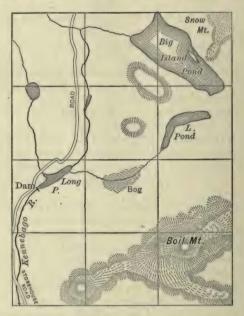
The maps of the past are of all grades of accuracy and utility. A checkerboard of lot lines, with the waters roughly laid down, and estimates of the stand of timber, is the utmost that many lumber companies can command. Some improve this by hatching to represent mountains and divides, and by going more carefully into water lines and areas.

Hatched Maps. The accompanying map represents part of a township owned by a Maine lumber company, and is a good example of a class of maps now having wide use. For the purposes of the map and of administration, the township was divided into sections, and as the lines were run, chainage was taken at the crossings of streams and main divides. In addition, some cruising was done within the lots, chiefly to ascertain the amount of timber. On this basis the map was drawn. The course of streams is shown approximately. Mountains and prominent ridges are hatched in. Main existing roads may be put in roughly.

A map like this, with lines on the ground to correspond with it, is of great service in the management of forest property. Logging contracts can be let with clearly defined boundaries; distance to haul is approximately known; in a rough way the nature of the ground is represented. It has, however, very evident limitations. Off the section lines, it is all judgment or guesswork, and the details of the country, such as have a very material effect

on all operations, are not shown and cannot be shown with that method of representation.

The cost of such a map is very slight over and above the cost of the survey work in sectioning. That in the region named commonly costs from \$600 to \$800 per township. If a region is divided into sections or quarter-sections, a



good cruiser can produce a map like this as fast as he can travel over the country.

Contour Maps. The actual shape of a country is best represented by contour lines. A contour line is a line of equal elevation, the line a man would follow if he traveled round a country keeping at a constant height, or what would be the shore line could a country be submerged to a given level. The base level of a map representing a country near the seashore would naturally be sea level. The first contour on the map might follow the line of 100

feet elevation, the second run 100 feet above that, and so on, one for each 100 feet. A little consideration will show that the lines indicate not only direction of the slope of the land, but also the rapidity of slope, for when contours are close together the ground is steep, while on flat land they are wide apart. Hill tops are circled by a succession of contour lines. On lower land they often run in a very sinuous course.

When one examines such a map and thinks of its construction, the first idea is that a tremendous amount of labor is involved. To follow out a succession of contour lines with ordinary surveying methods would indeed be an endless task. That is not the method of construction, however. It is rather sketching, guided by the location, in horizontal position and height, of a sufficient number of points. If one knows how high the top of a hill is above its base, that tells one at once how many contours, 100 feet apart, come between the two, and a glance at the hill perhaps will tell if it is of even slope. Similarly the location of divides and ridge tops, and, on the other hand, of low points, whether occupied by water or not, gives control points which aid in representing the slope of the land. The main problem of the topographer is how best to make these locations — most accurately and at least cost.

General Considerations. The instruments and methods available for the production of topographic maps have been described on previous pages. In employing them, to secure practical results, very much depends, of course, on their effective use and proper combination. In this relation, some general principles of surveying work and the conditions of woods work, as distinct from those of ordinary

surveying, require first to be stated.

1. A hunger for accuracy is part of the make-up of every good surveyor and map-maker. At the same time, he has to remember that if such work costs more than it is worth to the man who pays for it, it will not be done. Accuracy to a certain degree is necessary; on the other hand, there are limits of cost. A proper balance between the two is required. The result may be called an efficient map.

- 2. In securing an efficient map, a main principle to hold in mind is the relation between accurate and expensive work and work of a lower degree of accuracy. If elevations in a topographic survey were put in by level only, and horizontal positions fixed by compass and chain, an accurate result would be had, it is true, but it would be at enormous cost. On the other hand, the use of barometer and pacing alone might furnish a map so inaccurate as to be of little account. The effort must be to construct a skeleton of reliable points and lines, to which less accurate and costly work may be tied - to put points within reach, one might say, of the weaker method or instrument. Survevor's compass and chain, staff compass and pacing, and sketching form such a series in the horizontal determination of points. The level, the aneroid, and sketching are similarly related in height work. Sketching is the final term in any case, and much depends on it for both accuracy and appearance. In a way, it is easy, but real excellence in the art depends on a combination of eye, memory, and artistic sense.
- 3. Throughout any ordinary work of this kind, it has to be understood that much detail is too fine for representation or is really unessential, and on that account the topographer should neglect it. Makers of accurate maps neglect only what does not show on the scale of the map. Woodsmen will generally find it necessary to adopt a more liberal rule.

The conditions under which forest mapping is done have an influence on methods in the following ways.

- 1. Timber growth itself presents an obstacle to clear sighting. That favors the compass as against the transit for boundary work, and in the same way, in topographic mapping, triangulation and the vertical angle are put at a disadvantage as against methods which can be carried on under the cover of the woods.
- 2. Forest topography should generally be tied to property boundaries, rather than to topographic prominences. Commonly, a survey of his boundaries is the first and most important work to be done for an owner who wants accurate knowledge about his land. It will, there-

fore, save time and money if the interior features can be tied to them.

3. Topographic maps of forest property should be especially clear in respect to road lines and other points of importance in lumbering operations. The map-maker should, therefore, understand these operations. It will, also, save time and money if topography and timber can be examined together, at the same time, and by the same man.

With these principles in view, the following are methods recommended for the production of forest maps. It is well in discussion of the matter to divide the work into two classes — that on small tracts, where close work is required, and that on larger tracts, where different methods must be employed and a lower standard of accuracy may be allowed.

2. Mapping Small Tracts

A tract of eighty-nine acres, well timbered and of strong relief, that was surveyed by the class of 1907 in the Harvard School of Forestry will serve as illustration. The following steps were taken in the process.

1. Boundaries surveyed by compass and chain; marked stakes left every twenty rods; bounding lines and corners remarked. Two days' work for three men, more if there is

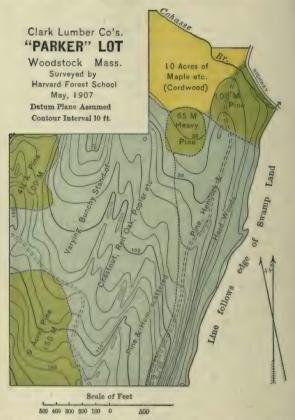
special difficulty with the old boundaries.

2. Elevation of one convenient point ascertained or assumed, and levels run over the roads crossing the tract, leaving bench marks plainly marked every twenty rods or so. Levels, also, run down to point x. (See page 119.) One half day's work for two men.

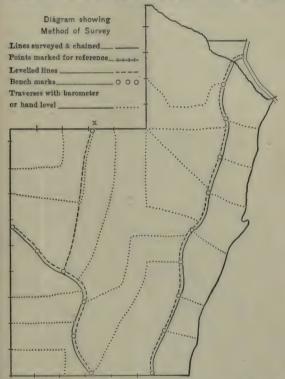
3. Outlines of tract plotted to scale on paper; this pinned on traverse board with meridian of survey parallel to N and S edge of board; roads run in with the chain and position of bench marks taken. One half day's work for three men.

4. Sheet on the board without the tripod taken into the field, a scale serving for alidade; detail mapped in by short foot traverses from the known points; elevations got partly by aneroid, partly by hand level. One day's work

for one man. Any board to hold the sheet will do, a small compass being used to orient it. By the time this work is done, a practical man may, in addition, have learned about all he wants to know regarding the timber.

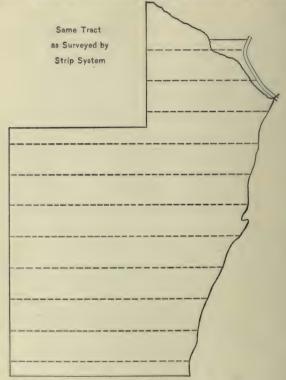


5. Since the lot is to be operated from a portable mill set near its northeast corner, go over the lot with the map in hand and see that the topographic difficulties and opportunities are correctly represented. Alternative Methods. 1. Compass and chain may be used to survey the roads and the plotting done off the field. This is most convenient in wet weather, but when a traverse board is at hand and can be used, it will be found the quickest method of survey and the least liable to error.



2. Transit and stadia might be substituted for both level and traverse board in the survey of the roads, and, where the woods are open enough, in mapping the detail of the topography. This method involves much computing, is generally cumbersome, and except in the hands of a skilled and practiced man is liable to give rise to error.

3. After the boundaries are surveyed and the primary point in elevation is fixed, a topographic survey and timber estimate might be made together by means of the strip system of survey described on page 188. For the topographic work, a barometer would be carried in the party



and the elevation of needed points read and noted or plotted down in connection with the chainage by the notekeeper. If the air pressure was not steady, it would be necessary for the barometer man once in a while to leave the party and go back to the base for correction. The combination of barometer and barograph gives rise, in a method already not too accurate, to additional errors, and should not be employed except when it is the only practicable method.

This method of survey may suffice in favorable conditions, and where the requirements are not of the strictest. Work with the level, however, is quick and sure, and in general it will be found advisable to use it freely.

The Map. In plotting tracts of this size, and up to a few hundred acres in extent, scales of 400 feet or 20 rods to the inch are found to go well with a 10-foot contour interval, and to furnish a serviceable map. A larger scale and a smaller contour interval would naturally go together.

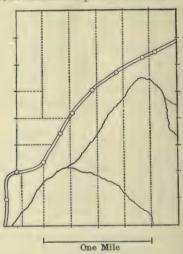
3. Mapping Large Tracts

A. With Land already Subdivided. If the region to be mapped comes under the public land surveys, or if there are plain and reliable lines of other origin on the ground, a skeleton of level lines with barometer work tied to them is the treatment indicated. Generally the level work is best carried along the waters or roads. Ponds and lakes form the best sort of reference points, and frequently natural water levels perform a large part of the work required. Section lines may, however, furnish in some cases the best routes available, while on very broken land it might be necessary to resort to the vertical angle.

How the barometer work shall be done depends on circumstances. If the weather is perfectly steady, or the level points are near enough together, elevations may be read direct without a weather change correction. If, however, the weather is shifting, and the cruiser must stay away from known points many hours at a time, a station barometer or barograph will have to be employed. In any case, the topography can be mapped at the same time that the timber is being examined.

B. Topography Based on Survey of Roads or Streams. If the tract to be surveyed is an undivided township, or is in any other form that is too large for accurate mapping, it may be cut up by one means or another into smaller areas that can be handled. The lines of easy subdivision naturally

furnished by a large timber tract are its streams. On these transit and stadia furnish the most efficient means of survey. If roads are available, the same method may be employed, or another may be substituted.



Surveyed bounds with chainage marks
Road surveyed by stadia, reference points
fixed by stadia and by level
Citata anamana mith hanamatan

On the tract used in illustration, the road, rather than the stream, was used for the subdivision. The different steps in the process of survey were as follows:

1. Outside boundaries run with compass and chain.

Chainage marks for reference left every quarter mile.

2. Road across the tract surveyed by transit and stadia, using the needle and setting up the instrument at alternate stations. Points marked at short intervals. See notes on page 86.

3. Level line run along road, giving elevation of points

established in the stadia traverse.

4. Strip surveys run between the road and the boundary

(see page 188), tving into the marks left. Elevations got by aneroid, corrected by barograph. Numerous modifica-tions of the rectangular system made as required.

Alternative Methods. 1. On roads the traverse board with chain is undoubtedly the best instrument for making a survey of fair accuracy. The compass and chain might also be used. But when streams are utilized, unless on ice, stadia measurement will be found to be best and quickest.

- 2. The level might be dispensed with, and the transit used as a level on the same settings from which it is used to get bearing and distance. This works best on a stream with grade all one way, and, in the case of a party by itself in the backwoods, is probably the best means of getting data of this kind. One additional man is then required for maintenance.
- 3. Instead of the strip survey, using compass and chain, compass and pacing may be employed with circular plots for the timber. It may also be better or necessary to discard both rectangular systems, and work out the topography by means of road lines, passes, etc., controlling features in the lumbering development.

C. Subdivision and Topographic Survey Combined. The following procedure has been carried out on a considerable scale on undivided townships in New England. The methods employed have been found to be cheap and practical, and the maps resulting have stood the tests of use and time.

1. Boundaries renewed and tract divided into sections by compass and chain. Topographic notes taken; chainage marks left every quarter mile. Two months' work for

a party of seven men.

- 2. Elevation of some point above sea level obtained, if possible; if not, datum plane assumed at or below lowest point on the tract. Level lines run over roads and streams to ponds, camps, and other accessible points, well distributed through the tract. Commonly a week's work for two men.
- 3. Detail of topography and timber worked out together. Mountain peaks located by cross bearings; streams and roads by compass and pacing traverse; other features

partly by traverse, partly by straight-line travel across the sections. Elevations by barometer checked by the barograph whenever it is necessary to remain away from known points a considerable time. Timber estimated and topographic notes obtained at same time. Cruising, reduction of notes, and map making about six weeks' work for the explorer, who may need a companion or camp man.

Comments. 1. Division into mile squares may look expensive, like going a long way round to secure topographic data. These lines, however, have value on other accounts; have, in fact, proved their value over and over again in timber land administration. As before stated, they are useful in definitely bounding logging contracts, they are perfectly understood by logging foremen, and are of great service to them in their timber estimates and the laying out of their roads. They are, in addition, of great service in keeping track of subsequent cutting or other developments on the land.

On the other hand, the mile square is not so large an area but that it can be mapped accurately and its timber estimated according to the methods here recommended.

2. The strip survey system might, of course, be used instead of the one-man system employed. The advantages of each will be understood from what comes before and after.

3. It may be advisable in some cases to separate entirely the topographic and timber work. In general, however, the thoroughly equipped man will find that travel that

helps him in one direction helps also in the other.

The Maps. Maps of forest property should be on a large scale to allow the preservation of notes about small bunches of timber, etc. Four inches to the mile for tracts of large size has proved serviceable. As to contours, a fifty-foot interval will serve, in the rough land of New England, to represent most features of the topography.

The results of such a survey are, for business purposes, best embodied in two map sheets, one showing the waters, relief, and other permanent features of the country, the other exhibiting all the facts concerning the timber. This last should be on tracing linen, so that it may be laid

over the topographic sheet, and the two seen in relation. Not only the amount of timber is thus exhibited, but the steepness of the ground it stands on, and the distance it must be hauled. It will appear, too, whether a valley has been cut clean to a divide. On this timber sheet, cuttings and other operations of succeeding years may be plotted. If it gets too complicated, it may be thrown away and a new one substituted.

A sample map of this kind is reproduced on reduced scale herewith. These maps may also be supplemented by topographic models. Contour maps are not read easily by every person, as, for instance, by some lumbermen, but a model of the land, as it lies out of doors, is immediately grasped by all. With the aid of a blue print of the map which may be cut up and used as a pattern a model is cheaply built out of cardboard or veneer. With such a model at hand, a contract may be let or plans of work talked over in the office with the same clearness as to major features as if men stood on the ground.

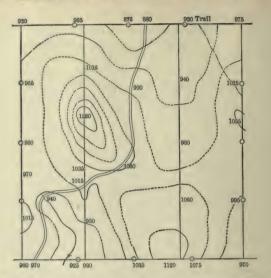
Following is a topographic map of a section of land as derived from traverse of the boundaries, a road, and two trips across it. After that come notes of the road traverse and of one of the trips across it. For notes of survey of south line see page 29. On the map observed elevations are written in. Contours as seen are solid; contours inferred are broken.

Principles of Cruising. A plan of cruising designed to secure topographical and timber data every man will think out for himself and a new one for each tract undertaken. The following, however, are believed to be sound principles for guidance in this class of work.

1. Main streams, roads, lakes, etc., should of course be traversed, and they may be important enough to demand some other method of survey than compass and pacing. One should be very careful, too, about waste lands, burns, and the boundaries of heavy bodies of timber.

2. It is generally advisable to explore the country one section at a time, for in that way one comes out with the clearest ideas upon it.

3. Cross country travel which locates brooks and ridge



tops by intersection may suffice for topographical purposes, while it gives a juster view of the timber than could otherwise be gained. Locations, too, will be more accurate along such a line than where a crooked route is followed.

4. Extreme points are in general the ones to read on for height, — that is to say, ridge tops, brook crossings, etc. One may combine with this also a system of reading at regular intervals. It will be enough to read the thermometer half a dozen times during a day to get the course of the temperature, unless extremely high points are occupied.

5. Relative heights are frequently of far more importance for logging purposes, as, for instance, in connection with the grade of roads, than is absolute elevation. It is often advisable, therefore, to establish sub-centers of work and determine elevations relatively around them rather than refer readings always to a distant base station. On the same principle, if a region is hard to get at with the level, it may serve the purpose of the map to fix the height of some central point in it by two or more aneroid readings, and then work around that.

Starti	ng at s	outh line of Township, 25 rods E on the Sline
of Se	ction a	s given in survey notes. Elevation 970 ft.
as as	certain	ed from pond nearby determined by level.
		Thence in Section 25
Bearing	Paces	
N20°E	200	Along easy Slope right, in good timber,
N5°E	350	to swamp
N50E	75	to small brook running SE. Elevation 940'
N82°E	250	at 100' into timber again
N73°E	150	up slope, to pass between hills
N65°E	325	right & left Elev. 1060'
N42°E	175	on a general slope East of about 10%
N25°E	400	to flat land and
N20°E	225	Elevation 990'.
N	300	In flat land with thick spruce growth
N8°W	225	to North line section 25. 190 rods East
		on it as given by Survey notes.
		Elev. 880 ft. Checked on B.M. half an hour later

COMPASS AND PACING TRAVERSE OF ROAD ACROSS SAME SECTION.
ELEVATIONS READ FROM FOOT SCALE OF BAROMETER

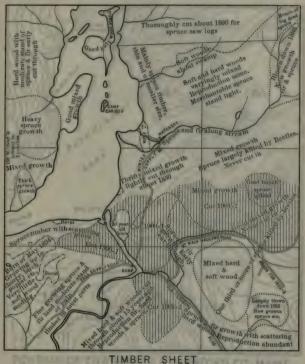
- 6. There is occasionally a locality especially critical from the lumbering point of view, such, for instance, as a pass which makes it possible to haul from one drainage to another with a level road. The topographer ought to be enough of a lumberman to recognize these points, and when he does he will put special time and pains upon them.
- 7. Field observations may be recorded either in the form of running notes, or mainly in the shape of sketches on a plat of the ground. Probably a combination of the two methods will be found most satisfactory. A note book especially ruled for the purpose to the same scale as the final

	Bar. Camp. (Elev. by level 901') 6 A.M. 29.350)	
	T.A. 60° Barograph 6 A.M. 29.250	Bar.	Compute
	Canada road on N. line section 25 Time 7.10	29.360	881
Steps	Go West on Section line		
190	½ mile mark of survey. Flat Spruce ground.	29.365	876
510	3 mile mark. Slope N.E. then N.	29.295	935
515	Section corner. Gentle slope N.W. All Spruce		
	timber. Bar. 7.40	29.305	920
	Return to 100 Steps E. of 34 mile mark and		
	go S. 7° W. in Section. Start at 7.50 A.M.		
350	Gentle Slope N. N.W. Spruce growth	29.205	1016
400	Top of hill, falls Steeply E. and W.	28.990	1220
470	Down strong grade S.W. Timber on hill mixed	d	
	and short. Bottom rough	29.175	1035
175	Canada hay road on easy land	29.195	1015
375	Down easily in large mixed growth to edge	,	
	of swampy land	29.260	950
280	Township line 60 steps East of 34 mile ma	rk.	
	Bar. 9.35 (T.A.65°)	29.280	930
•	Bar. Camp 11 A.M. (T.A. 69) 29.280		
	Barograph II, 29.175		

STRAIGHT TRAVERSE ACROSS SECTION. ELEVATIONS BY BAROMETER CORNECTED BY BAROGRAPH

map will be found a great saving of labor and an aid to clearness.

8. The map is best worked up on the ground. The added accuracy and certainty gained in this way more than pay for the cost of carrying necessary equipment around. The topography may be drawn in pencil on the final manuscript sheet, and an outline sketch on any kind of paper will serve to gather up the timber notes temporarily.

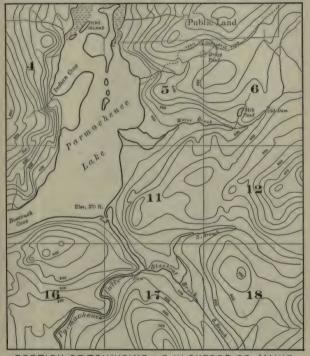


TIMBER SHEET
Explored 1900 Cutting since that date marked by section lining

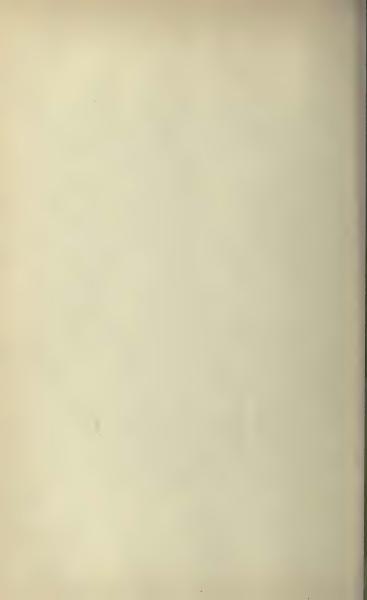


T23H2 A36WT ON 10-23

A. The map is the result of the provided The sold-La sorrey and permany polarity to this may more than part for the result of marriage moments or quipment to send This type graphy may be chosen to perman on the first managed married and a common density and the first managed married and a common density and the first permanent of the send of the first permanent of the send of



PORTION OF TOWNSHIP 5 R IV OXFORD CO. MAINE
Topographical Sheet Datum Plane, Umbagog Lake
Contour Interval = 50 feet



D. Western Topography. Use of the Clinometer. The above described methods grew up in the East among varied conditions of topography and value. Brush that interferes with sighting is widely prevalent, and another determining factor is the general employment of horse logging, a style of operation for which close regulation of grades is not essential. Conditions in the West are frequently different from the above, in respect to one or more particulars.

The aneroid barometer has not on that account yielded its place entirely. Particularly in Western Washington and Oregon does it still hold the field, because of the dense brush widely encountered, which makes almost impossible the clear sighting necessary for the employment of any other height-determining instrument. On the contrary, the temptation is to rely on the aneroid for work that it should not be called upon to do. Where, as is the case here, railroads are employed for nearly all main transportation, heights with a reliable basis are essential if a map is to be widely serviceable. Frequently the ground lies in such a way that the routes of future railroad development are evident. Levels run along these routes, with aneroid work for the rest, is then the natural treatment. Just this method has been employed in numerous cases.

Such logical and adequate treatment is not always possible, however, nor is it always permissible under the restrictions of the work in hand. A variety of methods is in fact employed, especially for the control work. As for the detail, the fact remains that when points in elevation have been reliably determined at distances not more than from one to two miles apart, good aneroids intelligently used will give topography sufficiently accurate for general purposes, while here as elsewhere their use saves expense by permitting the topographic and estimating work to be done together. Complaints of the results of aneroid work frequently arise from unskilled use and from employment of instruments of inferior character. The quality of instruments obtainable at moderate cost has within a very few years greatly improved. It is not to be

denied, however, that rapid weather changes sometimes make accurate work difficult.

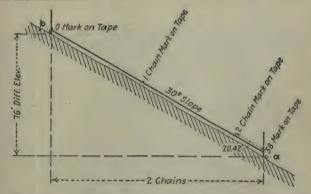
Some interior mountain territory is characterized by lightly forested ridges contrasting with great density of timber and brush along the streams, while logging methods are often such that accurate knowledge of grades on valley lines is not essential. In circumstances such as these, circuits of transit and stadia work carried over the ridges have proved a satisfactory method of height control. When areas concerned have never been covered by the land surveys, angles have been turned and read in addition for the purpose of control in the horizontal direction.

With control laid out in this way the early plans of reconnaissance in such country involved, as the next step, the crossing of valleys with strip surveys, the aneroid being relied on for elevation. This plan of work, starting from known points on the ridges and running long lines independent of one another, crossing the brooks and valley bottoms (where grade was most important) at a long distance from known bases both horizontally and vertically, made demands on the aneroid which it was not able to meet successfully.

Height work along the stream lines was an evident corrective, but a substitute scheme that at the time of writing seems to be filling the requirement is the use of the tape and clinometer. Both instruments have, however, been subjected to modification. The clinometer has been made more efficient in numerous ways; in particular the arc has been enlarged and so graduated that instead of degree or per cent of slope it gives difference of elevation in feet for the given slope and a stated distance (66 feet or one chain in present practice). The tape used for the purpose is $2\frac{1}{2}$ chains long, two chains of it marked in links as usual, while the extra length or "trailer" is so graduated that the inclined distance along any slope which corresponds to two chains horizontal may be set directly. By these devices two short cuts are accomplished: first, difference in

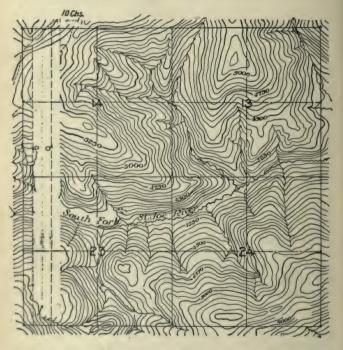
¹ For a fuller description of this method see "The Timberman," March, 1916, or "Engineering News," Vol. 75, No. 1, p. 24.

elevation is found directly from the slope observation; second, with similar directness surface chainage is converted into horizontal distance. These two things are the essentials wanted. To facilitate the work, the graduations on the trailer of the tape correspond with those on the arc of the clinometer.



The method will be grasped from the accompanying figure and the following explanation: If a party is ascending the slope indicated in the figure, the man ahead (who serves not only as head chainman, but runs the compass, takes notes, and sketches topography), as the tape comes to its end, sights with his clinometer at the height of his eve on the rear man (who may be the timber cruiser as well as rear chainman). The reading obtained, in this case 38, is the vertical rise per 66 feet horizontal on the slope between the two men. That corresponds to a vertical angle of 30°, but the fact, not being needed, is neglected. The topographer now calls out "38" to the rear man, who lets the tape run out to that mark, as a matter of fact 20.42 feet beyond the two-chain point. When the chain to this mark has been drawn straight and taut and pins are set. two chains is the horizontal distance between them. This the topographer may now plot on his map. The height of the new point (twice 38, or 76 feet above the first one) may also be used as the basis of sketching.

Two miles per day are readily covered by two men, drawing topography carefully and estimating a good stand of timber. Not only has cruising work been done by this method, but control work as well, using more care and two instruments. This last use of the method requires making circuits several miles in length around either subdivisions of



land or topographic areas. For cruising work the method is carried at farthest two miles to a tie point. Errors in direction and distance are seldom over $\frac{1}{2}$ chain per mile and the average error in height work is 10 feet. In very brushy country some tricks of the trade are introduced in the interest of speed, as sighting to the flash of a mirror or the metal note holder of the cruiser. In country of long

open slopes an alternative method is to take longer shots to noted objects, chain up, and compute the elevation.

Above is practice developed in the United States Forest Service. The cost is given as 12 cents per acre as a total for topography and cruise. Some commercial work is done on the same general plan, a five-chain tape being used and correction for distance made from tables in the field.

The accompanying map of mountainous land in Idaho shows at the left the topography along two miles of section line as developed by a survey for control purposes which surrounded four sections. This control work naturally is performed and checked in advance of the detail work. To the right the topography of the greater part of the area has been filled in, but a strip left blank indicates how it is built up, from parallel lines 10 chains apart crossing the territory. This map is completed in the field, a board and outline section sheets facilitating the purpose.

This method, though developed in special conditions in the West, promises, with some of its modifications, to win a considerable field of employment.

SECTION VIII

ADVANTAGES OF A MAP SYSTEM

Following are the advantages which a good set of maps renders to a large business concern. To secure these a good man will be required in the field to keep up lines, map the cutting of successive years, and watch the condition of the timber.

1. Great saving in the aggregate can be effected through the detection of small losses, such as windfalls and insect depredations, also by finding bodies of unhealthy timber, and as far as possible having such material cut and hauled.

2. The location of all sorts of roads, whether railroads, logging roads, or supply roads, is greatly facilitated. Exploring is saved, and distances are accurately known.

3. Operations can be planned and largely controlled from a center with all sources of information at hand.

The timber resources are known; also their location, and all related facts. The cut can be located for years ahead to the best advantage, so as to make driving and the hauling of supplies, for instance, come cheapest and handiest.

4. A map system preserves information about the land. An old lumberman or cruiser has a lot of information in his head that is lost to a business when he dies or steps out,

unless it is fixed in some permanent form.

5. A concern knows what it is possessed of, and has that information in the form most easily taken in by all intelligent men whom it may be desirable to inform; for instance,

stockholders, and possible money lenders.

6. A good map system in a business may pay for itself at the first change of management. A new manager coming into a business is in the hands of his employees for years until he can get first-hand knowledge of his country. With the aid of a good map system working command of a big property may be had in a year.

7. A reliable map system followed up for a term of years through a series of pictures of the land furnishes a record of its growth, and so enables a concern to grapple

with the question of future supplies.

PART III LOG AND WOOD MEASUREMENT

PART III. LOG AND WOOD MEASUREMENT

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PART III. LOG AND WOOD MEASUREMENT

SECTION I

CUBIC CONTENTS

The simplest way to measure the contents of a log is to take its length and mid-diameter and ascertain the cubic contents of a cylinder having those dimensions. Bark may be taken in or left out. By the use of a caliper and tape, a very close result may be had on logs that are not too long, provided care is taken either by inspection or by cross measurement to get a true mid-diameter. Trees cut nearly full length are given as a rule too large a value when measured in this way, — larger, that is to say, than their actual cubic contents. The percentage of overrun for large spruce cut off at 5 to 8 inches diameter in the top is about 6 per cent of their true volume.

When logs are placed in a pile the best that can be done is to use a diameter which is an average between the diameters of the ends, swell at the stump, if present, being

disregarded.

First among the tables for log measurement given in the back of this work is a table of cylinders with contents in cubic feet, standard measure. The lengths in feet are given in the first vertical column, the diameters in inches on the upper horizontal line, and the contents of any log is read off opposite its length and beneath its diameter. If the length is not given, add together such lengths as will make it up. Thus a log 12 inches in diameter and 47 feet long has the contents of a log 40 feet long + that of a log 7 feet long, or 31 + 5.5 cu. ft. = 36.5 cu. ft.

For practical purposes results near enough will be had if fractions of inches more than $\frac{1}{2}$ inch are taken as of the inch above, and fractions of $\frac{1}{2}$ inch and less are disregarded.

For convenient use in scaling, these figures should be stamped on the bar of a log caliper. They may be so arranged on a bar as to throw out a fair proportion for bark.

This system of log measurement is in actual use in but one business concern, so far as known to the writer, yet it is the simplest and most natural measurement for logs that are to be converted into pulp, shingles, excelsior, etc. It is not a difficult matter to arrange a factor or factors for converting cubic measure into board measure.

SECTION II

CORD WOOD RULE

The figures given in the table on page 239, those for cord measure, are not cubic feet of solid wood, but what have been called "stacked cubic feet"; - the space which wood will occupy in a pile. 128 of these make a cord. Like the preceding, these figures are ordinarily placed for conven-

ient use on the bar of a caliper rule.

These figures have been long and widely tested in practice, and when used as designed have given satisfaction. Logs should not be measured in too long lengths, for whole trees measured in this way may not hold out. Again, small, crooked, and knotty timber will pile up rather more cords than the rule gives. On a good quality of pulp wood these figures yield just about the same return as the results of piling. For further details see Section VIII, on cord measure.

SECTION III

THE NEW HAMPSHIRE RULE

The New Hampshire Log Rule is exactly the same as the last in principle, only an artificial unit of measure has been created. The "cubic foot" of New Hampshire log measure is 1.4 times the cubic foot of standard measure, and nearly twice the foot of the cord wood rule. The New Hampshire law regarding the matter is as follows:

All round timber, the quantity of which is estimated by the thousand, shall be measured according to the following rule: A stick of timber sixteen inches in diameter and twelve inches in length shall constitute one cubic foot, and the same ratio shall apply to any other size and quantity. Each cubic foot shall constitute ten feet of a thousand board feet.

This rule is extensively used in scaling spruce in Maine, New Hampshire, and Vermont. A broad caliper bar is stamped with the figures, and the stiff iron jaws attached throw out \(\frac{3}{4}\) inch from the diameter for bark. The diameter is taken in the middle of the log, and in ordinary practice logs of any length are measured as one piece. The values given by the rule run parallel to actual cubic contents and the rule is therefore a fair one as applied to pulp wood. It is not a satisfactory measure of the yield of logs at the saw, small logs being for that purpose overvalued and very large logs undervalued. As with cubic measure, however, its values could be readily converted into board measure by the use of different factors for logs of different sizes.

It is now the uniform practice wherever the New Hampshire rule is in use to take 115 feet by the rule for 1000 feet of lumber.

SECTION IV

BOARD MEASURE

1. General. A board foot is a piece of sawed lumber 12 inches square and one inch thick, or any piece, as 3×4 or 2×6 , which if reduced to 1 inch thickness has 144 square inches of area. It is properly the unit of sawed lumber, and there must always be more or less difficulty in adjusting it to the measurement of logs.

There are a large number of rules in the country to-day purporting to give the contents of logs of given dimensions in feet, board measure. Among these rules there is wide variation in the value given to logs of the same dimensions. In the manner of their use, too, there is a good deal of divergence, resulting sometimes in dispute and loss,

The figures of eight rules in extensive use in the United States and Canada — the Scribner, the Doyle, the Decimal, the Maine, the New Brunswick, the Quebec, the Spaulding, and the British Columbia — are printed in this work (see pages 243–260). The International rule, devised by Dr. Judson F. Clark, formerly forester of Ontario, is also given (page 254). In regard to these rules and their relation to log measurement and saw product several general observations may be made.

(1.) On sound, smooth, soft-wood logs when manufactured according to the best present practice, the figures of all the commercial rules are conservative with the exception of the Doyle rule on very large logs. This is especially

true with reference to small logs.

(2.) Board rules give to large logs a greater valuation in proportion to cubic contents (actual amount of wood) than to small ones. Thus the Scribner log rule to 8-inch logs of small taper allows five feet per cubic foot of wood contents; to 16-inch logs seven feet, to 30-inch logs eight feet. This principle is a just one for logs that are in fact to be sawn, because the waste in manufacturing in the case of small logs is much greater, but on this account a board rule is not a just measure for logs designed for pulp or other such uses.

(3.) The rules are adapted to use on short logs with little taper. When logs are long enough to be cut in two for sawing, or to yield side boards for a part of their length, to derive contents from length and top diameter is not a fair thing. In such cases a second measure of diameter should be taken, and this can be done accurately only with a caliper. Allowance for "rise" or taper, whether for each log by judgment or according to some rule agreed upon, is more or less inaccurate and should be resorted to only in case of necessity. It may be said as a general rule that 20-foot lengths are as long as it is safe to scale logs in.1

On the other hand, since strongly tapering logs in almost every case are rougher than those of gentle taper, varying taper in logs of reasonable length is largely neutralized by quality.

(4.) There is wide variation in the details of scaling practice, and a trustworthy rule in consequence may, in the hands of an unskilled or careless man, give very unsatis-

¹ Except in the case of Pacific Coast timber.

factory results. In some matters, especially culling for defects, latitude must be allowed to the scaler. In general, however, practice is weak in the direction of strict mechanical accuracy. Reference is made to section VI following.

The method of construction, field of use, and relation to saw product of the above named rules are as follows:

2. Scribner and Decimal Rules. The figures of the original Scribner rule were obtained by drawing diagrams of the end sections of logs 12 to 48 inches in diameter and the boards which in the mill practice of the time could be sawed out of them. It is a very old rule and in wide use. As printed, extended down to 6 inches, it is the legal rule in the state of Minnesota.

Omitting unit figures of the Scribner rule and taking the nearest tens has given the Decimal rule, so called, legal in Wisconsin and adopted by the United States Forest Service.

3. Spaulding or Columbia River Rule. This rule was derived by similar methods as the preceding, $\frac{1}{3}$ inch being allowed for saw kerf. It is in more extensive use on the Pacific Coast than any other.

4. Doyle Rule. This rule was constructed by the following formula: — Deduct 4 inches from the diameter of

Diameter	No. Logs	Doyle Scale	Product	Overrun
6-8 in.	28	289	903	213%
7–9 in.	54	831	2159	159%
8–12 in.	101	2603	5471	110%
10-17 in.	104	6324	9976	58%
18-20 in.	90	15440	20215	31%
21-24 in.	126	30929	37744	22%
25-33 in.	31	11866	13368	12%

the log for slab, square \(\frac{1}{4}\) of the remainder, and multiply by the length of the log in feet. This is a very illogical rule and gives results widely varying from saw product in

logs of different sizes, though in a run of logs the results obtained may approximate a fair thing. Very small values are given to small logs, too small by far for normal logs economically manufactured, while beyond about 36 inches in diameter values are given that are above the product of the saw. It crosses the Scribner rule at 25 inches in diameter, the Maine rule at 34. A test made by Dr. J. F. Clark in 1905 in a Canadian band mill cutting sound, straight pine into boards resulted as shown on page 141.

The Doyle rule is in more general use than any other in the United States and Canada, and is the one printed in recent editions of Scribner's "Lumber and Log Book."

This rule has been combined with the Scribner into the Doyle-Scribner rule, the figures of the Doyle rule being taken for small logs where the Doyle figures are lower, and of the Scribner rule on the largest logs where these figures are less. This Doyle-Scribner rule has been used

largely on hard woods.

5. Maine, also called Holland Rule. The figures of this rule were derived from diagrams. That is to say, circles 6, 7, 8, etc. inches in diameter were plotted and within these the boards that could be sawed, an inch thick with 4 inch for saw kerf. Not only the boards derived from the inscribed square were reckoned, but the side boards if they were as much as 6 inches wide. No rounding off of the figures was done, so they are a little irregular, but that takes care of itself in a run of logs.

This rule is used largely in Maine and to some extent elsewhere. It has been carefully tested at the saw, and the conclusions are as follows:— Sound spruce and pine logs 12 to 18 feet long, of best merchantable quality, manufactured at a circular saw cutting 4-inch kerf will yield in the shape of inch boards just about the number of feet of edged lumber that the rule gives. A band saw will get more, and there will be a larger product if the logs are put into plank or timber. More will also be got the longer the logs run, up to the point where they are scaled in two pieces.

How sawing practice affects the product at the saw was clearly shown by a test made by the United States Forest Service in various spruce mills of Maine. Some results of this test are given in tabular form. All logs were straight and sound, and exact conditions were as follows:

Band Mill No. 1, \(\frac{1}{8}\)-inch saw kerf, lumber cut just 1 inch thick. Mill run for economy and utmost product of long lumber, giving product of about 40 M daily.

Band Mill No. 2, same saw kerf. Mill run for speed

rather than economy, product being 58 M a day.

Rotary Mill, 16-inch saw kerf, lumber even inch thick. Gang Saw, 12-inch kerf, lumber even inch thick, logs

sawed alive or through and through.

TABLE I. YIELD IN INCH BOARDS OF LOGS 16 FEET LONG AS SAWED IN DIFFERENT MILLS

Top Diam.	Band Mill No. 1 Logs turned	Band Mill No. 1 Sawed alive	Band Mill No. 2 Sawed alive	Rotary Sawed alive	Gang	Scale by Maine Log Rule
6 in.	30	26	20	18	24	20
7 in.	41	36	29	25	34	31
8 in.	53	47	39	35	43	44
9 in.	66	59	51	46	54	52
10 in.	81	73	64	59	67	68
11 in.	96	88	79	73	80	83
12 in.	112	106	95	89	94	105
13 in.	130	125	113	107	109	120
14 in.	149		133	127	126	140
15 in.	171		154		145	161
16 in.	196		178		165	179

TABLE II. PRODUCT IN INCH BOARDS OF LOGS OF DIF-FERENT LENGTHS AS SAWED IN BAND MILL NO. 1

Shows how in careful practice yield increases relative to scale as the logs are longer.

Тор		Lengths in Feet								
Diam.	8	10	12	14	16	18	20	22	24	
6 in.	13	17	22	26	30	34	39	44	50	
8 in.	25	32	39	46	53	60	68	76	84	
10 in.	39	49	59	70	81	91	101	113	124	
12 in.	54	68	83	97	112	126	141	156	172	
14 in.	73	92	111	130	149	170	190	211	232	
16 in.	95	120	145	170	196	223	250	278	306	

TABLE III. PRODUCT OF MILLS WHEN SAWING DIMENSION STOCK, MOSTLY 2 AND 3 INCH PLANK

Overrun is the percentage by which the product exceeds the scale of the logs as given by the Maine log rule.

Band Mi	ll No. 1		Rotary			
Lengths	Average Top Diam.	Over- run	Lengths	Average Top Diam.	Over- run	
16 ft. and under 17-20 ft. 21-24 ft.	10 in. 10 in. 8½ in.	24 % 23 % 37 %	16 ft. and under 17-20 ft. 21-24 ft. 25-28 ft.	10 in. 10½ in. 12 in. 9½ in.	0 % 6 % 11 % 15 %	

6. New Brunswick Rule. This is the legal rule for scaling lumber cut on the crown lands of New Brunswick, and is generally employed for log measurement in that province. Its values are somewhat below those of the Maine rule.

When logs of a smaller top diameter than 11 inches are to be scaled, it is done under the following rule: A 7-inch log contains 2 ft. B. M. per foot of length, an 8-inch log $2\frac{1}{2}$ ft., a 9-inch log 3 ft., a 10-inch log 4 ft.

One notable thing about the New Brunswick rule is that

taper is allowed for in lengths over 24 feet.

7. Quebec Rule. This is the legal rule for measuring logs in the province of Quebec. Values are close to the Scribner Rule; in many cases they are identical. The

figures were derived by plotting.

8. Theory of Scale Rules and Clark's International Log Rule. The theory of the measurement of saw logs in board measure has been more carefully studied by Dr. Judson F. Clark than by anyone else, and a rule called the International Log Rule was devised by him, on the basis of this reasoning, which he also tested at the saw. The main points in this study are as follows:

Taper of Logs. While logs exhibit a great variety of taper, it has been found (1) that rough logs taper more than clear, smooth logs, so that quality tends to neutralize taper; (2) that average taper does not differ greatly in different localities or with different species. This average taper as a result of much measurement is found to be safely 1 inch in 8 feet. This in modern economical mill practice increases the yield of lumber in the form of side boards, and the above stated allowance for taper is therefore introduced into the rule for all lengths over 8 feet.

Crook and Sweep. In this study due allowance was made for irregularity of surface, and crook averaging $1\frac{1}{2}$ inches in 12 feet of length, found to be characteristic of white pine logs on the Ottawa River, was counted normal. Above the limit of $1\frac{1}{2}$ inches in 12 feet, any given degree of crook was found to affect the product of small logs more than of large logs, and that in proportion to their diameters. That is to say, a crook of 3 inches in 12 feet throws out twice as great a percentage from a 10-inch log as from one 20 inches in diameter.

Shrinkage and Seasoning. Logs are commonly scaled green, while sawed lumber must hold out on a survey made when it is dry. In computing his rule Dr. Clark figured that boards would be cut $1\frac{1}{16}$ inch thick to allow for this.

¹ See Forestry Quarterly, Vol. IV, No. 2.

Saw Kerf. This loss in logs of different sizes is proportional to the area of their cross-section, or to the square of the diameter. It varies in proportion to the thickness of saw kerf as well. As embodying an average of good

present practice, 1 inch was allowed.

Loss in Edging Lumber. This includes not only that portion of a log which is thrown away in the form of edgings, but also the fractions of inches in the width of boards, which in Dr. Clark's studies were uniformly thrown off. It is counted to be in all logs proportional to the surface, or, what amounts to the same thing, to the diameter. Counting boards to be merchantable down to the size of 2 ft. B. M., Dr. Clark found that an allowance of .8 foot board measure for each square foot of surface under the bark, or, what amounts to much the same, a layer .8 inch in thickness around the surface, would justly allow for this waste.

Formula for the Rule. The above elements being put into mathemetical form with D representing top diameter inside bark, there is obtained for 4-foot sections the formula

$$(D^2 \times .22) - .71 D = contents B. M.$$

Adaptation to Other Conditions. The product for other widths of saw kerf than $\frac{1}{8}$ inch may be obtained by applying the following per cents:

For $\frac{7}{64}$ inch kerf add For $\frac{7}{16}$ inch kerf subtract For $\frac{4}{16}$ inch kerf subtract For $\frac{5}{16}$ inch kerf subtract For $\frac{5}{16}$ inch kerf subtract For $\frac{7}{16}$ inch kerf subtract For $\frac{7}{16}$ inch kerf subtract For $\frac{7}{16}$ inch kerf subtract 20.8 per cent.

Should the 1_6 -inch allowance for shrinkage not be made in the mill practice in question, this may be allowed for in a similar way. According to Dr. Clark's assumptions, each board with its saw kerf means 1_{16}^3 inch in thickness taken out of the log.

If mill practice in other ways is not so economical as the rule presupposes, that is to say, if logs are sawed with more waste in slab and edging than has been assumed, or if logs vary in taper and straightness from the standard, that is considered by Dr. Clark to be proportional to the surface or diameter, and he recommends that it be allowed for by making a comparison between the scale and mill product, and then adjusting the zero mark on the scale stick more than one inch from the inch mark on the stick in accordance with the results of that comparison. Dr. Clarke's rule will be found on page 254 in the same section with the other board rules.

SECTION V

THE NEW YORK STANDARD RULE

In northern New York logs are cut as a rule 13 feet long, and a log of that length and 19 inches in diameter at the top, inside bark, is the common unit of log measurement. It is called a "market" or "standard," and logs

of other dimensions are valued in proportion.

The "standard" is thus another artificial unit of log measurement, more artificial, perhaps, than any other here dealt with. Standard measure in logs of the same length runs very close to cubic measure. Thus a log 19 inches in diameter at the top and 13 feet long has 26 cubic feet in it; four logs 9½ inches in diameter and 13 feet long, also making one standard, contain the same amount of wood approximately, while a 38-inch log of the same length has four standards and 104 cubic feet of contents. A log 26 feet long, however, has more than twice the wood contents of a 13-foot log on account of taper. For that reason the use of standard measure outside of a region where short standard lengths are cut would be likely to make trouble.

Standard measure does not run parallel to board measure or to the yield of logs of different sizes at the saw. The standard log,—a log, that is to say, 19 inches in top diameter and 13 feet long,—scales by the Scribner rule 195 feet, and, in practice, five standards are often reckoned as the equivalent of a thousand. Four 9½-inch logs, together making one standard, scale but 144 feet by the rule, or seven standards to the thousand, and the actual ratio between standards and thousands is stated to run all the way from 4 to 14.

The ratio between cords and standards is nearly constant in logs of all sizes if cut of equal length. In the Adirondack woods 2.92 standards are commonly reckoned as one cord.

SECTION VI

SCALING PRACTICE

Logs are best scaled when they are being handled over, as on a landing or mill brow, for then all parts can be seen and got at. Measurement in the pile, especially for long logs, is both difficult and unsatisfactory.

1. Length. A tape worked by two men is an accurate measure of length. Short logs may be accurately measured with a marked pole, and for long logs a carefully adjusted wheel with brads in the ends of its spokes is cheap to use and reasonably accurate. Measurement with a four-foot stick has a very wide range of accuracy, according to the way it is done.



GERMAN NUMBERING HAMMER

Valuable timber cut into standard log lengths is commonly allowed two inches extra to permit trimming at the saw, this amount being disregarded in the scale. If logs are cut without measuring, in which case they are as likely to be ten inches over foot lengths as two inches, the extra inches are commonly thrown off just the same. That practice, however, means in 16-foot logs a loss of $2\frac{1}{2}$ per cent on the scale or the timber. On 30-foot logs, it means $1\frac{1}{2}$ per cent.

2. Diameter. The diameter measure for any board rule is obtained at the small end of the log and inside the bark. It is important in large and valuable timber that an average diameter be taken. In dealing with fractional inches,

there is a variety of practice. Some scalers read uniformly from the inch nearest the exact diameter; some disregard all fractional inches and take the next inch below; some vary the practice according to length and taper of the individual logs.

Probably, the most just practice to follow, as a general rule, is to throw off all fractions of inches up to and including one half inch, and to read fractions over one half as of the inch above. This practice, in logs under 16 inches in diameter, gives results from 7 to 10 per cent greater than if all fractions of inches are thrown out.

3. Culling for Defects. Defects in logs consist in irregularity of form, in shakiness, and in decay. Knots are not properly considered as defects, but as a factor in general quality. All these matters vary with the species, with the locality, and with the individual log. They are matters which have to be dealt with locally and individually, and little can be written that is likely to be of service and not liable to do more harm than good.

The curved or sweeping form is a common defect in logs. Scalers frequently have rules for allowing for it, but these differ so widely that they cannot be transcribed here. (See page 145 for the result of this defect in logs of

different sizes.)

Irregular crooks in logs cannot be classified. A man can sight along a log and estimate what proportion of it can be utilized by the straight cuts of a saw, and this guided by mill experience is the only way of dealing with the matter.

Seams caused by frost and wind form another class of defect, more frequent in northern woods and in trees grown on exposed places. Sometimes these are shoal and have little or no effect on saw product. Sometimes they reach nearly or quite to the heart of a log.

A fairly general practice on northern spruce cut for sawmill use is to discount 10 per cent for straight, deep seams, and for twisting seams up to 33 per cent, or even to throw

out the whole log.

It is to be remarked that these defects have, when reckoned in percentage, a far greater effect on small logs than on large ones. Thus a three-inch sweep in a 15-inch, 12foot log takes but a small percentage out of its total yield at the saw, while a 6-inch log with the same sweep is practically useless for full length, edged lumber. Again, strong taper may largely neutralize the effect of considerable irregularity in outside form. Lastly, in practical scaling, a certain amount of irregularity in outside form must be considered normal and be taken care of by the conservatism of the log rule.

Shakiness in logs is far more frequent in some species than in others. Thus hemlock is largely affected by it, while there is very little of it in spruce. In large measure, it should be considered as an element of quality, affecting the grade of the product, not a defect affecting the scale of the logs. When, however, a considerable section of a log is rendered worthless, it should be thrown off in the scale. How much to throw off is a matter of judgment and of mill

experience.

Decay may be complete, utterly destroying the value of a whole log or a section, or it may be partial, allowing the production of a low grade of lumber. Decay varies much according to species and locality, and it occurs in various forms. Of the northern soft-wood trees, fir is most liable to unseen defects,—a log perfectly sound to all outside appearance may "open out" very poor at the saw. To a less extent white pine in some localities is affected in the same way.

Generally, however, the ends of a log or some mark on its surface, such as rotten knots, "punks," and flows of pitch give indication to the practiced eye of defect beneath. How much to allow is then a matter of judgment based

on mill experience.

The following table ¹ has been made up, giving the loss due to round center defects extending through or affecting the full length of a log. For four- or five-inch defects, it amounts to the same thing as throwing out a scantling having the same side as the hole has diameter.

As stated at the start, careful mill training is the only safe basis for the correct culling or discounting of logs. Some scalers have that; some do not, and have to rely either

¹ Graves' "Forest Mensuration."

TABLE OF LOSS BY HOLES OR ROT NEAR THE CENTER OF LOGS, GOOD FOR DEFECTS MORE THAN 4 INCHES FROM THE BARK

Diam.	Length of Logs in Feet						
of Hole	10	12	14	16	18	20	
Inches	Board Feet						
2 3 4 5 6 7 8 9	5 0 14 20 27 36 45 56 67	8 11 17 24 33 43 54 67 81	7 13 20 28 38 50 63 78 93	8 15 23 32 44 57 72 89 107	16 25 36 49 65 81 100 120	10 18 28 40 55 72 90 112 133	

on arbitrary rules or on guesswork. Proper discount may vary greatly, too, with the mill practice and product. A mill with a box factory attached, or sawing round-edged stuff which is measured regardless of crooks, wastes little or nothing on account of defective form. For a mill which can market only three-inch deals at a profit, an entirely different system of scaling is appropriate.

SECTION VII

Thousands of unrecorded tests of scale rules have doubtless been made at the saw, using local and current scaling and sawing methods. During the last few years a number of such tests have been made under stated conditions so carefully guarded that they may serve a general purpose. Reference is made to the tests recorded on pages 143 and 144 of this work. The following also are reliable and of interest to northern workers in timber.

The wide variation in the yield of logs as sawed under different conditions is a matter of great importance in several ways to the worker in timber, chiefly, perhaps, for its bearing upon timber estimates. The relative competence of sawyers is one cause of this, and that, according to careful mill men, may readily amount to 10 per cent. Then market demand affects the matter, some mills being so situated that they can market only the larger sizes of lumber. The type of saw employed and the methods of handling on the carriage also have their effect.

TABLE I

Yield in inch boards, squared, of second growth white pine logs. Based on 740 logs; study by Harvard Forest School.

Growth extra tall and smooth; large and small trees in the stand, which was cut clean; logs with 2 in. crook or over thrown out. Sawed by circular saw cutting 4-inch kerf. In scaling, fractions of inches up to .5 were thrown off, fractions of .6 and over taken as if of inch above. Boards merchantable down to 2 feet, surface measure; some wane allowed.

Тор	Yield B.M.		
Diameter	12-foot Logs	14-foot Logs	
5 inches 6 inches 7 inches 8 inches 9 inches 10 inches 11 inches 12 inches 13 inches 14 inches 15 inches 16 inches 17 inches 17 inches	14 20 26 34 43 53 67 81 95 110 128 147 170 202	15 23 30 39 50 61 76 90 105 122 139 160	

A practice that in some localities of recent years has greatly increased the merchantable product of logs is that of sawing waney or round-edged boards. Portable mills in southern New England sawing lumber for boxes or finish follow this practice largely, and stationary mills in many localities have a box or other saw to which they can turn over the small and crooked logs for this most economical

form of manufacture. When boards in this form are surveyed they are measured at the average width, inside bark, on the narrow side, without discount for crooks.

This practice has brought about great economy in the use of timber, and when done with thin saws, has secured from logs a far greater product than current scale rules give. Several of the tables given herewith are of special interest in this connection. In all these tables top diameter means diameter of the upper end of the log inside bark.

TABLE II

Yield in inch boards of second growth white pine logs, sawed with a circular saw cutting \(\frac{1}{4}\)-inch kerf. Greater part of boards not edged, but measured for width at an average width, inside bark, on narrow side, without discount for crook.

Based on 1180 logs. From Massachusetts State Forester.

		Length of	Log — Feet	
Top Diam. Inches	10	12	14	16
inches	Vol. Bd. ft.	Vol. Bd. ft.	Vol. Bd. ft.	Vol. Bd. ft.
4 5 6 7 8 9 10 11 12 13 14 15 16 17 18	9 13 17 23 30 39 48 58 69 80 92 104 117 131	13 17 22 29 37 47 58 70 83 96 111 129 146 165 184	17 21 27 35 44 55 68 82 97 113 131 150 170 192 220	21 26 32 40 51 64 79 98 115 136 158 180 205 230 256

As the edged lumber was taken from the larger and straighter logs and after those logs had been sided on the carriage and turned down, the yield was probably as large as if all boards had been left round-edged.

TABLE III

Same logs but grouped according to mid diameter outside bark.

Mid		Length of Log — F	eet		
Diam.	10	12	14		
Inches	Contents — Board Feet				
5 6 7 8 9 9 11 12 13 14 15 16 17 18 19 20 22 23 24 25 26	7 10 15 22 28 35 44 53 61 70 82 95 109	8 13 19 27 34 43 53 64 76 88 104 119 136 155 173 193 211 235 256 281 304	10 16 23 31 40 50 63 77 91 106 125 144 163 184 204 226 247 273 298 328 355 384		

The figures of the above tables were closely confirmed, except in the smallest sizes of logs, by similar figures obtained by the U. S. Forest Service for the Forest Commission of New Hampshire. The saws in this latter test cut \(\frac{1}{2}\)-inch kerf; 60 per cent of the product was round-edged stuff, the balance being squared; 70 per cent of the lumber was cut 1 inch thick, the balance $2\frac{1}{8}$ and measured as 2 inches. In the sizes under 8 inches the Massachusetts mills cut somewhat closer.

TABLE IV

Comparison of Maine Log Rule and results of sawing as shown in Tables I and II. 12-foot logs.

Ì		Results of Sawing				
Top Diameter Inches			Round-edged Lumber Table II			
4 5 6 7 8 0 10 11 12 13 14 15 16 17 18	15 23 33 39 51 62 78 90 107 121 134 154 174	14 20 26 34 43 53 67 81 95 110 128 147 170 202	13 17 22 29 37 47 58 70 83 96 111 129 146 165			

TABLE V

Yield in $\frac{5}{8}$ -inch boards of pine logs 4 feet long (+ 2 inches for trimming).

Diameter	Yield	Basis
	Surface Measure	274515
4 inches	4 feet	3 logs
5 inches	6 feet	48 logs
6 inches	9 feet	121 logs
7 inches	13 feet	109 logs
8 inches	17 feet	75 logs
9 inches	22 feet	84 logs
10 inches	28 feet	40 logs
11 inches	34 feet	36 logs
12 inches	41 feet	21 logs
13 inches	49 feet	11 logs
14 inches	57 feet	6 logs
15 inches	66 feet	4 logs
16 inches	75 feet	6 logs

Log diameter taken at top end, inside bark. Saw kerf inch. Boards not edged, but measured at an average width on narrow side. From Massachusetts State Forester.

A cord of pine wood sawed and measured in this fashion yields about 1000 feet of box boards. Sawed one inch thick, it is counted by Massachusetts box board men to yield about 650 feet surface measure.

TABLE VI

Yield in round-edged boards of second growth hard wood logs 12 feet long cut $1\frac{1}{8}$ inch thick with circular saw cutting $\frac{1}{4}$ -inch kerf. Based on 1831 logs.

Grouped according to top diameter.

Grouped according to mid diameter.

Top Diameter Inside Bark	Yield, Surface Measure, of 12- foot Logs
4 inches 5 inches 6 inches 7 inches 8 inches 10 inches 11 inches 12 inches 13 inches 14 inches 15 inches 16 inches 17 inches	8 feet 11 feet 16 feet 16 feet 22 feet 30 feet 39 feet 65 feet 82 feet 100 feet 120 feet 141 feet 165 feet 192 feet

Mid Diameter Outside Bark	Yield, Surface Measure, of 12- foot Logs
6 inches 7 inches 8 inches 9 inches 10 inches 11 inches 12 inches 13 inches 14 inches 16 inches 16 inches 17 inches 18 inches 19 inches	11 feet 15 feet 21 feet 21 feet 29 feet 37 feet 49 feet 61 feet 75 feet 107 feet 126 feet 143 feet 143 feet 145 feet 187 feet 210 feet
14 inches 15 inches 16 inches 17 inches 18 inches 19 inches	91 feet 107 feet 126 feet 143 feet 165 feet 187 feet

From New Hampshire Forestry Report for 1905-1906.

SECTION VIII

CORD MEASURE

The exact legal definition of the term "cord" varies in different localities. For the present purpose it is a pile of wood 8 feet long and 4 feet high, with the top sticks rising somewhat above the line, the sticks themselves sawed 4 feet long or chopped so as to give an equivalent. Such a pile occupies 128 cubic feet of space. A cord foot is $\frac{1}{8}$ of a cord, or a pile 4 feet high, 4 feet wide, and 1 foot long.

The actual solid contents of the wood which a piled cord contains depends on a number of factors. First is the care used in piling, a matter which need only be mentioned here. Other factors are the straightness and smoothness of the wood, its size, assortment, and whether split or not.

In regard to the first of these factors, while it is perfectly evident that straight, smooth, well-trimmed wood must pile closer than its opposite, no hard and fast rules can be given. Taking round wood of given quality, the following rules can be laid down:

1. Large wood piles closer than small wood.

2. The same wood put up in one pile with sizes mixed occupies a little less space than if the larger and smaller

sizes are piled separately.

3. The effect of splitting varies much with the quality. Smooth, straight-grained wood when split may be packed into the same space that it occupied before. On the other hand, small or crooked wood when split piles much more loosely.

In regard to the actual solid contents of a piled cord,

the following rules will approximately hold.

1. Smooth, round wood 8 inches and up in diameter, such, for instance, as the best pulp wood, has .8 of its contents in solid wood or yields 102 cubic feet solid to the cord. White birch of best quality will yield nearly or quite the same.

2. Small pulp wood from 3 to 8 inches in diameter contains about .7 of its stacked volume in solid wood, or 90

cubic feet to the cord. Smooth hard wood yields about the same.

3. Still smaller round wood, wood that is crooked and knotty, and good split hard wood contains in solid wood about .6 of the outside contents of the pile or 77 cubic feet per cord.

4. Small, crooked wood cut from limbs may run down

as low as 27 solid cubic feet per cord.

5. ¹The longer a lot of wood is cut, the greater will be the vacant space left in piling. Fair sized pulp wood, for instance, which when cut 4 feet long will measure a cord, if cut in 2-foot lengths will pile up in 2 to 3 per cent less space. The same wood, on the other hand, if cut 8 feet long and measured in the pile will measure nearly 6 per cent more; if 12 feet long, about 12 per cent more.

Wood in thorough air-drying shrinks about 10 per cent on the average, hard woods as a rule more than soft. If wood checks and cracks freely, something like half the total shrinkage is taken up in this form. Two inches extra height in the pile are commonly allowed on green wood

in Massachusetts.

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To Measure Wood in Cords. When the wood is 4 feet long, measure the height and length of the pile in feet, multiply together, and divide by 32. The result will be contents in cords. If the wood is more or less than 4 feet long, multiply length, width, and height of the pile together, and divide by 128. If wood is piled on sloping ground, the length and height should be measured perpendicular to one another.

For measurement of logs into cord measure, see page 138. The French cord of the Province of Quebec is 8' 6" × 4' × 4' 3", containing, therefore, 144 cubic feet, as against 128 for the cord current elsewhere.

¹ See Zon on this subject in Forestry Quarterly, Vol. I, No. IV.

PART IV TIMBER ESTIMATING

PART IV. TIMBER ESTIMATING

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PART IV. TIMBER ESTIMATING

SECTION I

INTRODUCTION

METHODS of estimating timber vary greatly in different regions and with different men. They vary also with the value of the timber involved and with the purpose for which the work is done. In this last connection cost is a guiding principle; in general, that method of doing a piece of work is best which secures a result sufficiently accurate for the purpose with the smallest expenditure of time and money.

Lump Estimate by the eye has not gone out of use, and in fact never will cease to be employed. The immediate judgment that a good lumberman forms, simply by walking through a piece of timber, that it contains a hundred thousand, a million, or ten million feet, is for many pur-

poses close enough to the mark.

Similarly an experienced man, in timber of a kind with which he is familiar, forms an idea by direct impression of how much a piece of land will yield per acre. The men who can do that are more numerous than those who are able to judge the whole piece. The faculty is easier to acquire, and in general the results are safer and more reliable.

Such estimates as these are indispensable in actual business. Frequently they enable a man to pass correctly upon a proposition for purchase or sale. But while their necessity and their reliability within limits may be admitted, no illusions should be indulged in with regard to them. For one woodsman who can actually give a close and reliable estimate after these methods, there are many who only think they can; nothing is better known in the timber business than widely variant and totally erroneous estimates of standing timber. Further, a man who uses these methods is frequently very lame when he gets into a country with which he is unfamiliar. Lastly, when time consumed and training involved are considered. estimates of this nature may not be the cheapest by any means.

There is a general tendency among timber estimators, commendable in the main on the ground of safety and conservatism, to put their figures below the mark. As for the general degree of accuracy obtained, there seems to be no reason founded on experience this side of the Atlantic to greatly change the verdict of experience in Europe 1 that good and experienced men in timber with which they are familiar are liable to errors up to 25 per cent.

It is true, moreover, that the weakness of these traditional methods is generally recognized. More careful and elaborate methods are in fact practiced in many sections of the country, and the area is fast extending in which the treatment demanded by the situation is not

really an estimate but a survey.

SECTION II

INSTRUMENTAL HELPS

The helps that may be used in the survey of standing timber are as follows:

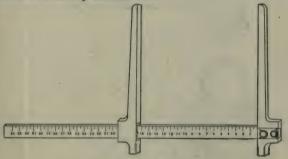
1. FOR DIAMETER MEASUREMENT

Calipers for measuring the diameter of trees may be constructed by the woodsman himself, or they can be purchased of dealers. The best are made of light-colored hard wood and have the inches plainly marked on both flat sides of the bar. The jaws are detachable for convenience in transportation, and the sliding arm is so fitted with adjustable metal bearings that it is truly square and gives a correct diameter when pressed firmly against a tree or log.

Substitutes for the caliper, useful in some circumstances, are the Circumference Tape, a steel tape so graduated that when a circumference is measured a diameter is read.

¹ Schlich's "Manual of Forestry."

and the **Biltmore Stick**. This last is in construction a wooden bar of about the dimensions of an ordinary scale rule; in use it is held horizontal, tangent to the tree being measured, and at the natural (but a constant) distance from the eye of the observer. Then, one end of the stick being aligned with one side of the tree, where the line of sight to the other side cuts the stick it is graduated for the given diameter.¹ Both instruments have proved serviceable on the Pacific Coast, where the timber is so large that a caliper is cumbersome, and because of their portability they have a field of use elsewhere. They are not, however, as quickly manipulated as the caliper in steady work on timber of ordinary dimensions.



TREE CALIPER

2. Counter or Tallying Machine. Timber Scribe. Bark Blazer

These simple little instruments, the last of which can be home-made if necessary, are very serviceable in forest work, particularly in timber estimating.

3. THE DENDROMETER

The dendrometer is an instrument for measuring the diameter of a tree at a considerable distance above the ground. There are several forms of this instrument, most of them costly and complicated, that are employed in scientific investigation. With these the practical woods-

¹ See Appendix on theory and accuracy of this instrument.

man has no concern. Such a man when he wishes to know the diameter of a standing tree at a point out of reach will ordinarily either estimate it or cut the tree down.



BARK BLAZER



Occasionally, however, timber may be met with which is of sufficient value for special purposes to require measurement in this way. In such a case the engineer's transit may be employed, and by its aid it is not a difficult matter to determine either the height at which any given diameter is attained or the diameter at any given height. A very simple little instrument for diameter measure-

ment has been devised, which is described by its inventor as follows: 1



"The Biltmore pachymeter is used in connection with a target or piece of board graduated in inches, marked

¹ Forestry Quarterly, Vol. IV, p. 8.

black and white, which target is fixed horizontally at any

point desirable at the base of the tree.

"The instrument itself consists of a piece of metal about 18 inches long and $1\frac{1}{2}$ inches wide, containing a longitudinal slot about $\frac{1}{4}$ inch wide and 17 inches long. The edges of this slot must be strictly parallel. Its actual width is entirely irrelevant from the mathematical standpoint.

"It might be stated that any stick or pole, even a walkingcane, having parallel edges, will answer the purpose of establishing and measuring upper diameters. The Biltmore pachymeter is merely a device convenient to handle.

"The observer holds the pachymeter pendulum fashion by the hand of the outstretched arm in a position parallel to the tree trunk, and moves the instrument backward or forward until the edges of the slot cut off even with the desired diameter shown on the target. Then, the eye following upward along the trunk and sighting through the slot, that point on the tree bole is readily obtained where the bole cuts off with the edges of the slot. The position of this point above ground can be ascertained easily with the help of any hypsometer."

SECTION III

HEIGHT MEASUREMENT

There are many methods of measuring the height of trees. As serviceable as any are the following:

1. Windfalls are often of great assistance in ascertain-

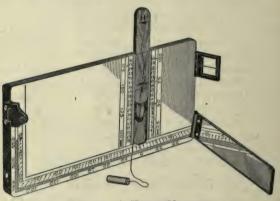
ing the height of timber.

2. A pole 15 or 20 feet in length may be set up alongside the tree to be estimated and then, standing some distance away, the cruiser may run his eye up the tree and judge how many times the length of the pole will be contained in it. A pencil held erect at arm's length in range of the pole and then run up the tree will help the eye in making the judgment.

3. A cane or staff may be used on the principle of similar triangles. Hold the staff firmly in the hand with the arm straight and horizontal. Swing the end of the staff down

by the face and adjust the hold till the end of the staff just comes by the eye. The distance from the eye to the staff and from the hand up to the end of the staff are now equal. Go off from the tree to be measured, holding the staff erect, until you can sight by the fist to the base of the tree and by the top of the staff to the top of the tree. Pace or measure to the tree and this will give its height.

4. The Abney clinometer, shown on page 93 of this work, may be used for height measurement in much the same manner. Set the level tube at an angle of 45° with the line of sight and go off from the tree on a level with



FAUSTMANN'S HEIGHT MEASURE

its base until, sighting at the top of the tree, you see by the bubble that the tube is level. The distance from the observer to the tree is then equal to the tree's height.

5. A second method employing the same instrument is as follows: Stand at a point where both the top and the base of the tree can be seen and at some convenient distance from it, as 100 feet. Sight to the top of the tree and observe the angle of inclination, and again to the base of the tree, observing that angle also. Go into the table of tangents with the angles in turn, find the decimals corresponding, and multiply by the length of base. The sum of the two figures is the total height of the tree.

Example: Standing 80 feet from a tree, the angle to the top is found to be 31° and that to the base $8\frac{1}{2}$ °, of depression. From the tables the tangent of 31° is found to be .6009; multiplying this by 80 gives 48 feet for the height of the tree above the level of the eye. Again the tangent of $8\frac{1}{2}$ ° is found from the tables to be .1495 and this multiplied by 80 gives 12 feet. 48+12=60 feet, the total height of the tree.

6. Faustmann's height measure works in much the same manner, but gives the desired height directly without the use of tables. This instrument may be had of dealers at a cost of from \$6.50 up. It is compact, not complicated, and will be found of great service in estimating.

SECTION IV

VOLUME TABLES AND TREE FORM

A competent woodsman can tell from the looks of a tree somewhere near what it will scale, saw out, or yield in cord wood according to the practice with which he is familiar, and this without any measurements. Or a caliper may be used instead of the eye for diameter, and some kind of determination made of the height of the tree or the length and size of the logs into which it may be cut. The point of such judgment and measurements as a rule is their wider application. The single tree so examined is taken as the type of many, and the stand of an acre or of a considerable territory is thus estimated.

In this process the assumption is made that trees of the same dimensions are approximately similar in shape, while for the individual tree the fundamental factors determining contents are recognized as height and diameter. These two factors in any kind of timber work cannot possibly be disregarded. Whatever the scaling or mill practice of a locality may be, and into whatever form a tree's trunk is dissected before manufacture, the height of the tree and its diameter at some point near the base are the chief factors determining contents. These factors, consciously or unconsciously, are in the mind of every estimator.

Scientific study of tree form began by making the same assumption and selecting the same factors. While it

was known that single trees depart widely from the type, it was assumed that for trees having the same diameter and height an average volume could be ascertained which would hold approximately throughout the distribution of the species. Proceeding on this assumption, tables were worked out for the different tree species and these when applied in actual business proved close to the fact and vastly improved the work of timber valuation in Germany a hundred years ago.

European measurements of logs and standing timber do not recognize anything corresponding to the board foot, but everything is reckoned in solid contents. The same rule holds in the scientific study of tree form in all countries where it has been pursued, the unit in the United States being the cubic foot. For all such studies, too, the total height of the tree as a well-defined factor capable of ready measurement has usually been employed rather than any size limit set part way up, and a diameter breast high, or 4½ feet above the ground, has been settled upon as the basis of all diameter comparisons. The area of a cross-section of a tree at this point is called the basal area, and the same term is applied to a number of trees or to a stand of timber. In the study of tree form, the term form factor has proved to be a useful one. The form factor of a tree is the percentage which the volume of any tree (usually reckoned in cubic feet, outside the bark) makes of the volume of a cylinder having the same height and the tree's breast diameter. Illustration: A tree 15 inches in breast diameter and 75 feet high may, after caliper measurement every 4 feet along it, prove to have 38.6 cubic feet in it. A cylinder of these dimensions contains 92 cubic feet. The form factor, therefore, is .42.

For many years past the study of tree form has been ardently pursued, and many interesting facts and laws have been ascertained. In large measure these results have been brought to bear on the actual business of European countries where timber is grown as a crop under uniform conditions. In this country, where the forests are natural and as a rule irregular, it will be many years before the same can be true. The following, however,

may for one reason or another be of interest to the worker in timber:

(a) Near the ground a section taken lengthwise of a tree is concave outward, due to the swell of the roots. Above that, to a point somewhere near the lower limbs of a forest-grown tree, the stem has almost a true taper. From the lower limbs up, the form is roughly conical, with a sharper taper than below, the taper usually increasing toward the top.

(b) Of two trees having the same breast diameter, the shorter will usually have the larger form factor. This results from the relation just mentioned. Of two trees having the same height, the stouter, more openly grown tree will usually have a little larger form factor than the

other.

(c) Of two trees having the same dimensions, the older one, as a rule, has the larger form factor. The effect of other conditions of growth can seldom be clearly traced.

(d) Different soft wood species do not differ from one another so greatly but that a volume table made for one

may for some purposes be used for others.

A large form factor in all these cases simply means that the given tree more nearly approaches the form of a cylinder, or, in other words, that it has a large amount of wood for its height and diameter. That carries with it more scale, more sawed lumber, or more cord wood.

A table giving the contents of trees of stated dimensions is called a Volume Table. For scientific purposes solid content is given, standard measure, but a table may be worked out in cords, board feet, or any other unit required. The tables employed by European foresters at the present day are worked out commonly on the basis not only of height and diameter but of age classes or of some other determining factor, and they have proved to give the contents of standing timber very accurately.

Tables of this kind have also been frequently devised for estimating in this country. Usually these are local, worked out in the timber of the region in question according to local scaling methods; often also allowing the cull which is found to characterize the region. Such volume tables have frequently been based on diameter alone. In other cases - and this is essential unless a region is very uniform in its timber growth - height has been taken into consideration as well.

Thus many western and southern cruisers have made up tables giving the contents of trees of each inch in diameter and yielding 2, 3, 4, etc., logs as these would be cut in local practice. Again, an old Adirondack manager made up a table showing the number of spruce required per cord of pulp wood for trees 7, 8, 9, etc., inches in diameter, and short, medium, or tall, as the case for his region might be. Local volume tables, thoroughly based and used correctly, are the most reliable kind.

General Volume Tables for business purposes are of two varieties, the trees being classified either by total height or by length of merchantable timber. The assumption on which the first is based, that trees which have the same diameter and total height do not, when taken in numbers, vary in form throughout the region of their distribution, may, with a caution on the matter of age.1 be considered safe for most purposes. It is true, however, that some Pacific Coast timbers, with a very variable thickness of bark and the root swelling of large trees running above a man's height oftentimes, have to be handled with special caution.

The other variety of tables classifies trees in height by the number of standard log lengths they will yield or the height at which their boles attain a specified diameter. Under this plan the point to be observed is brought nearer the estimator. It is not, however, as sharply defined a point as in the other case, while, as explained on pages 274-275, special opportunities for error arise through vari-

ability in lumbering practice.

Another matter that has to be reckoned with in the valuation of standing timber, and which becomes in some species and regions a consideration of great importance, is defectiveness in quality. This no general volume table can allow for. It has to be worked out for each locality according to the judgment or experience of the estimator.

¹ See pages 161, 262, and 277.

Thirdly, a general volume table given in units of merchantable material assumes certain standards of lumbering practice. In one region, or on a property carefully handled, stumps may be sawed close to the ground, tops taken up to a small diameter, and every economy employed in cutting to advantage the material between; while in another region, or on another property, a large percentage of the wood of every tree cut down may be left to rot on the ground. Similarly in the mill there is great variety of practice, - location, equipment, market requirement, and men's capacity all having their effect here, as was explained and illustrated in earlier pages of this work. Then the question may not be at all of saw practice, but of the results of scaling, and here, as every lumberman knows, there is the widest diversity. The scale rules in actual use differ from one another in the values they give to the same log, in some cases by a ridiculous amount, while the practices that have grown up in their application are in some cases entirely artificial. Details need not be entered into here — a word to the wise is sufficient — but an example will bring the fact home. The Maine log rule, for instance, is believed by many to be the best commercial rule on the market, agreeing closely with the results of good saw practice; yet a Penobscot mill man once testified before a legislative committee that buying 26 million feet of logs by market scale for a season's stock, he sawed 30 million feet of long lumber out of it and slabbed heavily for a pulp mill besides.

Of the volume tables included in this work it may be said that their basis is clearly stated, including the number of trees involved, the standards of cutting and mill or scaling practice assumed, and the responsibility for the observations. They can, therefore, to a large extent be changed over to suit practice of another type. The tables original with this work, those for spruce and white pine, are based on figures taken from a large number of trees. These came from a wide range of country, and the computations show that no clear difference of form was introduced by the element of locality. Each tree was computed separately for its volume in the units desired (cubic feet,

board feet, or cords); the results have been averaged, evened by curves, and then the board-foot tables have been discounted by a small percentage to allow for normal defects of form and quality. Cutting practice that is economical, but not extreme, has been supposed throughout, the idea being to get, as nearly as possible, a conservative figure for good and economical practice.

In applying all these tables, considerable defects must be allowed for in the form of a discount. It is further to be clearly understood that they apply to timber as it runs and may be considerably off as applied to single trees.

In volume tables for hard woods merchantable length is in most cases preferable to total height as a factor because these trees characteristically spread out at the top, at once rendering total height hard to measure and destroying utility for lumber. Such tables also, because of greater irregularity of form and greater liability to defect in hard woods, are in general less trustworthy than soft wood tables. Several "graded volume tables," classifying the yield of trees by lumber grades, are in existence, but their utility apart from the local conditions in which they were constructed does not seem clear.

The way in which these volume tables may be tested and made to conform to the practices of any given locality is illustrated as follows:

A spruce property is to be explored on which cutting and scaling methods are as follows: — Timber runs up to about 20 inches in diameter and 75 feet in height; trees are cut down to the size of 12 inches on the stump or 11 breast high. Logs cut for saw lumber, one log from a tree, cut off where it will scale best. Logs are therefore seldom over 40 feet long and run from that down to 28 or 30. Scaling done with Maine log rule. If a log is 26 feet long or under, it is scaled as one log with the top diameter inside bark; if 27 to 30 feet, as two logs of equal length giving the butt log an inch larger diameter than the top; from 31 to 35 feet in the same way but allowing 2 inches "rise," and 3 inches on log lengths of 36 to 40 feet. In addition a level discount of 10 per cent is made on all logs to cover defects.

A half day's time spent following the logging crew and

examining trees as they are felled results as follows:—20 normal trees 17 to 20 inches in breast diameter when scaled by the above methods give 4730 feet B. M., while trees of the same dimensions are given in the volume table on page 238 5720 feet. The actual scale, therefore, is 17 per cent less than the tabular values.

24 trees 14 to 16 inches in diameter which by the table should yield 4080 feet scale up 3480, or 15 per cent less.

30 trees 11 to 13 inches in diameter that by the table should yield 4380 feet, actually scale 3500, or 20 per cent less.

The figures of the volume table may now be reduced by these percentages in those heights and sizes where on the given job the figures are required. The working table will then be as follows:

Breast Diam.		Feet in Height						
Inches	50	55	60	65	70	75		
11 12 13	52 60 72	56 68 80	64 80 92	72 88 100	84 96 112	92 108 125		
14 15 16	85 100	100 115 130	110 130 143	125 145 155	140 160 175	155 175 190		
17 18 19 20		142 155 175 195	158 175 195 220	175 195 215 245	190 210 240 270	210 230 265 295		

SECTION V

PRACTICE OF TIMBER ESTIMATING

The methods that should be employed in a survey of standing timber depend on a great variety of facts of which the main ones are these: the size of the tract to be examined, the method and fineness of its subdivision, the variety in its stand of timber, the value of the timber, and the experience and qualifications of the estimator. These methods are best discussed in two divisions, — first, methods for small tracts with valuable timber as a rule; and second, those for large tracts where more extensive processes must be employed.

A. SMALL TRACTS

1. In the case of very valuable timber it may pay the owner or purchaser to examine each tree individually, ascertain its contents carefully, and study it for defects. The net contents of each tree as so ascertained will then be put down separately in the notes, and in case several parties are interested, each tree may be stamped with a number to correspond with one in the notes. At any rate, blazing each tree examined is a good means to make sure that all are taken and to prevent measuring any twice.

Such procedure as this is appropriate to very large and valuable pine or to valuable but over-mature hard woods, which are especially liable to be defective. Volume tables might help in such cases, but they cannot be fully trusted; a scale rule at hand would be to many men of quite as much assistance. For instruments, a caliper would come in play along with an instrument to measure heights accurately, while use might be found for some form of the dendrometer. But the best part of the equipment of the estimator in such cases is local experience in cutting

and sawing the same class of timber.

2. When timber in good stand and of considerable value is involved, it may be advisable to caliper each of the trees and measure a sufficient number to obtain the range of heights. After the stand is measured, sample trees of different sizes may be estimated after careful examination, or such trees may be felled and measured. Better than either of these methods, however, is a volume table giving the yield of trees of the given kind and dimensions. Volume tables, however, cannot be depended on to allow justly for defects. That is a matter for the judgment of the estimator.

The above method works well in woods of approximately even type. When, however, the stand has a great variety of form and quality, the difficulty in making a true valuation is greater. In that case it may be practicable to cut it up into nearly homogeneous parts.

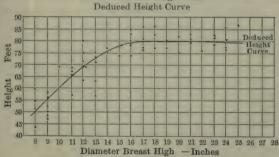
The following example taken from practice will illus-

trate the methods of working in a simple case.

Estimate of about 7 acres of land, covered nearly throughout with white pine standing fairly evenly, but not as a rule very dense. Concluded after inspection that no such differences of type or

	Field	Observations	Comp	uted Vol	umes
Breast Diam.	No. Trees	Observed Heights	Deduced Height	Scale Each	Total Scale
8" 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26	85 70 70 75 78 69 66 81 71 63 63 52 47 32 12 11 6 8	51-47-50-54-59 50-47-52-48-56-57 69-55 56-56-66-67-68 72-75-69-80-69-63 57-65-71-75-73 74-78-80-79-83 74-80-85 77-77-86-81 77-83-86 80-77 75-82 79-83-81 76 79-82-83 77-86-77-82 87	50' 55' 60 65 69 73 76 78 80 80 80 80 80 80 80 80 80 80 80 80 80	50' 70 95 130 96 162 203 245 290 337 405 446 525 570 620 665 715 770	4250' 4900 6650 9750 12636 14007 16170 23490 23785 23310 25515 23140 21855 17800 6840 3990 5720 23310

Plot of Observed Heights and



form existed as to call for differentiation of treatment. Instruments employed, caliper and Faustmann's hypsometer. Steps of the survey as follows:

a. Merchantable trees (those 8 inches and over in diameter breast high) calipered and scored in inch diameter classes.

b. Some 60 heights measured with the hypsometer. These might have been averaged for each diameter class, but a better plan is to plot all the heights on cross-section paper and draw a curve through them as in the accompanying sketch. From this curve the average height of the 8-inch trees is read off as 50 feet, of the 9-inch trees as 55 feet, and so on. The larger trees of the grove, those 16 inches and over in diameter, averaged 80 feet in height.

c. From the proper volume table the contents of a single tree of each size class is now taken and multiplied by the number of trees in the class. For the tract in question Table No. 4 gives the figures wanted, the product of the trees in boards, both round-edged and square-edged lumber. In this table the contents of a tree 8 inches in breast diameter and 50 feet high is given as 50 feet B. M.; that of a tree 9 inches x 55 feet, 70 feet, and so on. No discount appearing necessary for defects, by addition of the contents of the size classes the total stand of the lot is obtained. This comes to 253 M feet, of which in the practice of the locality 20 per cent may be sawed into good plank, 30 per cent into edged boards, and the balance of 50 per cent, the smaller trees and rougher logs, put into round-edged box-board lumber. The recorded figures, the plot and height curve, and a table showing the way the figures are put together, are given on the preceding page.

The estimate after this fashion of 250 M feet of timber of this size is a light day's work for two men. Three men

form an economical crew for big jobs.

3. In the valuable timber lands of the Lake States and South it is customary to estimate each forty acres by itself, and the methods of estimation frequently cover the whole stand. Pacing is largely used as a measure of distance, and the cruiser is generally equipped with some kind of volume table giving as often as not the board contents of trees of different diameters yielding 2, 3, 4, or 5 16-ft. logs. Usually two men work together. In that case the helper may run a compass line across one end of the "forty," ten rods or so from its boundary, leaving marks enough so that on the return trip it can be followed. Through the strip so cut off the cruiser circulates, keeping watch of his other bound and scoring down, as he passes, the merchantable trees according to species and in appropriate classes. As a rule very little measurement of height or diameter has been done in the past. The two men keep abreast of one another. When one strip has been covered another is taken in the same way. After the whole "forty" has been covered addition of the

figures obtained gives its timber stand. In well-timbered land two to four "forties" a day can usually be covered

by these methods.

In recording the results of such an estimate the size and quality of the timber are of course noted as well as its amount, and general notes on the growth, topography, and lumbering conditions of the land are also recorded. Following are sample notes of such an exploration:

Twp. 29 N. R. 7 W. S. E. ‡ of S. E. ‡ of Sec. 8.

White Pine, 7 logs average to M.; 30% uppers
Norway Pine, 8 logs to M.

Hemlock, 11 logs to M.

Basswood, 7 logs to M.

Maple, 14 logs to M.

Total

Total

Land slopes to North. Clay soil; very stony. Two ravines running N. W. and S. E. through the "forty." Tamarack swamp of about five acres in N. W. corner.

Another method of timber cruising carried out by one man alone is described as follows in the "Woodsman's Handbook":

A "forty" is 80 rods square. The cruiser who uses the method now to be described has found by trial that 500 of his natural

paces are required to go 80 rods. He begins at the corner of a "forty," say at the southeast corner, and steps off 125 paces on the south line, and so covers one-quarter of the side. He then stops and, facing north, counts the trees of the "forty," first to an estimated distance of 125 paces on the right hand, and then to an estimated distance of 125 paces on the left hand, and in each case to a distance

Plot	VI	D1-4	1	
Fiot	V I	Plot	1 V	
Plot	VII	Plot	IV	
Plot	VIII	Plot	III	
Plot	IX	Plot	II	8
Plot	X	Plot	I	100 paces
			125 paces	

of 100 paces in front of him. thus including the area represented in the diagram as Plot I. He then steps north 100 paces, and in the same way counts the trees in Plot II, and repeats the operation successively for Plots III, IV, and V. He has then a complete count of the trees on the eastern half of the "forty." He then walks west 250 paces along the north line of the "forty." Facing south, he now counts all the trees on Plots VI, VII, VIII, IX, and X in the same way as before, and thus completes counting the trees on the entire "forty."

There is, of course, great variety in the details of the work as practiced by different men, and a plan that is really inadequate may be effective nevertheless because of the ability of the cruiser. Such a method as the foregoing cannot be called a survey. It is an estimate purely, depending on the training of the cruiser and subject to the errors which change in his condition and his surroundings introduce. Nor does the fact that all the area is supposed to be covered give assurance on the matter of accuracy. It may indeed set up a standard too difficult to be actually carried out, so becoming a source of additional error.

4. The following, from an old Michigan cruiser whose work has been largely in hard woods, serves to introduce the principle of covering a percentage of the tract to be estimated, a principle more fully illustrated in connection with large tracts on later pages.

I have been a surveyor, engineer, "land-looker" since boyhood, and the system that I use is based upon the information that I have been able to pick up along that line during that period. The work has carried me to the forests of nearly every state that counts forest products among its most important assets.

The usual object of an estimate is to fix a value that can be used as a medium of exchange, although I have recently been called upon to estimate many tracts just before the commencement of logging operations in order to ascertain what the probable

product would be.

The report of the cruiser is required to show the log scale of a given tract, also the amount of tan bark, cord wood, telephone poles, railroad ties, etc., — in fact the entire forest product that is of value. This must be not only of standing timber, but of down timber that has a value as well.

His report must also show the topography of the tract, and the channels through which the product must be passed in the course of its transportation from the land, whether by railroad, water, or

logging road.

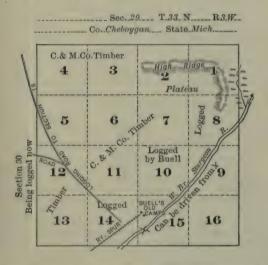
This work must be based upon some system that will eliminate so far as is possible all guesswork. There are many systems of cruising now in use, each of which has its advocates. I do not know of any other cruiser who is using the same system that I use, perhaps for the reason that I have made it up from my own work.

In my work I use a tree caliper. I have a book printed especially for the tally of the trees as I call them off to my assistant. I have also a form of report blank made to fit the rest of the scheme.

You will note that I number each forty-acre parcel in an undivided section on the same plan that sections are numbered in a township, except of course that there are only 16 lots in this case.

Hereafter the term "lot" applies to a forty-acre tract.

Arriving at the tract to be examined, I usually first go entirely around the area so as to discover if there are any high ridges, and if so to determine their course; also to see whether or not the tract is all timbered, and to locate any vacant areas on its outer edges. While making this circuit we mark points at each 125 paces on the boundary. If the land is uniformly level, it is immaterial at which point on the boundary line the work is commenced. If the tract is very rolling, the strips taken must be run so as to cross the ridges at as nearly right angles as is possible.



Suppose we are at the southeast corner of the section and that we have an entire section of fairly level land to examine. My pacer and compassman (I have but one assistant) steps off 125 paces, say in a westerly direction, along the south line of lot 16, starting from the southeast corner of the section. This brings us to a point 20 rods west of this corner and a line drawn directly north from this point should be parallel with the east line of the lot, also parallel with the center line, if one were in existence, and 20 rods distant from each of them. We proceed north from this point. At 50 paces the assistant halts, gets his tally-book and hard pencil into action, and jots down each tree as I call them off to him. He heads the vertical columns with the varieties of timber common to the tract and tallies each kind under the proper heading.

		Examination Lot. 1						29
	Ma	de by						y, 1908
C. L.	Ma	ple		Bass	Beec	ch	He	mlock
12 - 1 12 - 2		48 160						
13 - 1 13 - 2	1	19 64						
13 - 8	HU	180-		50	HJU IIII	450	,	50
14-2		410				110	1	110
15 - 1 15 - 2	HH HH I	216 1320			///	300	1	72 240
15 - 3 16 - 1								
	HA HA HII	864 2520	1	180	1111	576 540	///	432
16 - 4			1					

As soon as the assistant reports that he is ready I take the nearest tree and put the calipers upon it at a point where it would be cut in ordinary logging operations. I then walk around the tree and "size it up" generally to find any defect that may exist, also to judge how many 16-ft. logs would be cut from this particular tree. Suppose it is a maple and that it calipers 22 inches, and that it will yield a 48-ft. stem or three 16-ft. logs. I call to my pacer "Maple, 22—3," and he tallies in the maple column opposite the 22—3 of the figures in the left-hand margin of the page. In this way we get a record of every tree in a strip 4 rods wide, 2 rods each side of our compass line. My caliper blade is graduated to 57 inches from the stationary arm, just ½th of two rods, and if there is any question as to a tree's being in the strip it is very quickly settled by taking seven lengths of the caliper blade as I walk toward

the tree from the compass line.

Having taken the trees to a point a little in advance of my assistant, he proceeds on for 50 paces more and the calipering process is repeated. If the undergrowth is of sufficient density to prevent our seeing any large pine, bit of cedar swamp, or anything else that we should see, we make frequent explorations from the end of each 100 steps, my assistant going in one direction at the same time that I go in the opposite. No trees are measured in these side explorations unless we find something that is not common to the entire tract. Having returned to our line we proceed north, halting at each 50 steps to take the timber, also to note any ridges, logging roads, streams, springs, or other points that should appear in the report. When we have arrived at 500 paces my assistant changes his tally to lot 9 and we proceed north in the same way, changing at 1000 paces to lot 8 and at 1500 to lot 1. At 2000 paces, if the section is "full" we should be at the north line of the section, at a point 20 rods west of the northeast corner. As it rarely occurs that our compass line has been so accurate as to bring us out at exactly this point, we find the mark made during

our circuit of the section and pace from it westerly along the north line of the section for 250 paces, 40 rods. This brings us to a point from which a line drawn south will be parallel with the center line of lots 1, 8, 9, and 16, and with the west line of these lots and 20 rods distant from them. We proceed south on this line, taking the timber in the same manner as we took it in going north in the east half of the same lots. Arriving at the south side of the section we again go west 250 steps and then north through the easterly half of lots 15, 10, 7, and 2, and so on until the section is completed. A single "forty" or "eighty" or any sized tract is handled in the same way. This gives a caliper measure of every tree on 4 acres of each lot or on 10th of its area. Should a closer estimate be necessary the strips are taken every 10 rods instead of 20 rods, which gives 1th of each lot. If there are places in the tract from which owing to any cause the timber has been removed, the area must be shown on the report and proper deductions made from the estimate. If these vacant areas are crossed by the strips, care must be taken that they are not crossed lengthwise, as that would lessen the estimate too much; on the other hand, if they are crossed properly no deduction need be made from the tally.

When the calipering of the trees on the tract is completed the contents of the trees tallied are taken from the volume table, the scales footed, and the several footings multiplied by 10 or 5 accord-

ing to the number of the strips taken.

My volume table is of my own making. During the last twenty years I have been called upon very frequently to measure trespass until measures have been taken of thousands of trees of each diameter. This work has been done in every section of the State in which hard wood has been cut during that period, and has been added to at every opportunity that has offered. The stumps were calipered by taking the measure both outside and inside the bark; the length of the stem was taken, together with the diameter of the top, inside the bark. On this basis the log scale was made according to the Doyle rule. The scale of trees of the same diameter and even of the same stump diameter and length vary considerably on account of the different tapers toward the tops, making it necessary to get a large number of trees from which to work up a table. The average of the total scale of all the trees of a certain diameter has been taken as the amount of scale to be allowed for all trees of a certain stump diameter and height.

The results of the work as I have stated have been very satisfactory. Many of the tracts have been cut the same season that we made the estimate, and the log scale is usually from 10 per cent to 20 per cent above my estimate. I should not care to get much nearer than this. It would not be safe, as some firms cut the timber much more closely than others, depending upon the article to be made from the timber, the disposal of the waste product for

fuel, and so on.

No accurate estimate can be made without the use of the caliper. It entirely eliminates all favoritism on account of ownership or employer, and it makes possible a close acquaintance with the trees which shows up the defects. No cruiser sees the timber alike every day. His judgment varies as the man himself varies each day. The caliper eliminates this trouble, as it always measures the trees just as they are.

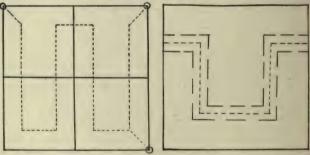
Care should be taken to get the smallest diameter at the base; many trees, especially on slopes, are flat and measure several inches more one way than another. Trees that show much defect are an

unknown quantity and should be thrown out entirely.

Two active men will get over a half-section in a day, and do it well if the timber is not too small and the undergrowth is not too dense.

Sometimes I am called upon to give a rough estimate of a tract in a hurry. I handle this in the same way that I have shown above, except that I do not use the calipers, but guess at the diameters as well as at the length. In this manner one can get over the ground as fast as the assistant can tally the trees, and we usually estimate about 12 lots per day under this system. Of course the results are not so accurate as when the caliper is used.

The above is illuminating in many directions, suggestive of varying conditions and requirements, and varying methods of treatment in response. Further under this subdivision there will be included only a reference to the "horseshoe" plan of cruising employed by many Lake States and Southern cruisers. Diagrams of a northeast



quarter-section and of a forty illustrate the plan of travel, so designed as to reach into all parts of the subdivision concerned. Along this route the cruiser commonly covers by detail estimate a strip 50 paces wide, which gives a large percentage of the whole area.

5. The field of ocular estimate is to be found especially

in small bodies of timber and in tracts of small dimensions. This is because a man can really see and grasp them. Such estimates are particularly useful for timber of small value or in very bunchy and irregular woods, which it is hard to survey. In such circumstances the judgment of a good woodsman is sometimes the best valuation that is practicable.

The ability to estimate timber after this fashion is gained by practice, and is based on personal experience and capacity; consequently each man goes about it in a way of his own. To know the area of the tract in question is generally of great assistance, and most men will be continually studying the matter of average stand per acre. As a preliminary step in arriving at this it is generally desirable to settle

maximum and minimum stand as well.

For the contents of single trees a woodsman may rely on a mere glance, or he may figure carefully. A northern Maine lumberman, for instance, looking at a fair-sized spruce might estimate that it will cut a log 10 inches in diameter at the top and 30 feet long, and such a log he might know will measure 180 feet in local scaling practice. Again, in regions where logs are cut short, and several are taken from a good-sized tree, men frequently jot down the estimated contents of the several logs and add up the figures to get the tree's total contents. Using such methods to get at the size of the trees, lumbermen then go on, in one way or another, to get the contents of bodies of timber or stand per acre.

Frequently, however, the impression gained is a direct one, of quantity on a whole tract or of constituent bunches. A man cannot tell just how such figures come into his mind, but they do arise there, dependent somehow on his experience, perhaps in laying out roads or chopping timber. Such training is effective, and when the judgment arising as a result of it has been actually tested and found sufficiently close and reliable for any given purpose, it would be folly not to use it. But every one knows that such judgments are fallible, as in the nature of the case they could not fail to be. Differences in size and height may escape a man if the stands traversed look generally alike; the atmosphere and the lay of the land both have an effect on

the appearance of timber; a man's condition also varies from day to day, affecting his judgment in this matter, as

in every other.

The above is the faculty of the old lumberman. On the other hand, the forester who has studied the rate of growth and the yield of timber has, in area, soil quality, and density of stocking, factors which he can profitably use to help him in his estimate of quantity. A fully stocked acre of white pine on good soil in Massachusetts, for instance, will yield at forty to sixty years of age a thousand feet of lumber for each year it has been growing,—a standard which a man may use to check the judgment through a considerable range of conditions.

Ocular estimate has been spoken of as especially appropriate to small tracts of land, but as a matter of fact the methods and principles here stated are still employed to a large extent in the valuation of the largest tracts as well, and even for the purposes of sale and purchase. This is perhaps not as it should be, but it has at least partial justification in the fact that as business goes the amount of timber on a tract is not the only element in value: often it is not the largest, even, for in addition availability, safety, the suitability of a tract to given purposes, and the financial situation of the parties concerned must all be considered. Sometimes a tract by reason of its relation to a given investment or manufacturing enterprise really must be had, almost regardless of its timber resources; while, on the other hand, though rich in timber, another tract may be dear at a small price. Accurate estimates of the quantity of timber, therefore, may be a secondary matter.

When large tracts are estimated by the eye, it is commonly done on the basis of so much to the acre, either from the looks of the stand or by comparison with some similar tract already cut. Subdivisions, if they exist, might be estimated separately, and the estimated area of waste lands would then be thrown out of account. Some old lumbermen might also estimate by valleys, judging quantity from the density of the timber and the length of the

roads necessary to operate it.

6. Recount of the work done on a tract of 89 acres

in Massachusetts, having considerable value and a varied stand of timber, will illustrate the different methods of timber estimation and the way of going to work in a particular case. This tract was mapped topographically. The methods employed for that purpose are described in Part II and a complete map of the tract is given on page 114. The steps contributing to the timber estimate are as follows:

a. Boundaries run out to get area; chainage marks left

at frequent intervals.

b. Some 65 M feet of heavy and valuable pine timber calipered tree by tree; numerous heights measured; contents ascertained from volume table.

c. Three bodies of thick young pine circled by staff compass and pacing to get area. Average stand of each bunch ascertained by laying out quarter-acre sample plots representing 10 to 20 per cent of the area. Trees on these plots calipered; heights measured or estimated; contents taken from volume tables.

d. Ten acres of hard-wood swamp in north end estimated for cord wood by similar but quicker methods.

e. Balance of 60 acres of ground is covered with scattering pine and hemlock, chestnut fit either for box boards or railway ties, poplar, red oak, and other hard woods. Northerly 37 acres considerably better than the other 23. Ran strip surveys across the two parts representing 10 per cent of the area, running the strips across the ridges and the belts of timber. Calipered the trees into classes of pine, hemlock, chestnut, poplar, hard woods fit to saw, and cord wood; estimated saw contents from tables, such as were at hand, adjusted to the locality and practice, with due reference to heights; estimated cord wood from tables, experience, and judgment.

The field work involved in steps b, c, d, and e represented one day's work for four men. Result was the following:

ESTIMATE OF CLARK BROS'. PARKER LOT, WOODSTOCK, MASS.

White Pine (including 50 M good plank)	660 M
Hemlock	35 "
Chestnut	156 "
Poplar	63 "
Red oak, etc.	67 "
Total saw timber	981 "
Also hard-wood fire wood, 600 cords.	

These methods are those of an estimator not in frequent dealings with timber of this class. The owner of the lot, a man of long experience and in constant practice, would have chained or paced out the pine areas, and estimated their stand per acre from experience. The scattering soft wood and the heavy bunch of pine he would have estimated in a lump sum. The main elements of value being then dealt with, he would probably rely on his judgment for the rest after looking carefully through it. With a helper, he would take as much time as was actually consumed, or more. This man, one of the most successful operators in Massachusetts, says that using these methods he can estimate pine lots within 5 to 10 per cent as a rule, but occasionally makes a blunder of 30 to 50 per cent.

Other successful men in the same region, a region where stumpage values are high and competition for merchantable lots very sharp, show great variety in their methods. One man calipers all the timber on a lot he expects to purchase, assuring himself about stand and value in that way, and in addition securing data which tell him what he can best put the trees into. Others use no instruments but, relying on experience and taking plenty of time to look around, make a lump estimate. That there is great difference in cost among all these methods is not certain. It is sure, however, that for most men that method is best which has in it less guess work than measuring. But the facts recounted illustrate the principle that there may be several good methods of doing a given piece of work, and that the choice may turn on the training and habits of the estimator.

B. ESTIMATION OF LARGER TRACTS

When land areas, as is frequently the case in the United States, are of large size, and particularly if the stand upon them is small and the value low, only a percentage of the area can be covered by a timber survey, and the problem is to make that percentage as representative of the whole as possible. Amidst the great variety of methods employed, three main types of work may be distinguished.

1. Type and Plot System

According to this method the land to be passed on is divided up into types of known area and approximately like stand, without, however, necessarily leaving marks on the ground. Through these subdivisions of his area the cruiser travels, studying the size, height, density, and condition of his timber, and forming as he goes an estimation of the average stand. This estimate he checks by a number of sample plots, run out with the tape, and examined with care. The plots are usually laid out either in square or circular form, though the strip system is perfectly applicable.

Very satisfactory results have been arrived at by this method where a considerable area in sample plots has been surveyed or where the estimator is a man of judgment and experience. But choosing a few sample plots to represent a tract is recognized as a very delicate matter. Beginners generally select too good a piece, and the man who is really competent to do it can usually make a close guess at the whole thing. As with all other methods of estimating, area should be known from surveys, and that

in not too large units.



A good example of the application of this system comes from a national forest supervisor who had to estimate for a timber sale a tract of some 1200 acres. It lay in the form shown, with a ridge running down the middle of it, which naturally formed the first line of subdivision. The tract was

therefore surveyed with compass and chain and a dividing

line run along the ridge top.

Then on each side of the ridge three distinct types of timber stand were recognized. The heaviest timber, red fir of good size, was in the middle; the north end was lighter, with a mixture of lodgepole pine; the south end had been damaged and rendered very thin by fire. These blocks were therefore blazed out and roughly surveyed. Thus the land was divided into six compartments of approximately even stand and of known area.

Then with a party of three men the supervisor ran 4-rod strip surveys 1 through each compartment, covering in each from 10 to 15 per cent of the area. Having no volume tables, he scored down instead the logs judged to be in the trees passed, in 16-ft lengths and by inch-diameter classes. In the office the contents of these logs were ascertained from the scale rule, multiplied by the number of each size, and added together. If then 10 per cent of a compartment had been covered, multiplying by 10 gave the stand of the compartment, which was the result desired.

With trustworthy volume tables and calipers better results could probably be had, but those here obtained were satisfactory. General good judgment is essential in carrying out such a survey, but, that given, a man can do it who has not had long woods and mill training. In fact, in the same forest one or two green but intelligent men are said to have been quickly trained so that their figures

could be relied on within 10 or 15 per cent.

2. THE STRIP SYSTEM

The strip system of estimating has been used rather widely in woods work, not infrequently in connection with land subdivision. As a survey party is running through the woods, it is sometimes made the duty of the chainmen to count the merchantable trees for a stated distance on each side of the line run, the contents of the trees being determined oftenest by an estimate of the number necessary to make up a thousand feet. The same system in effect is sometimes used by the cruiser who counts the trees passed within a certain distance as he travels across a lot, or the work may be done more elaborately, and the caliper and hypsometer introduced to any extent thought advisable.

The methods of a Michigan cruiser who employs this system were described on page 178. Following are methods pursued on tracts of considerable size by a number of progressive concerns at the South dealing with pine and a variety of hard wood timbers.

The strip lines are usually 1/4 mile apart; they may be

¹ See next article.

carefully run and marked in advance by a survey party, or a compassman going along with the timber estimator may run and pace them. Topography may be mapped; notes are taken of swamp boundaries and other changes in the character of ground or timber.

The strip estimated is either one or two chains wide, split by the line of travel; thus either 5 or 10 per cent of the gross area is covered. The estimating party proper consists of three men, two to caliper the timber breast high, and one of good training who is responsible for the work as a whole and who does the recording and estimating. His note book has separate space for each species and under each a line for diameters by inch classes. Each tree on the strip is scored down as calipered, or it may be the number of 16-foot log lengths.

In such a vast region there is bound to be much variation in utilization, scaling, and mill practice so that when volume tables are employed they are usually of local origin to correspond. Since, however, the country is of very gentle topography, height and taper within the same species are unusually even. Two inches taper for each 16-foot log above the butt log has been found to be widely characteristic of pine timber, and three inches of hard wood timber. Some tables then have been made up on the basis of these regular tapers.

Small Diameter of Butt Log Inside Bark		Number of 16-foot logs									
	1	2	5	8							
		Conten	ts in Fee	t Board	Measure						
15 16 17 18	160 180 200 230	280 320 360 410	360 420 480 550	410 480 560 650	440 520 610 710	540 640 750					

Accompanying is an extract from a volume table ¹ constructed on this plan, giving figures that, when manufac-

¹ From "Southern Timber Tables" by Howard R. Krinbill, Newbern, N. C. Copyrighted.

ture of highest present economy is practiced, approximate mill output. A peculiar feature will be noted in this table — that the base diameter employed is not diameter breast high, but diameter inside bark at the top of the first log length. A reduction from calipered diameters is required therefore, for bark thickness and for taper. This reduction is made either tree by tree in the field by estimate or in the office by classes on the basis of measures taken in logging operations. Timber quality is a matter of importance. It is seldom or never dealt with in the field other than by way of general comparison and experience.

The strip system was also largely employed in the early years of the United States Forest Service, with the object of ascertaining not merely the merchantable timber on the tracts examined but also the number and kind of young trees growing there as a basis for recommendations as to treatment. The method and cost of strip survey work as carried out by the Service men are indicated in the following extract from the "Woodsman's Handbook":

Sample acres are laid off in the form of strips, 10 surveyor's chains long and I chain wide, and the diameters of all trees to be included in the estimate are measured at breast height with calipers. At least three men are required to do effective work under this method. One man carries a note book, or tally sheet, and notes the species and their diameters as they are called out by the men who take the measurements. The tallyman carries the forward end of the chain, the other end of which is carried by one of the men taking the measurements. The chain is first stretched on the ground and the trees are calipered within an estimated distance of 33 feet (one half chain) on each side of the chain. When all trees adjacent to the chain have been calipered the whole crew moves on the length of another chain in the direction chosen (by the tallyman). The chain is again stretched on the ground and the trees are calipered on each side of it as before. This same operation is repeated until the trees have been measured on a strip 10 chains long. Notes are then made of the general character of the forest and the land, according to the requirements of the investigation. If heights are desired they may be taken by a separate crew, or as the calipering crew encounter from time to time trees whose heights are desired, they may stop long enough to take such measurements.

In an average virgin forest a crew of three men will caliper the trees on from 20 to 40 acres in one day if only trees of merchantable size are included, or from 15 to 25 acres if the small trees also are calipered. Small trees are measured principally in studying the question of future growth.

FORM OF NOTES

Locality. T.5. R.18. W.E.L.S., Maine_____ Type_Hardwood. Slope____ Date_Sept. 17, 1901 Sheet. No. A. 41

D.B.H.	Spruce	Dead	Fir	White Birch	Beech	Hard Maple	Pine	Popl.
2 in.	MU		N					
3 "	Ø :.							
4 "	☒.			:				
5 "	☒.							
6 "	:			⋈				•
7 "	11		•	国.	:	_	_	
8 "				☒.				
9 "	:		•	⊠:	4	•		M.
10 "			•	:				
11 "								-

On large tracts satisfactory estimates can be made by the measurement of about 1 out of every 30 acres. In very extensive forest tracts the Bureau of Forestry usually measures not more than one or two out of every hundred acres.

This method is clearly adapted to securing knowledge of the make-up of a forest, and of its stand of merchantable timber if good volume tables are at hand to go with it. In the latter connection perhaps the greatest difficulty that arises is in applying the proper heights to the different diameters. This is slight if the tract is of small size and uniform character, but considerable on large tracts with uneven topography and varying stand. In addition constant care is required to make sure that the strip is kept of right width, in other words that all trees less than 2 rods from the line run are included and none at a greater distance. Careful men do indeed quickly get trained to

this so that their eyes are true, but with the best of men an occasional swing-off of the chain is necessary. Defects in timber also remain to be allowed for.

As applied to large tracts the strip system may either be employed within types the boundaries of which have been ascertained, as was explained in the last article, or it may be laid out in long lines across country and itself be used to define those boundaries and to get the topography. A number of townships in Maine have been surveyed in the following manner:

a. Township lines re-run and re-blazed; chainage marks

left every half mile.

b. A center line run through the township, this also

being chained and marks left each half mile.

c. From a main camp on the center line, 4-man parties ran strip surveys from a mark on the center line out to the boundary, checked on the mark there, set over a half-mile, and ran back. This was 2 days' work, and the party consequently carried outfit required to stay out one night, the main camp meanwhile being moved along the center line. Note was kept of the ridges and streams crossed, also of the lay of the land, of the bounds of cuttings, and of marked types of timber. Elevations on such a survey may be got by barometer, and a topographic map made up as a result.

3. LINE AND PLOT SYSTEM

A third system employed with some variations in different parts of the country, most largely perhaps among spruce men in the East, combines features from both the foregoing. Under this system the cruiser while at work travels in straight lines through the country to be explored, using his eyes as well as may be while actually traveling, but stopping at regular intervals to count and estimate the trees on an area about him. The area usually chosen is a quarter acre, which has a radius of 59 feet, or, for most men, of 23 paces. For a check on this distance a tape line should always be carried in the pocket, and every morning, as well as occasionally through the day, the eye should be checked by actual measurements.

Carefully training in this way, a man will find himself

able to guess within 2 feet of the 59.

The timber may be estimated according to any method deemed most satisfactory. It may be calipered by an assistant and the factor of height gone into to any extent thought best, but most men in the spruce region do that only as a check, while in common practice, after counting the trees of any species or class, they estimate their contents on the basis of so many to the cord or to the thousand. Occasional calipering and height measurement as a check on the eye are highly desirable, and volume tables also are a help in most cases. But some species of trees (as cedar and beech in many localities) are so imperfect and defective that volume tables, if they were in existence, could not be depended upon. Such timber has to be estimated out of hand, and lumbering experience, together with the figures of the scale rule carried either in a man's head or in his pocket, will prove the best equipment for it.

One advantage of this method is its cheapness—one man may do the work alone. Further, all doubtful points are settled on the ground, face to face with the timber—there is no discounting or computing afterwards more than to add up the results. Then the small size of the area and the nearness of the observer to the trees under consideration enable him, if he has proper experience and judgment, to set contents very close. Lastly it will be seen that the systematic travel followed gives, in a simple country, material for mapping its timber types, also its topography, as was explained in Part 2 of this volume.

Following are specimen notes of a line of estimate run directly across a section with quarter-acre counts taken 150 paces apart. The timber is scored in the following classes: (a) spruce above cutting limit of 14 inches stump diameter in board feet; (b) smaller spruce down to 6 inches breast diameter in cords; (c) fir in cords; (d) cedar in feet B. M.; (e) pine; (f) good hard-wood logs. Number and contents of trees both given.

This method of timber cruising may be employed on land areas of any size, and has been largely employed on

areas of a mile square, or "sections."

To travel the boundaries of a square mile and twice across it, taking quarter acres each 20 rods as determined by pacing, gives about 21/2 per cent of the area actually covered by the estimate, and that percentage can be relied upon to give, in land which has any regularity of type, a close approximation to the truth. To do that and what goes with it, section after section through a township, is just about a fair day's work.

Sp. Logs	Sp.Pulp	Fir	Cedar	Pine	HardWood	
4-400	33	16-12	12-300			
9-1200	28-4					
8-1800	2	8-/			Soft wo	ods on flat
3-400	7-1			1-100	land, Sto	ny but
3-500	7-1	34-4			Smooth	logging.
10-2000	7-8	24-3	4-100		Abundan	t reproduct-
9-1300	2	.9-1.3			ion of fi	r, with spruce
8-1000	7-1	12-12		2-300	& occasio.	nal pine in
11-1500	23-21	8-1			openings	ş
8-1000	37-3					
5-800	19-2	5-1	Last 60	rods in	2-300	
3 -700	6-6	43	mixed	growth	5-900	
Acre 4200'	5.4 C.	4.7C	133'	133'		
				1		

The last two methods described as usually employed are alike in this, that in the endeavor to get at a fair sample of the country they depend mainly on mechanical arrangements rather than choice. This as a general rule is a safe thing to do. There will always be enough things left to exercise the best judgment of the estimator. On the other hand, neither this nor any other system should be followed blindly. If part of the tract is especially valuable, especial pains should be taken with it. As a rule it will be found safe to ascertain the area of such tracts and

estimate them separately, while on the other hand the area of bogs, burnt lands, barren mountain tops, etc., should be ascertained and thrown out of account.

C. STIMMARY

The above described are well tried methods of timber estimating and survey, but what has been written affords hardly more than suggestions as to how any particular job may best be done. Each method has its merits which may strongly recommend it for some particular circumstances. Very much too depends on the training and qualifications of the man doing the work. Every man long in the business commonly has a line of work in which he becomes proficient, developing methods best suited to himself to which in ordinary cases he will adhere. In conclusion, the following guiding principles may be laid down:

1. Estimates by lump sum are not usually reliable or at the present day sufficient.

2. Estimates of so much to the acre are much easier

to make and more likely to be close to the fact.

3. In any kind of timber estimate or survey, the area of the land ought to be known, and that in units not too large. Within limits the smaller they are the better, all the more so if each unit contains but one timber type.

4. Every time a measurement is substituted for a guess or judgment, the more reliable will be the result. On the other hand, experience and good judgment never cease to be required in the business.

5. No estimate is worth much, practically speaking, which fails to take height into account as well as diameter.

- 6. Quality in some circumstances is quite as material to an adequate timber survey as quantity. Its determination is fully as difficult.
- 7. "The more defective the trees are, the more preferable is the cruiser's judgment and long local experience in the mill and in the woods to mere measuring." 1 The same is true where great differences in value are dependent upon quality or grade.

¹ Schenck's "Forest Mensuration."

- 8. Very bunchy timber can be estimated only in bunches or tree by tree. No general system of lines or plots can be trusted to give safe results.
- 9. In the emergencies which arise in actual business, a little rough and ready land surveying is often the most vital part of a reliable timber estimate. One or two lines run with compass and chain will frequently check areas of waste land or of different stand in effective fashion. Transit and stadia work on streams or roads often affords very material help. There is continual call for the sort of results that can best be obtained by means of compass and pacing.

D. PACIFIC COAST METHODS

Much Pacific Coast timber is 200 feet and over in height and of diameter to correspond, while the stand sometimes passes 20 million feet per quarter section. It is evident, therefore, that because of the values involved intensive methods of cruising are appropriate, while peculiarities of method are suggested by the very size and height of the timber. Of the region as a whole the portion west of the Cascade Mountains in Washington and Oregon, producing Douglas fir, "Oregon pine" as it was called formerly, is most active and characteristic, and the following refers to that region unless specified otherwise.

SUCCESSIVE LOGS IN A FIR

							Top Diam.	Scale	% of Total
1st 2nd	32-foot 32-foot	log					31 28	1420 1160	33
3rd 4th	32-foot	log					31 28 25 20	920 560	33 27 21 14
5th	32-foot	log	٠	٠		٠	14	230	5
	Total							4290	100

Adjustment of methods to the conditions is illustrated particularly by the volume tables employed, for those at present in most extensive and responsible use are constructed on principles that have very seldom been employed elsewhere. After basal diameter, taper per 32-foot 1 log is the next factor allowed for, total height of the tree is disregarded, and number of logs is the third factor in the tabulation. This has reason behind it as well as experience. In timber of such dimensions total height is not readily estimated; the lower logs of the tree are very much the largest and far the best in quality; a log more or less in the top, comparatively small in size, full of large knots and liable to be broken up in felling, is of small account in the estimate anyway.

In connection with these tables, basal diameter also is handled in a peculiar manner. In some tree species thickness of bark is very variable, while the root swelling of large trees frequently reaches to the height of a man and higher. Diameter therefore is taken as nearly as may be where the tree takes on its regular form, considerably above breast height as a rule; deduction is made for any swelling not thus allowed for, and double the thickness of bark as actually found is then subtracted. By this means, the wood alone is dealt with, and basal diameter is aligned with the general shape of the tree.

In view of the facts above mentioned it is clear further how windfalls furnish the best obtainable assistance to the cruiser's judgment in respect to height and taper, also that the diameter tape and Biltmore stick possess advantages over the caliper. Then two additional problems arising out of the size of the trees confront the cruiser: first, breakage in felling is a much more important factor than elsewhere, and its amount varies widely with the ground conditions; second, the defect arising from decay and other sources, very hard to judge, to detect even, in timber of this height, has to be handled with extreme care — careful looking, the examination of windfalls, experience, perhaps the outturn of adjacent timber serving as a guide to it.

The "forty" is the ordinary unit of area for cruising and a timber report, and it is gridironed with straight line travel. Pacing serves ordinary purposes as a dis-

¹ Tables based on 16-foot logs are also in existence.

tance measure; a vernier compass is usually employed for the sake of more accurate line running. Twenty to fifty per cent of the gross area is commonly covered by actual estimate, one hundred per cent in some cases. The unit party for the work consists of two men, compassman and cruiser, of whom one handles distance, area, and topography, while the other is responsible for the timber. Details of practice vary much, as elsewhere, in accordance with the purpose of a cruise, conditions found, and the training of different estimators. Following is a description of a method as near standard as any, widely employed in work of high responsibility.

a. Section lines are usually freshened up and rechained, and a center line may be run through each section. The main purpose of this work is to set stakes for the guidance of the cruising party. It is so laid out that the actual cruise or estimating lines will run as nearly

as may be across the features of the topography.

b. The cruising party, starting at one corner of the section to be examined, proceeds to the nearest stake, 21/2 chains from it, whence the compassman, with the declination set off in his staff compass, travels parallel to the side line of the section, keeping account of his pacing, taking aneroid readings at changes of the ground, and sketching topography. Behind him follows the cruiser, who for a width of 5 rods on each side, estimates the timber. 500 steps, 4 tallies, make a quarter mile, the width of a 40. At that point the scoring of timber begins anew, for the new forty being entered. So the work proceeds until the opposite section line is met (or at half that distance if the section is subdivided), when the pacing is checked up, the compass work tested on the stake and declination reset if necessary. Offset is then made to the second stake, 7½ chains from the corner, from which point a parallel line is run in the opposite direction. Four such lines are run across each tier of forties. With 16 such lines the cruise of the section is completed.

c. The detail of the estimating work is as follows:— First, in nearby timber being cut, or in ordinary circumstances by examination of windfalls, the cruiser trues up his judgment on the contents of the trees. In this connection his volume table is of assistance since study of the height and taper of the down timber shows to what portion of his tables its form relates it. Two and three inches per 32 foot log are light tapers, not infrequent in hemlock and young fir, but four and five are usual in mature fir timber. This examination also tells something as to log quality and the amount of defect. Along with it the cruiser makes sure by numerous tests that his eve is true on basal diameter. With these points settled his preliminary work is done and, with an eye out for factors that influence breakage and particularly for "conks" and other signs of unsoundness, he will proceed confidently. The figures he sets down on his tablet represent his judgment of the merchantable contents of trees as he passes them, species, individual form, defect, and breakage all being allowed for. The conscientious man, however, applies frequent check by further examination of windfalls and occasional measurement of strip width and of basal diameters.

SAMPLE OF CRUISER'S FIELD NOTES (Usually made on celluloid sheets)

Fir	Dead &	Cedar	D&D	Hem.	Spruce	Poles					
FIL	Down	Ceuai		Trem.	Spruce	Fir	Hem.	Cedar			
2-6 M 1-2.5 6-30 2-7.5	2 1.5	17 14 1-3.	.8	1-1.5 2-2.5 13 1-1.	1–5 M	1 Ave	1 erage 45	111			

d. Checks from outside are a feature of the work as carried out on a large scale commercially. The different cruisers in a large party may be set to check one another as a corrective and for uniformity; a head cruiser periodically checks each man to catch up any slackness, correct any wrong tendencies, and give advice or directions.

Two miles of line per day are the standard product for this method of cruising, giving eight working days to the section, which involves a cost of about 25 cents per acre outside of the checking, overhead and office work. Ordinary variations are:—

a. Double running each forty instead of running four times through it as above, a method widely practiced as costing less and considered sufficiently accurate in many circumstances. The cruise lines in this case are started 5, 15, 25, etc. chains from the section corner to divide the area equally. Sometimes, also, the strip is widened.

b. For preliminary work, one strip only may be run per quarter mile, and after a certain amount of that with its results in training, even this may be discontinued and

a man rely on general observation.

c. A 100 per cent cruise is carried out in some cases. In this case a second compassman may advantageously be employed and the cruiser work between lines run and marked by the two men, the exact width of the strip being then of no consequence. Sometimes, also, a second estimator is employed to take care of certain classes of the timber.

d. Some men, instead of estimating the timber on strips, estimate circular areas so spaced along the compass line that they touch one another. For this practice it is claimed that a man can do better estimating work standing quietly at a center than while travelling, with his mind more or less distracted about footing, etc. In earlier times indeed a circular plot system was general, while another usual procedure was to count the trees on these circles or on strips to the length of one tally, and derive their contents from that of the average tree as estimated. Few follow this last practice at present, however.

In conclusion on this branch of the subject, the following, by a man of long experience and acknowledged competence in this line of work, is introduced for the light it throws on the broad aspects of the matter.

We work in general by the strip system but under a less hardand-fast rule than formerly. More is left to the judgment of our cruisers as to the number of runs through a subdivision necessary to secure correct results. Thus, if we find one forty that is densely timbered with a small uniform growth, we find that we secure better results by taking narrower strips, the equivalent of one sixteenth of a forty instead of one eighth. Where trees stand so thickly on the ground it is almost an impossibility for men to keep an accurate count on a wide strip as they can on one of half the width, and we find that the basis of much of the error that occurs in our work is due to inaccurate tree counting.

If the timber is large and particularly accurate results are desired, we now run 12 times through each forty and frequently work between blazed lines. That is, instead of running through the middle of the strip, the compassman sets over one-half its width and spots the trees on the opposite side from the cruiser to give the cruiser a line to work to on the return strip. This works very satisfactorily where the brush is not too dense.

Again, under certain conditions where we have a uniform stand of large timber, we run 4 times, taking strips equivalent to one-twelfth of a forty. This plan, we believe, gives better results

than two strips each covering 1/8 of the whole.

These notes give some idea of how we attempt to carry on our work, but in the last analysis this cruising business resolves itself into one of personal capacity and attention upon the part of the cruiser rather than the method employed. A careful, conscientious and hard-working woodsman whom we can depend upon to go over the ground is more valuable than a more expert cruiser who takes much for granted. There was a time when I hoped to develop timber cruising to a point from which we could look upon our estimates as being absolutely reliable, but so long as there are influences that will work upon the minds of men, there will be variation and error. A man may do excellent work today and be totally unfit to be in the woods to-morrow, all for reasons which none of us can explain. A man must have confidence or he will be of little value. On the other hand I think I may safely say that the greatest element of uncertainty and error in men's work is their proneness to feel that familiarity has developed infallibility. The man who never develops absolute confidence in his eye and judgment and who checks himself up frequently, seldom goes far wrong.

There is, too, another side to this whole matter, one often neglected, but of great importance, and that we consider in our work as best we can. That is the standard of utilization of the timber. As a matter of fact there is surprising difference in the way timber is cut, though I could not define this as a percentage. A concern milling its own timber cuts closer than one selling its logs; and there is variation with the market itself. Then occa-

sionally a tract is cut with such carelessness that the yield is very materially cut down. We have to meet the wishes of our customers if clearly expressed, but we protect ourselves by an explicit statement of the kind of utilization which our estimates imply, and by an exact showing of the basis on which the work was done.

Timber Quality. While the above applies specifically to the Douglas fir country, much the same methods are employed in the Interior and California, with resort to others of less intensiveness, similar to those in use elsewhere, when stands are lighter or less valuable. The preceding, however, is inadequate in one field of importance, in that quality of timber has been given scant emphasis. This throughout the region is no less important a factor in value than quantity. In fact, in very much territory timber has no commercial value unless its products are suitable for other than ordinary building purposes.

In the case of Douglas fir and timbers associated with it west of the Cascades this matter is simplified by the fact that log grades instead of lumber grades are made the usual basis of quality rating, the log grading rules in force in the market thus furnishing the standard to which the field man works. Since, however, both dimension and lumber quality enter into these, their application is

not simple.

The grading rules for Douglas fir logs in force on Puget Sound follow; those of the other log markets are very similar. Spruce is commonly graded like fir. With cedar, because of the variety of products into which the wood may be manufactured, grading varies from time to time and locally. Hemlock logs and those of the species rarely met are sometimes classed in two log grades, those above 16" in diameter and surface clear, and all others.

No. 1 (also called *Flooring*) logs shall be logs in the lengths of 16 to 32 feet and 30 inches in diameter inside the bark at the small end and logs 34 to 40 feet, 28 inches in diameter inside the bark at the small end, which in the judgment of the scaler contain at least 50 per cent of the scaled contents in lumber in the grades of No. 2 Clear and better.

No. 2 (or Merchantable) logs shall be not less than 16 feet long and which, having defects which prevent their grading No. 1, in the judgment of the scaler, will be suitable for the manufacture of lumber principally in the grades of Merchantable and better. (Merchantable lumber must be free from knots or other defects in size or numbers such as to weaken the piece.)

No. 3 (also called No. 2) logs shall be not less than 16 feet long which, having defects that prevent their being graded higher, are, in the judgment of the scaler, suitable

for the manufacture of Common lumber.

Cull logs shall be any logs which in the judgment of the scaler will not cut 33½ per cent of sound timber.

An essential to reliable timber grading is experience, a background of knowledge of the out-turn of similar timber. In the next place, close examination of the stand is required as to the number and size of limbs and knots and for indications of these, or of other defects, that may lie beneath the surface. Age is a help here (these stands are commonly even-aged over considerable areas). Many cruisers go no farther than this and set percentage figures for log grades as the result of a broad judgment.

When further detail is thought desirable, the volume tables before mentioned are of assistance, giving as some of them do for a tree of given diameter, taper, and merchantable length the percentage each successive 32-foot log bears to total contents. One standard volume table

contains the following directions: -

"Determine the percentages of the different grades as contained in a given percentage of the trees on each 40 acres by selecting, for instance, an average tree on each tally and carefully determining the percentage of the different grades of logs contained in these sample trees and applying the average to all trees on the forty."

To illustrate, in the notes on page 199, 11 trees, 46 M feet, are scored down in the column of living fir, giving an average volume of 4200. 4 inches taper and 4 logs may fit this timber; if so, a tree yielding 4330 feet (see extract from taper table) gives a close approximation. Of such a tree a 32' butt log constitutes 37 per cent, the second log 28

per cent, and the third 21 per cent, while top diameters are approximately 33, 29 and 25 inches respectively. One of these logs is large enough for No. 1; it may or may not be clear enough. Second and third logs are of sufficient size, and likely to be of a quality, to put them in the second grade.

Methods in this branch of the work, however, vary greatly. A few, in the endeavor to reduce the field of judgment, have gone into much detail and devised forms of notes which record trees by sizes and log grades in each tree as its contents is estimated. Of the percentage of successive logs, it may be said that the above relations are fairly typical—that is to say in normal fir timber large enough so that log grades are of importance, about 35 per cent of the total contents of trees is contained in the butt log if cut 32 feet long, the second log will add 25 to 30 per cent more, and about 20 per cent will be in the third log. Breakage and defect may throw out these relations, and they are different in extremely tall or short timber.

	Ft.	3]	Logs or 96	Fee	et		4 Logs or 128 Feet						
Butt Diam. Inches	In. Taper in 32	Diam. Top in.	Contents B. M	% 1st 1	% 2d	% 3d w	Diam. Top in.	Contents B. M	% 1st	2d	% 3rd	% 4th	
37	3 4 5 6 7 8 9	28 25 22 19 16 13 10	4230 3714 3234 2790 2386 2029 1729	43 46 50 55 60	33 33 32 32 31 28	24 21 18 11 09		5128 4330 3610 2979	37	28 29	22 21 19 17 	14 10	

Note. Half logs are given in the original tables.

Since a large share of the timber of the fir region is realized on by its owners in the form not of lumber but of logs, the inducement is small to go further than the log in quality work in that region. It is otherwise, however, in the regions characterized by pine, where there are no log markets and timber enters the commercial field in the shape of lumber with its great range in quality and value. Here the Forest Service, endeavoring in its own business to get away from the judgment of the individual applied in too broad a way, has started a line of inquiry that should in time prove serviceable to business. grades in this case again are made the basis to which the field man works, but mill and vard studies, carrying the product of those logs through the process of manufacture to point of sale, afford a means of going further, to an estimate of lumber quality and value. Definitions of the log grades that have been formed for yellow pine follow, and brief notes on the yield of those grades may be serviceable to some, although, with a small field covered, it has been found already that logs graded by the same man under the same rules vary considerably by locality in their yield of high grade lumber.

Yellow Pine Log Grades of the U.S. Forest Service.

Clear logs shall be 22 inches or over in diameter inside the bark at the small end and not less than 10 feet long. They shall be reasonably straight-grained, practically surface clear, and of a character which in the judgment of the scaler are capable of cutting not less than 25 per cent of their scaled contents into lumber of the grades of C. Select and better.

Shop logs shall be 18 inches or over in diameter inside the bark at the small end, not less than 8 feet long, and which in the judgment of the scaler are capable of cutting not less than 30 per cent of their scaled contents into lumber of the grades of No. 2 Shop and better.

Rough logs shall be 6 inches or over in diameter inside the bark at the small end and not less than 8 feet long, having defects which in the judgment of the scaler prevent their classification into either of the two above grades.

Logs cut from rather large and high class timber at different points of interior Oregon, graded according to the above rules, have yielded as follows:

Clear logs 60-65 per cent No. 2 Shop and better, about half of it of grades B and C Select.

Shop logs 40-45 per cent No. 2 Shop and better, a fifth to a fourth B and C.

Rough logs have yielded about 15 per cent No. 2 Shop and better.

For the Novice. From the foregoing it will be inferred that the best timber cruising in the Pacific region is a highly expert business, requiring in addition to accuracy and alertness, thorough personal training and judgment in high degree. There are always learners in the field, however, and occasionally inexpert men are so situated that with whatever equipment they can command they must do their best to size up the quantity and value of timber. To such, a caution in respect to the loss of apparent volume that breakage, shake and decay may cause and the very large part that location, and especially quality, play in the value of timber is an essential service. Then it is true and worthy of regard that in these circumstances simple methods may actually give the best results.

A man may learn much in a logging operation where timber similar to that he is concerned with can be examined after it is felled and bucked into logs. He can see how much is broken up, whether the timber is rotten or sound, and from the cross cuts and surface indications of the logs examined at close range get an idea of the prevalence of knots, shakes and other blemishes. Then he can scale up the logs from a number of trees, ascertaining the total length utilized and the quantity of merchantable timber derived from each tree. This he will attach to its length and base diameter and endeavor to link up with trees of similar dimensions standing.

Such work as this will enable a man to understand a volume table, and he may even get enough measures to make one for himself in some size groups, with which he may check published volume tables. Or old devices and short cuts¹ may be tried out with the idea of sharpening

¹ Such as the following:-

Average the base diameter of the tree and the top diameter of its merchantable timber; get the scale of a log of that diameter

the observation and training the judgment. The best result that can come from such work (it can be gained only with time and experience, and some men never will acquire it) is the capacity to make a close estimate of the contents of a tree standing.

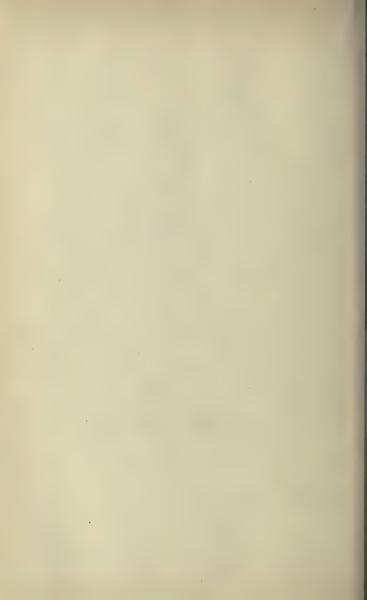
Contents of the average tree in a piece of timber, obtained by methods of this kind, may be made a starting point for the next step in the process. A man may count all the trees standing on a small piece of ground, using safeguards that he will readily think up to get all the trees in and not to count any a second time. If the territory is too large for that, sample acres in any number can be run out in fair average ground and the trees counted up on them.1 A square acre is 209 feet on a side, about 80 paces. A circular acre is 236 feet in diameter. Or, some form of the strip method may be used as described on the preceding pages. The area of ground without timber should be thrown out: single trees or bunches that are of exceptional size and quality should be treated separately. Material loss from breakage enters when about 100 feet in merchantable length is passed, and runs up to nearly or quite 50 per cent on very broken land with heavy timber.

The above, compared with really adequate, professional cruising, is only an expedient; still, carried out by a clear-headed man, it might really be worth more than what passes oftentimes as something more ambitious. Such a man, too, can sometimes find out what he wants to know, or manage to protect his own interests in matters of this kind, without resort to timber cruising. Some men also have judgment on the contents of a body of timber as a whole who are unfamiliar with a systematic timber estimate, and would be slow and uncertain in the execution of it.

32 feet long; multiply by the number of 32-foot logs less one-half log.

Or, to base diameter add one-half of base diameter and divide by 2; multiply by .8, square and divide by 12. The result is the number of feet in the stick per foot of its length. 3 to 5 per cent may sometimes be added for contents above the point stated.

¹ For a caution on this head, see page 187.



PART V

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SECTION I

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STADIA REDUCTIONS

Horizontal Distance

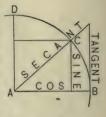
	0'	10′	20'	30′	40'	50′		0'	10'	20'	30'	40'	50'
0° 1° 2° 3° 4° 5° 6° 7° 8° 10° 11° 12° 13° 14° 15°		100.0 100.0 99.8 99.7 99.5 99.2 98.9 98.4 98.0 97.5 96.9 96.3 95.6 94.0 93.2	100.0 99.9 99.8 99.7 99.4 99.1 98.8 98.4 97.9 97.4 96.8 96.1 95.4 94.7 93.0	99.9 99.8 99.6 99.4 99.1	100.0 99.9 99.8 99.6 99.3 99.0 98.6 98.2 97.7 97.2 96.6 95.9 95.2 94.4 93.6 92.7	99.9	17° 18° 19° 20° 21° 22° 23° 24° 25° 26° 27° 28° 29°		91.3 90.3 89.2 88.1 87.0 85.8 84.5 83.2 81.9 80.6 79.2 77.7 76.2	91.1 90.1 89.0 87.9 86.8 85.6 84.3 83.0 81.7 80.3 77.5	91.0 89.9 87.7 86.6 85.4 84.1 82.8 81.5 80.1 77.2 75.7	90.8 89.8 88.7 87.5 86.4 85.2 83.9 82.6 81.2 79.9 78.4 77.0 75.5	90.6 89.6 88.5 87.3 86.2 84.9 83.7 82.4 81.0 79.6 78.2 76.7

Difference of Elevation

								Pro	por	tion	al I	Part	8	
0'	10'	20'	30′	40'	50′	1'	2'	3'	4'	5'	6'	7'	8'	9'
27° 40.45 28° 41.45 29° 42.40	15.73 17.37 19.00 20.80 22.18 23.73 25.25 26.74 28.20 29.62 31.01 32.36 33.67 34.94 36.17 37.35 38.49 49.58 40.62 41.61 42.56	17.65 19.27 20.87 22.44 23.99 25.50 26.99 28.44 32.58 33.89 35.15 36.37 37.54 38.67 39.76 40.79 41.77 42.71	14.62 16.28 17.92 19.54 21.13 22.70 24.24 25.75 27.23 28.68 30.09 31.47 32.80 34.10 35.36	24.49 26.00 27.48 28.92 30.32 31.02 34.31 35.56 36.77 37.93 39.04 40.11 41.12 42.09 43.01	43.16	03 03 03 03 03 03 03 03 03 03	.06 .06 .06 .06 .06 .06 .05 .05 .05 .05 .05 .05 .05 .04 .04 .04 .04 .04 .04 .04 .04 .05 .05 .05 .05 .05 .05 .05 .05 .05 .05	.05	.12 .12 .12 .12 .11 .11 .11 .11 .10 .10 .10 .09 .09 .08 .08 .08 .08 .08 .07 .07 .06 .06	.14 .14 .14 .14 .14 .14 .13 .13 .13 .13 .12 .12 .11 .10 .09 .09 .08 .08 .08	.18 .17 .17 .17 .17 .17 .17 .16 .16 .16 .16 .15 .15 .15 .15 .15 .14 .14 .14 .12 .11 .11 .10 .00 .00 .00 .00 .00 .00 .00	.20 .20 .20 .20 .20 .20 .20 .20 .19 .19 .18 .18 .17 .17 .16 .15 .15 .14 .14 .14 .13 .13 .13 .12 .11 .11	.23 .23 .23 .23 .23 .22 .22 .22 .21 .21 .20 .20 .20 .19 .18 .18 .16 .15 .14 .13 .13 .13	.266 .266 .266 .255 .255 .255 .244 .233 .233 .231 .211 .200 .199 .199 .117 .176 .156 .156 .157 .144 .131

SOLUTION OF TRIANGLES

The figure may refresh to good purpose the memory of the field worker. In it are graphically represented the functions (sine, cosine, secant, and tangent) of the angle *BAC*. The



cosine, cosecant, and cotangent of BAC are respectively the sine,

secant, and tangent of CAD, the complement of BAC.

B of the angle A in the right-angled

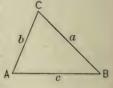
triangle A B C are as follows:

Sine
$$A = \frac{CB}{AC}$$
 Cosine $A = \frac{AB}{AC}$

Tangent
$$A = \frac{BC}{AB}$$
 Secant $A = \frac{AC}{AB}$

By these formulas, and the use of the tables of sines and tangents, all the parts of any right-angled triangle may be obtained if two sides, or an acute angle and a side, are given.

All the parts and the area of an oblique triangle may be obtained if any three parts including one side are given. Let A, B, C represent the angles, and a, b, c the opposite sides, of any oblique triangle; then the solutions are as given on the next page.



		The state of the s
Given	Sought	
A, B , a	C, b, c	$C = 180^{\circ} - (A + B)$
		$b = \frac{a}{\sin A} \sin B$
		$c = \frac{a}{\sin A} \sin C$
A, a, b	B, C, c	$\sin B = \frac{b \sin A}{a}$
		$C = 180^{\circ} - (A + B)$
		$c = \frac{a \sin C}{\sin A} \qquad .$
A, B, C, a	Area	$Area = \frac{a^2 \sin B \sin C}{2 \sin A}$
C, a, b		$\frac{1}{2}(A+B) = 90^{\circ} - \frac{1}{2}C$
	$\frac{1}{2}(A-B)$	$\tan \frac{1}{2}(A - B) = \frac{a - b}{a + b} \tan \frac{1}{2}(A + B)$
- 6	\boldsymbol{A}	$A = \frac{1}{2} (A + B) + \frac{1}{2} (A - B)$
	В	$B = \frac{1}{2} (A + B) - \frac{1}{2} (A - B)$
	c	$c = (a + b) \frac{\cos \frac{1}{2} (A + B)}{\cos \frac{1}{2} (A - B)}$
		- ` '
		$= (a - b) \frac{\sin \frac{1}{2} (A + B)}{\sin \frac{1}{2} (A - B)}$
	Area	$Area = \frac{1}{2} a b \sin C$
a, b, c	A	Let $s = \frac{1}{2}(a+b+c)$
., ., .		
		Then $\sin \frac{1}{2}A = \sqrt{\frac{(s-b)(s-c)}{bc}}$
		$\cos \frac{1}{2}A = \sqrt{\frac{s(s-a)}{bc}}$
		$\tan \frac{1}{2}A = \sqrt{\frac{(s-b)(s-c)}{s(s-a)}}$
	B, C	Similar formulas
	Area	$\sqrt{s(s-a)(s-b)(s-c)}$

C		Dis	t. 1	Dis	t. 2	Dis	t. 3	Dis	t. 4	Dist	t. 5	1	
Cou	r80	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	-	
ô	15	1.0000	0.0044	2.0000	0.0087	3 0000	0.0131		0.0175	5.0000	0.0218	89	45
	30	0000	0087	1 9999		2.9999	0262	3.9998	0347	4.9998	0436		30
1	45	0.9999	0131	9998	0262	9997	0393 0524	9997	0524 0698	9996	0654	89	15
	15	9998	0218	9995	0436	9993	0654	9990	0873	9988	1091	00	45
	30	9997	0262	9993	0524	9990	0785	9986	1047	9983	1309		30
-	45	9995	0305	9991	0611	9986	0916	9981	1222	9977	1527	00	15
2	15	9994 9992	0349	9988	0698 0785	9982	1047 1178	9976 9969	1396 1570	9970 9961	1745 1963	88	0 45
	30	9990	0436	9981	0872	9971	1309	9962	1745	9952	2181		30
	45		0.0480	1.9977	0.0960	2.9965	1	3.9954	0.1919	4.9942	-		15
3	0	9986	0523	9973	1047	9959	1570	9945	2093	9931	2617	87	0
	15	9984	0567	9968	1134	9952	1701	9936	2268	9920	2835		45
	30	9981	0610	9963 9957	1221 1308	9944	1831 1962	9925 9914	2442 2616	9907	3052 3270		30
4	45	9979 9976	0698	9951	1395	9927	2093	9903	2790	9878	3488	86	15
_	15	9973	0741	9945	1482	9918	2223	9890	2964	9863	3705	-	45
	30	9969	0785	9938	1569	9908	2354	9877	3138	9846	3923		30
5	45	9966	0828 0872	9931	1656 1743	9897	2484 2615	9863 9848	3312 3486	9828 9819	4140 4358	85	15
O	15	9962			0.1830	1	0.2745	3.9832			0.4575	99	0 45
	30	0.9958 9954	0.0915	9908	1917	9862	2875	9816	3834	9770	4792		30
	45	9950	1002	9899	2004	9849	3006	9799	4008	9748	5009		15
6	0	9945	1045	9890	2091	9836	3136	9781	4181	9726	5226	84	0
	15	9941	1089	9881	2177	9822	3266	9762	4355	9703	5443		45
	30 45	9936 9931	1132 1175	9871 9861	2264 2351	9807 9792	3396 3526	9743 9723	4528 4701	9679 9653	5660 5877		30 15
7	0	9925	1219	9851	2437	9776	3656	9702	4875	9627	6013	83	0
	15	9920	1262	9840	2524	9760	3786	9680	5048	9600	6390		45
	30	9914	1305	9829	2611	9743	3916	9658	5221	9572	6526		30
_	45	0.9909		1.9817	0.2697		0.4046	3.9635	0.5394		0.6743	-	15
8	15	9903 9897	1392 1435	9805 9793	2783 2870	9708 9690	4175 4305	9611 9586	5561 5740	9513 9483	6959 7175	82	0 45
	30	9890	1478	9780	2956	9670	4434	9561	5912	9451	7390		30
	45	9884	1521	9767	3042	9651	4564	9534	6085	9418	7606		15
9	0	9877	1564	9754	3129	9631	4693	9508	6256	9384	7822	81	0
	15	9870 9863	1607 1650	9740 9726	3215	9610 9589	4822 4951	9480 9451	6430 6602	9350 9314	8037 8252		45 30
	45	9856	1693	9711	3387	9567	5080	9422	6774	9278	8467		15
10	0	9848	1736	9696	3473	9544	5209	9392	6946	9240	8682	80	0
	15	0.9840	0.1779	1.9681	0.3559		0.5338	3.9362	0.7118		0.8897		45
	30	9833	1822	9665	3645	9498	5467	9330	7289	9163	9112 9326		30 15
11	45	9825 9816	1865 1908	9849 9633	3730 3816	9474	5596 5724	9298 9265	7461 7632	9123	9540	79	0
	15	9808	1951	9616	3902	9424	5853	9231	7804	9039	9755		45
	30	9799	1994	9598	3987	9398	5981	9197	7975	8996	9968		30
10	45	9790	2036	9581	4073	9371	6109	9162	8146			78	15
12	0 15	9781 9772	2079 3122	9563 9545	4158	9344 9317	6237 6365	9126 9089	8316 8487	8907 8862	0396	10	45
	30	9763	2164	9526	4329	9289	6493	9052	8658	8815	0822		30
	45	0.9753	0.2207	1.9507	0 4414	2.9260	0.6621	3.9014	0.8828	4.8767	1.1035		15
13	0	9744	2250	9487	4499	6231	6749	8975	8998	8719	1248	77	0
	15	9734	2292	9468	4584	9201	6876	8935	9168	8669	1460		45
	30	9724 9713	2334 2377	9447	4669 4754	9171	7003	8895 8854	9338 9507	8618 8567	1672 1884		30 15
14	0	9703	2419	9406	4838	9109	7258	8812	9677	8515	2096	76	0
	15	9692	2462	9385	4923	9077	7385	8769	9846	8462	2308		45
	30 45	9681 9670	2504	9363	5008	9044	7511		1.0015	8407 8352	2519 2730		30 15
15	0	9659	2546 2588	9341 9319	5092 5176	9041 8978	7638 7765	8682	0184	8296	2941	75	0
	_	-	-		-	-					-	-	
		Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Co	urse
		Dia	t. 1	Dia	6. 2	Dist	. 3	Dis	C. 4	Dis	t. D		

	Die	st. 6	Dis	st. 7	Dis	st. 8	I Die	st. 9	II Die	t. 10	1
Course		Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	
0 15 30 45 1 0 15 30 45 2 0 15	9995 9991	0.0262 0524	6.9999 9997 9994 9989 9983 9976 9967 9957 9946 9933	0.0305 0611 0916 1222 1527 1832 2138 2443 2748 3053	7.9999 9997 9993 9988 9981 9973 9963 9951 9938 9924	0.0349 0698 1047 1396 1745 2094 2443 2792 3141 3490	8,9999 9997 9992 9986 9979 9969 9058 9945 9931	0.0393 0785 1178 1571	9,9999 9996 9991 9985 9976 9966 9053 9939 9923 9905	0.0130	15 89 0 45 30 15 88 0 45
45 3 0 15 30 45 4 0 15 30 45 5 0	5.9931 9918 9904 9888 9872 9854 9835 4815 9794 9772	0.2879 3140 3402 3663 3924 4185 4447 4708 4968 5229	6.9919 9904 9887 9869 9850 9829 9808 9784 9760 9734			0.3838 4187 4535 4884 5232 5581 5929 6277 6625 6972		0.4318 4710 5102 5494 5886 6278 6670 7061 7453 7844	9.9885 9863 9839 9813 9786 9756 9725 9692 9657 9619		15 37 0 45 30 15 86 0 45 30 15
15 30 45 6 0 15 30 45 7 0 15 30	5.9748 9724 9698 9671 9643 9614 9584 9553 9520 9487		6.9706 9678 9648 9617 9584 9550 9515 9478 9440 9401		7.9664 9632 9597 9562 9525 9486 9445 9404 9316		8.9622 9586 9547 9507 9465 9421 5378 9329 9280 9230	0.8235 8626 9017 9408 9798 1.0188 0578 0578 0968 1358 1747	9.9580 9540 9497 9452 9406 9357 9307 9255 9200 9144		45 84 0 45 83 0 45
45 8 0 15 30 45 9 0 15 30 45 10 0	5.9452 9416 9379 9341 9302 9261 9220 9177 9133 9088	0.8091 8350 8610 8869 9127 9386 9645 9903 1.0161 0419	6.9361 9319 9276 9231 9185 9138 9090 9040 8989 8937	0.9440 9742 1.0044 0347 0649 0950 1252 1553 1854 2155	7.9269 9221 9172 9121 9069 9015 8960 8903 8844 8785	1.0788 1134 1479 1825 2170 2515 2859 3204 3548 3892	8,9178 9124 9069 9011 8953 8892 8830 8766 8700 8633		9.9087 9027 8965 8902 8836 8769 8700 8629 8556 8481	1.3485 3917 4349 4781 5212 5643 6074 6505 6935 7365	82 0 45 90 15 81 0 45 30 15
15 30 45 11 0 15 30 45 12 0 15 30	5.9042 8995 8947 8898 8847 8795 8743 8689 8634 8578	1.0677 0934 1191 1449 1705 1962 2219 2475 2731 2986	6.8883 \$728 \$772 8714 8655 8595 8533 8470 8406 8341	1.2456 2756 3057 3357 3656 3956 4255 4554 4852 5151	7.8723 8660 8596 8530 8463 8394 8324 8252 8178 8104	1.4235 4579 4922 5265 5607 5949 6291 6633 6974 7315	8.8564 8493 8421 8346 8271 8193 8114 8033 7951 7867	1.6015 6401 6787 7173 7558 7943 8328 8712 9096 9480	9.8404 8325 8245 8163 8079 7992	1.7794 8224 8652 9081 9509 9937 2.0364 0791 1218 1644	45 30 15 79 0 45 30 15
45 13 0 15 30 45 14 0 15 30 45 15 0	5.8521 8462 8403 8342 8281 8218 8154 8089 8023 7956	1.3242 3497 3752 4007 4261 4515 4739 5023 5276 5529			7.8027 7950 7870 7790 7707 7624 7538	1.7656 7996 8336 8676 9015 9354 9692 2.0030 0368 0706	8.7781	1.9863 2.0246 0628 1010 1392 1773 2154 2534 2914 3294	9.7534 7437 7338 7237 7134 7030 6923 6815 6705 6593		15 77 0 45 30 15 76 0 45 30 15
	Dep.	Lat.	Dep.	Lat.	Dep.J.	Lat.	Dep.	Lat.	Dep.	Lat.	Course

-	Die	st. 1	Dis	st. 2	Die	st. 3	Die	st. 4	Dia	st. 5	1
Course	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	
15 18	9636	0.2630	9273	0.5261	8909	0.7891 8017	3.8591 8545	1.0521 0690	4.8239	1.315	74 45
16 0	9613 9600	2756 2798	9249 9225 9201	5429 5513 5597	8874 8838 8801	8143 8269 8395	8498 8450 8402	1025 1193	8123 8063 8002	3789 399	74 0
17 0 15	9576 9563	2840 2882 2924 2965	9176 9151 9126 9100	5680 5764 5847 5931	8765 8727 8689 8651	8520 8646 8771 8896	8353 8303 8252 8201	1528	7941 7879 7815 7751	420: 4410 4613 482	15 73 0
45	9537 0.9524	3007 0.3049	9074 1.9048	6014 0.6097	8612 2.8572	9021 0.9146	8149 3.8096	2028 1.2195	7686 4.7620	5038 1.5243	30
18 0 15 30 45	9497 9483 9469	3090 3132 3173 3214	9021 8994 8966 8939	6180 6263 6346 6429 6511	8532 8491 8450 8408	9271 9395 9519 9643	8042 7988 7933 7877	2361 2527 2692 2858	7553 7485 7416 7347	5678 5868 6079	30 30 15
19 0 15 30 45 20 0	9441 9426 9412	3256 3297 3338 3379 3420	8910 8882 8853 8824 8794	6594 6676 6758 6840	8366 8323 8279 8235 8191	9767 9891 1.0014 0138 0261	7821 7764 7706 7647 7588	3023 3188 3352 3517 3681	7276 7204 7132 7059 6985	6278 6488 6690 6890 7101	45 30 15
15 30 45 21 0	0.9382 9367 9351		1.8764 8733 8703 8672	0.6922 7004 7086 7167	2.8146 8100 8054 8007	1.0384 0506 0629 0751	3.7528 7467 7405 7343		4.6910 6834 6757 6679		45 30 15
15 30 45 22 0	9320 9304 9288 9272	3624 3665 3706 3746	8640 8608 8576 8544	7249 7330 7411 7492	7960 7913 7864 7816	0873 0995 1117 1238	7280 7217 7152 7087	4498 4660 4822 4984	6600 6521 6440 6359	8122 8325 8528 8730	45 30 15 68 0
15 30 45	9239 0.9222		8511 8478 1.8444	7573 7654 0.7734	7766 7726 2.7666	1359 1481 1.1601	7022 6955 3.6888		6277 6194 4.6110	893: 9134 1.9336	30
23 0 15 30 45	9188 9171	3907 3947 3987 4027	8410 8376 8341 8306	7815 7895 7975 8055	7615 7564 7512 7459	1722 1842 1962 2082	6820 6752 6682 6612	5629 5790 5950 6110	5940 5853 5766	9537 9737 9937 2.0137	67 0 45 30 15
24 0 15 30 45	9118 9100	4067 4107 4147 4187	8271 8235 8199 8163	8135 8214 8294 8373	7406 7353 7299 7214	2202 2322 2441 2560	6542 6470 6398 6326	6569 6429 6588 6746	5677 5588 5498 5407	0337 0536 0735 0933	45 30
25 0	9063 0.9045	4226 0.4266 4305	8126	8452 0.8531 8610	7189 2.7034 7078	2679 1.2797 2915	6252 3.6178 6103	6905 1.7063 7220	5315 4.5223 5129	1131 2.1328 1526	
26 0 15	9007 8988 8969	4344 4384 4423	8014 7976 7937	8689 8767 8846	7021 6964 6906	3033 3151 3269	6028 5952 5875	7378 7535 7692	5035 4940 4844	1722 1919 2114	64 0 45
30 45 27 0 15	8930 8910 8890	4462 4501 4540 4579	7899 7860 7820 7780	8924 9002 9080 9157	6848 6789 6730 6671	3386 3503. 3620, 3736	5797 5719 5640 5561	7848 8004 8160 8315	4747 4649 4550 4451	2310 2505 2700 2894	45
30 45 28 0 15	0.8850	4695	7740 1.7700 7659	9389	2.6550 6488	3852 1.3968 4084	5318	8470 1.8625 8779	4351 4.4249 4147 4045	3474	15 62 0
30 40 29 0	8788 8767	4733 4772 4810 4848	7618 7576 7535 7492	9466 9543 9620 9696	6427 6365 6302 6239	4200 4315 4430 4544	5236 5153 5069 4985	8933 9086 9240 9392	3941 3836 3731	3666 3858 4049 4240	45 30 15 61 0
15 30 45	8725 8704 8682	4886 4924 4962	7450 7407 7364	9772 9848 9924	6175 6111 6046	4659 4773 4886	4900 4814 4728	9545 9697 9849	3625 3518 3410	4431 4621 4811	45 30 15
30 0	8660 Dep.	5000 Lat.	7321 Dep.	1.0000 Lat.	5981 Dep.	5000	-	2.0000	3301 Dan	5000	60 0
		t. 1	Dep.		Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Course

TRAVERSE TABLE 21												
Course		st. 6	Die	st. 7	Die	st. 8	Dis	t. 9	Dia	t. 10		T
	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	-	-
15 15 30 45 16 0 15	5.7887 7818 7747 7676 7603	6034 6286 6538	6.7335 7454 7372 7288 7203	8707 9001 9295	7.7183 7090 6990 6901 6804	1715 2051	8.6831 6727 6621 6514 6404		9.6479 6363 6246 6126 6008	7144 7564	74	30 15
30 45 17 0 15 30	7529 7454 7378 7301 7223	7041 7292 7542 7792	7117	9881 2.0174 0466 0758 1049	6706 6606 6504 6402 6297	2721 3056	6294 6181 6067 5952 5835	5561 5938 6313 6689	5882 5757 5630 5502 5372	8820 9237 965-	73	30 15
	5.7144 7063 6982 6899 6816	1.8292 8541 8790 9038	6.6668 6574 6479 6383 6285	2,1341 1631 1921		2.4389 4721 5053		2.7438 7812 8185 8557		3.0486 0902 1316 1736	72	15
19 0 15 30 45 20 0	6731 6645 6658 6471 6382	9534 9781 2.0028 0275 0521	6186 6086 598 5 5882 5778	2790 3078 3366 3654 3941	5641 5527 5411 5294 5175	6045 6375 6705 7033 7362	5097 4968	9301 9672 3.0043 0413	4552 4409 4264 4118 3969	2557 2969 3381 3799 4209	71	0 45 30 15 0
15 30 45 21 0 15 30	5.6291 6200 6108 6015 5920	1012 1257 1502 1746	6 5673 5565 5459 5351 5241	4515 4800 5086 5371	4934 4811 4686 4561	2.7689 8017 8343 8669 8995	8.4437 4300 4162 4022 3881	1519 1886 2253 2619	3767 3514 3358 3201	5429 5837 6244	69	45
22 0 15 30	5825 5729 5631 5532 5433	2233 2476 2719 2961	5129 5017 4903 4788 4672	5655 5939 6222 6505 6788	4433 4305 4176 4043 3910	0615	3738 3593 3447 3299 3149	3350 3715 4078 4442	3049 2881 2718 2554 2388	7056 7461 7865 8268	68	30 15 0 45 80
23 0 15 30 45	5230 5127 5024 4919	3685 3925 4165	6.4554 4435 4315 4194 4072	2.7070 7351 7632 7912 8192	7.3776 3640 3503 3365 3225	3.0937 1258 1580 1900 2220	8.2998 2845 2691 2535 2375	3.4804 5166 5527 5887 6247	2050 1879 1706	9474		15 0 45 30 15
24 0 15 30 45 25 0	4813 4706 4598 4489 4378	4643 4882 5120	3948 3823 3697 3570 3442	8472 8750 9029 9306 9583	3084 2941 2797 2651 2505	2539 2858 3175 3493 3809	2219 2059 1897 1733 1568	6606 6965 7322 7679 8036	1355 1176 0996 0814 0631			0 45 30 15 0
	5.4267 4155 4042 3928 3812	2.5594 5831 6067 6302	6.3312	2.9800 3.0136 0411 0686 0960		3.4125 4441 4756 5070 5383	8.1401 1233 1063 0891 0719	3.8391 8746 9100 9453 9806	9.0446 0259 0070 8.9879 9687			45 30 15 0 45
30 45 27 0 15 30	3696 3579 3460 3341 3221	6772	2645 2509 2370 2231 2091	1234 1507 1779 2051	1595 1438 1281 1121 0961	5696 6008 6319 6630 6940		4.0158 0509 0859 1209 1557	9493 9298 9101 8902 8701	4620 5010 5399 5787 6175	63	30 15 0 45 30
	5.3099 2977 2853 2729 2604		6.1949 1806 1662 1517 1371	3.2593	7.0799 0636 6471 0305 0138		7.9649 9465 9280 9094 8905		8.8499 8295 8089 7882 7673		62	15 0 45 50 15
29 0 15 30 45 30 0	2477 2350 2221 2092 1962	9089 9317 9545 9773	1223 1075 0925 0774 0622	3937 4203 4470 4735 5000	6.9970 9800 9628 9456 9282	8785 9090 9394 9697 4.0000	8716 8525 8332 8148 7942	3683 3976 4318 4659 5000	7462 7250 7036 6820	8481 8862 9242 9622		0 45 30 15 0
	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	-	_
		t. 6	Dis		Dis.	-	Dist		Dist.		Cou	ILSE

Corre	0.05	Du	t. 1	Die	st. 2	Die	st. 3	D18	t. 4	1016	st. 5		
Cour	88	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.		
30	15	0.8638	0.5038	1.7277	1.0075	2,5915	1.5113	3,4553	2.0151	4.3192	2,5189	59	45
00	30	8616	5075	7223	0151	5849	5226	4465	0302	3081	5377	1	30
	45	8594	5113	7188	0226	5782	5339	4376		2970	5565	1	15
31	0	8572	5150	7142	0301	5715	5451	4287	0602	2858			(
0.1	15	8549	5188	7098	0375	5647	5563	4196		2746		1	45
	30	8526	5225	7053	0450	5579	5675	4106	0900	2632	6125		36
	45	8504	5262	7007	0524	5511	5786	4014	1049	2518			1
32	0	8480	5299	6961	0598	5441	5898	3922		2402		58	10
02	15	8457	5336	6915	0672	5372	6008	3829	1345	2286		90	4
	30	8434	5373	6868	0746	5302	6119	3736	1492	2170			30
		-											
		0.8410			1.0819		1.6229		2.1639		2.7049		1
33	0	8387	5446	6773	0893	5160	6339	3547	1786	1934	7232		
	15	8363	5483	6726	0966	5089	6449	3451	1932	1814	7415		4
	30	8339	5519	6678	1039	5017	6558	3355	2077	1694	7597		31
	45	8315	5556	6629	1111	4944	6667	3259	2223	1573	7779	1	1
34	0	8290	5592	6581	1184	4871	6776	3162	2368	1452	7960		
	15	8266	5628	6532	1256	4798	6884	3064	2512	1329	3140		4
	30	8241	5664	6483	1328	4724	6992	2965	2656	1206			30
	45	8216	5700	6433	1400	4649	7100	2866	2800	1082	8500		1
35	0	8192	5736	6383	1472	4575	7207	2766	2943	0958	8679	55	(
	- 1	0.8166			1.1543	2.4499			2.3086	4.0832			4!
	30	8141	5807	6282	1614	4423	7421	2565	3228	0706			30
	45	8116	5842	6231	1685	4347	7527	2463	3370	0579			1
36			5878	6180	1756	4271	7634	2361	3511	0451	9389		1
	0	8090						2258		0322	9565		4
	15	8064	5913	6129	1826	4193	7739		3652				
	30	8039	5948	6077	1896	4116	7845	2154	3793	0193	9741		3
	45	8013	5983	6025	1966	4038	7950	2050	3933	0063			1
37	0	7986	6018	5973	2036	3959	8054	1945	4073		3.0091		-
	15	7960	6053	5920	2106	3880	8159	1840	4212	9800	0365		4
	30	7934	6088	5867	2175	3801	8263	1734	4350	9668	0438	1	30
	45	0.7907	0.6122	1.5814	1.2244	2.3721	1.8367	3.1628	2.4489	3,9534	3.0611	1	15
38	0	7880	6157	5760	2313	3640	8470	1520		9400	0783	52	(
	15	7853	6191	5706	2482	3560	8573	1413		9266			45
	30	7826	6225	5652	2450	3478	8675	1304	4901	9130	1126		Die
	45	7799	6259	5598	2518	3397	8778	1195		8994	1296		14
39	0	7771	6293	5543	2586	3314	8880	1086		8857	1466		
	15	7744	6327	5488	2654	3232	8981	0976	5308	8720	1635		4
	30	7716	6361	5432	2722	3149	9082	0865	5443	8581	1804		30
	45	7688	6394	5377	2789	3065	9183	0754	5578	8442	1972	1	13
40	0	7660	6428	5321	2856	2981	9284	0642	5512	8302	2139	50	-
	- 1											00	
		0.7632			1.2922	2.2897	1.9384		2.5845	3.8162			4
	30	7604	6494	5208	2989	2812	9463	0416	5978	8020	2472	1	3
	45	7576	6528	5151	3055	2727	9583	0303	6110	7878	2638		1
41	0	7547	6561	5094	3121	2641	9682	0188	6242	7735	2803	49	
	15	7518	6593	5037	3187	2555	9780	0074	6374	7592	2967		4
	30	7490	6626	4979	3252	2469	9879	2.9958	6505	7448	3131		3
	45	7461	6659	4921	3318	2382	9976	9842	6635	7303	3294		1
42	0	7431	6891	4863	3383	2294	2.0074	9726	6765	7157	3457	48	K
	15	7402	6724	4-04	3447	2207	0171	9609	6895	7011	3618		4
	30	7373	6756	4746	3512	2118	0268	9491	7024	6864	3780		3
	45	0.7343	0.6788	1.4686	1 3576	2.2030	2 0364	2.9373	2.7159	3.6716	3,3940		1
43	0	7314	6820	4627	3640	1941	0460	9254	7280	6568	4100	47	
-	15	7284	6852	4567	3704	1851	0555	9135	7407	6419	4259		4
	30	7254	6884	4507	3767	1761	0651	9015	7534	6268	4118		30
	45	7224	6915	4447	3830	1671	0745	8895	7661	6118	4576		18
44	0	7193	6947	4387	3893	1580	0840	8774	7786	5967	4733	46	(
**	15	7163	6978	4326	3956	1489	0934	8652	7912	5815	4890	Z C	4!
	30	7133	7009	4265	4018	1398		8530	8036	5663	5045		36
	45	7102	7040	4204	4080	1306	1027 1120	8407	8161	5509	5201		12
45							1213	8284	8284	5355	5355	15	14
19(1)	0	7071	7071	4142	4142	1213	1213	0204	0504	()()()	()()()()	247	-
	-	-	-	-	7 /	-	Lat.	-	-	-	* .	-	-
		Dep.	Lat	Dep.	Lat.	Dep.	LASET.	Dep.	Lat.	Dep.	Lat.		

	Die	t. 6	Dis	t. 7	Die	st. 8	Di	st. 9	Dis	t. 10	F	-
Course	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	1	
30 15	5 1830	3.0226	6 0468	3.5264	6 9107	4.0302	7 7745	4 5340	8 6384	5.0377	FIRE	1
30	1698		0314	5528	8930		7547	5678	6163	0759		20
45	1564	0678	0158	5791	8753		7347	6016	5941			15
31 0 15	1430 1295	0902 1126	5,9844	6053	8573 8393	1203 1502	7145 6942	6353 6690	5717 5491			45
, 30	1158	1350	9685	6575	8211	1800	6738	7025	5264	12250)	30
45	1021	1573	9525	6835	8028	2097 2394	6532	7359	5035			15
32 0 15	0883 0744	1795 2017	9363 9201	7094 7353	7844 7658	2689	6324	7693 8025	4805			45
30	0603	2238	9037	7611	7471	2984	5905	8357	4339			30
45		3.2458		3.7868		4.3278		4.8688		5.4097		15
38 0 15	0320	2678 2898	8707 8540	8125 8381	7094 6903	3571	5480 5266	9018 9346	3867	4464		0 45
30		3116	8372	3636	6711	4155	5050	9674	3389			30
45	4.9888	3334	8203	8890	6518	4446	4832	5.0001	3147	5557		15
34 0 15	9742 9595	3552 3768	8033 7861	9144 9396	6323 6127	4735 5024	4613 4393	0327 0652	2904 2659			0 45
30	9448	3984	7689	9648	5930		4171	0977	2413			30
45	9299	4200	7515	9900	5732	5600	3948	1300	2165			15
35 0	9149			4.0150	5532	5886	3724	1622	1915			0
15 30	4.8998	3.4629	5.7165 6988	4.0400	6.5331	4.6172	7.3498	5.1943 2263	8.1664 1412	5.7715		45
45	8694	5055	6810	0897	4926	6740	3042	2582	1157			15
36 0	8541	5267	6631	1145	4721	7023	2812	2901	0902		54	0
15 30	8387 8231	5479	6451 6270	1392 1638	4516 4309	7305 7586	2580 2347	3218 3534	0644			45
45	8075	5899	6088	1883	4100	7866	2113	3849	0125			15
37 0	7918	6109	5904	2127	3891	8145	1877	4193		6.0182		0
15 30	7760 7601	6318 6526	5720 5535	2371 2613	3680 3468	8424 8701	1640 1402	4476 4789	9600 9335	0529 0876		45 30
45		3.6733	5.5348		6.3255	4.8977	7.1162			6.1222		15
38 0	7281	6940	5161	3096	3041	9253	0921	5410	8801	1566		0
15	7119	7146	4972	3337	2829	9528	0679	5718	8532	1909		45
30 45	6956 6793	7351 7555	4783 4592	3576 3815	2609	9801	0435	6026 6333	8261 7988	2251 2592		30 15
39 0	6629	7759	4400	4052	2172	0346	6.9943	6639	7715	2932	51	0
15 30	6464	7962	4207	4289	1951	0616 0886	9695	6943 7247	7439 7162	3271 3608		45
45	6297 6131	8165	4014 3819	4525	1730 1507	1155	9196	7550	6884	3944		15
40 0	5963	8567	3623	4995	1284	1423	8944	7851	6604	4279	50	0
	4.5794	3.8767	5.3426		6.1059			5.8151				45
30 45	5624 5454	8967 9166	3228 3030	5461 5693	0832 0605	1956 2221	8437 8181	8450 8748	6041 5756	4945 5276		30
41 0	5283	9364	2830	5924	0377	2485	7924	9045	5471	5606		0
15	5110	9561	2629	6154	0147	2748	7666	9341	5184	5935		45
30 45	4937 4763	9757 9953	2427 2224	6383 6612	5.9916 9685	3010 3271	7406 7145	9638	4896 4606	6282 6588		30
42 0		4.0148	2020	6839	9452	3530	6883	6.0222	4314	6913	48	0
15	4413	0342	1815	7066	9217	3789	6620	0513	4022	7237		45
30	4237	0535	1609	7291	8982	4047	6355	0803	3728	7559		30
43 0	4.4059	4.0728	5.1403 1195	4.7516 7740	5.8746 8508	5.4304	6.6089 5822	6.1092	7.3432	6.7880		15
15	3702	1111	0986	7963,	8270	4815	5553	1666	2837	8518		15
30	3522	1301	0776	8185	8030	5068	5284	1952	2537	8835		10
45	3342 3160	1491	0565 0354	8406	7789 7547	5321 5573	5013	2236 2519	2236 1934	9151 9466		15
15	2978	1867	0141	8845	7304	5823,	4467	2801	1630	9779		15
30	2795	2055	4.9928	9064	7060	6073	4193	3082		7.0091		10
45 45 0	2611 2426	2241 2426	9713 9477	9281 9497	6815 6569	6321	3917	3361	1019 0711	0401	45	0
							-					
	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Dep.	Lat.	Cou	rse
	Dis	t. 6	Dis	6. 6	Dis	0. 8	Dist. 9		Dist			

LOGARITHMS OF NUMBERS

No.	0	1	2	3	4	5	6	7	8	9
10	0000	0043	0086	0128	0170	0212	0253	0294	0334	0374
11	0414	0453	0492	0531	0569	0607	0645	0682	0719	0755
12	0792	0828	0864	0899	0934	0969	1004	1038	1072	1106
13	1139	1173	1206	1239	1271	1303	1335	1367	1399	1430
14	1461	1492	1523	1553	1584	1614	1644	1673	1703	1732
15	1761	1790	1818	1847	1875	1903	1931	1959	1987	2014
16	2041	2068	2095	2122	2148	2175	2201	2227	2253	2279
17	2304	2330	2355	2380	2405	2430	2455	2480	2504	2529
18	2553	2577	2601	2625	2648	2672	2695	2718	2742	2765
19	2788	2810	2833	2856	2878	2900	2923	2945	2967	2989
20	3010	3032	3054	3075	3096	3118	3139	3160	3181	3201
21	3222	3243	3263	3284	3304	3324	3345	3365	3385	3404
22	3424	3444	3464	3483	3502	3522	3541	3560	3579	3598
23	3617	3636	3655	3674	3692	3711	3729	3747	3766	3784
24	3802	3820	3838	3856	3874	3892	3909	3927	3945	3962
25	3979	3997	4014	4031	4048	4065	4082	4099	4116	4133
26	4150	4166	4183	4200	4216	4232	4249	4265	4281	4298
27	4314	4330	4346	4362	4378	4393	4409	4425	4440	4456
28	4472	4487	4502	4518	4533	4548	4564	4579	4594	4609
29	4624	4639	4654	4669	4683	4698	4713	4728	4742	4757
30	4771	4786	4800	4814	4829	4843	4857	4871	4886	4900
31	4914	4928	4942	4955	4969	4983	4997	5011	5024	5038
32	5051	5065	5079	5092	5105	5119	5132	5145	5159	5172
33	5185	5198	5211	5224	5237	5250	5263	5276	5289	5302
34	5315	5328	5340	5353	5366	5378	5391	5403	5416	5428
35	5441	5453	5465	5478	5490	5502	5514	5527	5539	5551
36	5563	5575	5587	5599	5611	5623	5635	5647	5658	5670
37	5682	5694	5705	5717	5729	5740	5752	5763	5775	5786
38	5798	5809	5821	5832	5843	5855	5866	5877	5888	5899
39	5911	5922	5933	5944	5955	5966	5977	5988	5999	6010
40	6021	6031	6042	6053	6064	6075	6085	6096	6107	6117
41	6128	6138	6149	6160	6170	6180	6191	6201	6212	6222
42	6232	6243	6253	6263	6274	6284	6294	6304	6314	6325
43	6335	6345	6355	6365	6375	6385	6395	6405	6415	6425
44	6435	6444	6454	6464	6474	6484	6493	6503	6513	6522
45	6532	6542	6551	6561	6571	6580	6590	6599	6609	6618
46	6628	6637	6646	6656	6665	6675	6684	6693	6702	6712
47	6721	6730	6739	6749	6758	6767	6776	6785	6794	6803
48	6812	6821	6830	6839	6848	6857	6866	6875	6884	6893
49	6902	6911	6920	6928	6937	6946	6955	6964	6972	6981
50	6990	6998	7007	7016	7024	7033	7042	7050	7059	7067
51	7076	7084	7093	7101	7110	7118	7126	7135	7143	7152
52	7160	7168	7177	7185	7193	7202	7210	7218	7226	7235
53	7243	7251	7259	7267	7275	7284	7292	7300	7308	7316
54	7324	7332	7340	7348	7356	7364	7372	7380	7388	7396
No.	0	1	2	3	4	5	6	7	8	9

LOGARITHMS OF NUMBERS

No.	0	1	2	3	4	5	6	7	8	9
55	7404	7412	7419	7427	7435	7443	7451	7459	7466	7474
56	7482	7490	7497	7505	7513	7520	7528	7536	7543	7551
57	7559	7566	7574	7582	7589	7597	7604	7612	7619	7627
58	7634	7642	7649	7657	7664	7672	7679	7686	7694	7701
59	7709	7716	7723	7731	7738	7745	7752	7760	7767	7774
60	7782	7789	7796	7803	7810	7818	7825	7832	7839	7846
61	7853	7860	7868	7875	7882	7889	7896	7903	7910	7917
62	7924	7931	7938	7945	7952	7959	7966	7973	7980	7987
63	7993	8000	8007	8014	8021	8028	8035	8041	8048	8055
64	8062	8069	8075	8082	8089	8096	8102	8109	8116	8122
65	8129	8136	8142	8149	8156	8162	8169	8176	8182	8189
66	8195	8202	8209	8215	8222	8228	8235	8241	8248	8254
67	8261	8267	8274	8280	8287	8293	8299	8306	8312	8319
68	8325	8331	8338	8344	8351	8357	8363	8370	8376	8382
69	8388	8395	8401	8407	8414	8420	8426	8432	8439	8445
70	8451	8457	8463	8470	8476	8482	8488	8494	8500	8506
71	8513	8519	8525	8531	8537	8543	8549	8555	8561	8567
72	8573	8579	8585	8591	8597	8603	8609	8615	8621	8627
73	8633	8639	8645	8651	8657	8663	8669	8675	8681	8686
74	8692	8698	8704	8710	8716	8722	8727	8733	8739	8745
75	8751	8756	8762	8768	8774	8779	8785	8791	8797	8802
76	8808	8814	8820	8825	8831	8837	8842	8848	8854	8859
77	8865	8871	8876	8882	8887	8893	8899	8904	8910	8915
78	8921	8927	8932	8938	8943	8949	8954	8960	8965	8971
79	8976	8982	8987	8993	8998	9004	9009	9015	9020	9025
80	9031	9036	9042	9047	9053	9058	9063	9069	9074	9079
81	9085	9090	9096	9101	9106	9112	9117	9122	9128	9133
82	9138	9143	9149	9154	9159	9165	9170	9175	9180	9186
83	9191	9196	9201	9206	9212	9217	9222	9227	9232	9238
84	9243	9248	9253	9258	9263	9269	9274	9279	9284	9289
85	9294	9299	9304	9309	9315	9320	9325	9330	9335	7340
86	9345	9350	9355	9360	9365	9370	9375	9380	9385	9390
87	9395	9400	9405	9410	9415	9420	9425	9430	9435	9440
88	9445	9450	9455	9460	9465	9469	9474	9479	9484	9489
89	9494	9499	9504	9509	9513	9518	9523	9528	9533	9538
90	9542	9547	9552	9557	9562	9566	9571	9576	9581	9586
91	9590	9595	9600	9605	9609	9614	9619	9624	9628	9633
92	9638	9643	9647	6952	9657	9661	9666	9671	9675	9680
93	9685	9689	9694	9699	9703	9708	9713	9717	9722	9727
94	9731	9736	9741	9745	9750	9754	9759	9763	9768	9773
96 97 98 99	9777 9823 9868 9912 9956	9782 9827 9872 9917 9961	9786 9832 9877 9921 9965	9791 9836 9881 9926 9969	9795 9841 9886 9930 9974	9800 9845 9890 9934 9978	9805 9850 9894 9939 9983	9809 9854 9899 9943 9987	9814 9859 9903 9948 9991	9818 9863 9908 9952 9996
No.	0	1	2	3	4	5	6	7	В	9

LOGARITHMIC SINES, COSINES,

Angle	Sin.	D. 1'	Cos.	D. 1'	Tan.	D. 1'	Cot.	
0° 0′	- ∞	- 11	10.0000		— ∞		100	90° 0′
0° 10′ 0° 20′ 0° 30′ 0° 40′ 0° 50′ 1° 10′ 1° 20′ 1° 30′ 1° 40′ 1° 50′ 2° 0′ 2° 20′ 2° 30′ 2° 40′ 2° 50′ 3° 0′	7.4637 .7648 .9408 8.0656 .1627 8.2419 .3088 .3668 .4179 .4637 .5050 8.5428 .5776 .6097 .66397 .66940 8.7188	301.1 176.0 125.0 96.9 79.2 66.9 58.0 51.1 45.8 41.3 37.8 34.8 32.1 30.0 26.3 24.8 23.5	.0000 .0000 .0000 .0000 .0000 .9999 .9999 .9998 .9998 .9998 .9996 .9996 .9996 .9996 .9996	.0 .0 .0 .0 .1 .0 .0 .1 .0 .1 .0 .1 .0 .1 .0 .1 .0 .1 .0 .1 .0 .1 .0 .1 .0 .1 .0 .1 .0 .1 .0 .1 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0	7.4637 .7648 .9409 8.0658 .1627 8.2419 .3089 .3669 .4181 .4638 .5053 8.5451 .5779 .6101 .6401 .6682 .6945 8.7194	301.1 176.1 124.9 96.9 79.2 67.0 58.0 51.2 45.7 41.5 37.8 34.8 32.2 30.0 28.1 26.3 24.9 23.5	2.5363 .2352 .0591 1.9342 .8373 1.7581 6911 .5332 .4947 1.4569 .4221 .3899 .3599 .3318 .3055 1.2806	89° 50′ 89° 40′ 89° 30′ 89° 20′ 89° 10′ 88° 50′ 88° 50′ 88° 30′ 88° 10′ 88° 10′ 88° 30′ 88° 10′ 87° 50′ 87° 50′ 87° 10′ 87° 10′ 87° 10′ 87° 10′
3° 10′ 3° 20′ 3° 30′ 3° 40′ 3° 50′ 4° 10′ 4° 20′ 4° 30′ 4° 50′ 5° 0′ 5° 10′ 5° 20′ 5° 30′ 5° 40′ 6° 10′ 6° 10′ 6° 10′ 6° 10′ 6° 10′ 6° 10′ 6° 30′ 7° 10′ 7° 10′ 7° 20′ 7° 30′	7.423 .7645 .7857 .8059 .8251 .8.8436 .8613 .8783 .8946 .9104 .9256 .9816 .9045 .9070 .90192 .0311 .0426 .0539 .0648 .0755 .0859 .0965 .09	22.2 21.2 20.2 19.2 19.2 17.7 17.0 16.3 15.8 15.2 14.7 13.4 12.9 12.5 11.3 10.9 10.7 10.7 10.4 9.9 9.7	.9993 .9993 .9991 .9990 9.9989 .9989 .9987 .9985 9.9983 .9982 .9981 .9980 .9977 9.976 .9973 .9976 .9978 .9978 .9978 .9978	.0 .1 .1 .1 .1 .0 .1 .1 .1 .1 .1 .1 .1 .1 .1 .1 .1 .1 .1	7429 7652 7885 8067 8261 8.8446 8624 8795 8960 9118 9272 8.9420 9563 9701 9836 9.0093 9.0216 0.345 0.0567 0.678 0.786 9.0891 0.995 1.0965 1.194	22.3 21.3 20.2 19.4 18.5 17.8 15.8 15.4 14.8 13.5 13.0 12.7 12.3 12.0 11.7 11.4 11.1 10.8 10.1 9.8	. 2571 .2348 .2135 .1933 .1739 .11554 .1376 .1205 .0728 .0728 .0728 .0437 .0299 .0164 .0907 .0907 .09784 .9664 .9547 .9433 .9322 .9214 .9109 .9005 .8904 .8806	86° 50′ 86° 40′ 86° 30′ 86° 20′ 86° 10′ 86° 0′ 85° 50′ 85° 30′ 85° 10′ 85° 10′ 84° 10′ 84° 10′ 84° 10′ 84° 10′ 84° 10′ 83° 30′ 83° 10′ 83° 30′ 83° 10′ 83° 30′ 83° 10′ 83° 30′ 83° 10′ 83° 30′ 83° 10′ 83° 30′ 80° 30′ 80° 30′ 80° 30° 30′ 80° 30° 30° 30′ 80° 30° 30° 30° 30° 30° 30° 30° 30° 30° 3
	Cos.	D. 1'	Sin.	D. 1'	Cot.	D.1'	Tan.	Angle

TANGENTS, AND COTANGENTS

LOGARITHMIC SINES, COSINES,

Angle	Sin.	D. 1'	Cos.	D. 1′	Tan.	D. 1'	Cot.	
15° 0′ 15° 10′ 15° 20′ 15° 30′ 15° 40′ 15° 50′ 16° 0′ 16° 10′ 16° 30′ 16° 50′ 17° 0′ 17° 10′ 17° 30′ 17° 50′ 18° 0′ 18° 10′ 18° 10′ 18° 30′ 18° 40′ 18° 50′ 18° 10′ 18° 30′ 18° 40′ 18° 50′ 18° 10′ 18° 30′ 18° 40′ 18° 50′ 19° 10′ 19° 20′ 19° 30′ 19° 50′ 20° 0′ 20° 10′ 20° 30′ 21° 10′ 21° 20′ 21° 10′ 21° 30′ 21° 10′ 22° 10′	9.4130 4177 4223 4269 4314 4359 9.4403 4447 4491 4533 4576 4618 9.4659 4700 4741 4821 9.4900 9.5126 5163 5109 9.5126 5163 5199 5235 5270 5306 9.5341 5375 5409 9.5543 55764 9.5543 55704 9.5767	4.7 4.6 4.6 4.5 4.4 4.4 4.2 4.1 4.0 4.0 3.9 3.8 3.7 3.8 3.7 3.6 3.5 3.6 3.7 3.6 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3 3.3	9.9849 9.9846 9.9848 9.9839 9.9836 9.9825 9.9825 9.9821 9.9817 9.9814 9.9810 9.9806 9.9786 9.9788 9.9788 9.9784 9.9790 9.9786 9.9785 9.9781 9.9787 9.9787 9.9789 9.9687	D. 1 3 3 4 4 3 4 4 3 4 4 4 4 4 4 4 4 4 4 4	9.4281 4381 4480 4479 4527 9.4575 4622 4669 4716 4762 4808 9.4853 4898 4943 5075 9.5118 5161 5203 5245 5287 9.5370 5411 5531 5551 9.511 5581 5587 9.589 5776 6.5804 9.5842 5879 9.5911 6028 9.6064	5.0 5.0 4.9 4.8 4.7 4.7 4.6 4.5 4.4 4.4 4.3 4.2 4.2 4.2 4.2 4.1 4.0 4.0 4.0 4.0 3.9 3.8 3.7 3.7 3.7 3.7 3.7 3.7 3.7 3.7	0.5719 -5669 -5670 -5527 -5528 -5378 -5331 -5284 -5288 -5331 -5284 -5288 -5192 -5057 -5013 -4969 -4925 -4713 -4671 -61630 -4589 -4549 -4469 -4429 -4389 -4350 -4311 -4273 -4273 -4234 -4196 -4121 -4083 -4099 -3972 -0.3936	75° 0′ 74° 40′ 74° 40′ 74° 20′ 74° 10′ 74° 0′ 73° 50′ 73° 40′ 73° 30′ 73° 20′ 73° 30′ 72° 50′ 72° 30′ 72° 30′ 72° 10′ 72° 10′ 72° 10′ 71° 10′ 71° 0′ 71° 10′ 71° 10′ 70° 50′ 71° 10′ 70° 10′ 7
22° 20′ 22° 30′	.5798 .5828	3.1 3.0 D. 1 '	.9661 .9656	.6 .5 D. 1 '	.6136 .6172	3.6 3.6 D , 1'	.3864 .3828 Tan.	67° 40′ 67° 30′ Angle

TANGENTS, AND COTANGENTS

Angle	Sin.	D. 1'	Cos.	D. 1'	Tan.	D. 1'	Cot.	
22° 30′ 22° 40′ 22° 50′ 22° 50′ 23° 20′ 23° 30′ 23° 40′ 23° 20′ 23° 50′ 24° 20′ 24° 20′ 24° 20′ 24° 50′ 26° 0′ 26° 10′ 26° 20′ 26° 40′ 26° 20′ 26° 50′ 27° 10′ 27° 20′ 20° 20° 20° 20° 20° 20° 20° 20° 20° 20°	9.5828 5859 5889 9.5919 .5948 .5978 .6006 9.6093 .6121 .6149 .6177 .6205 .6232 9.6259 .6286 .6310 .6392 9.6418 .6444 .6470 .6495 .6526 .6516 9.6570 .6550 .6550	3.1 3.0 3.0 2.9 3.0 2.9 2.8 2.8 2.8 2.8 2.7 2.7 2.7 2.7 2.6 2.6 2.6 2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5 2.5	9.9656 9.9651 9.9640 9.9640 9.9635 9.9629 9.9624 9.9618 9.9613 9.9607 9.9602 9.9584 9.9573 9.9567 9.9551 9.9549 9.9530 9.9530 9.9548 9.9530 9.9549 9.9530 9.9549 9.9530 9.9549 9.9549 9.9549 9.9549 9.9549 9.9549	.55.6 .5 .6.5.6 .5 .6.6 .6.6 .6.6 .7 .6 .6.6 .7 .6 .7 .6 .7 .6 .6 .7 .7 .6 .7 .7 .6 .7 .7 .6 .7 .7 .6 .7 .7 .6 .7 .7 .6 .7 .7 .6 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7	9.6172 .6208 .6208 .6249 .6314 .6348 .6383 .6417 .6452 .6520 .6587 .6620 .6587 .6620 .6752 .6785 .6850 9.6887 .6850 9.6887 .7909 .7040 .70	3.6 3.5 3.6 3.5 3.4 3.4 3.3 3.4 3.3 3.3 3.3 3.3 3.3 3.3	0.3828 .3792 .3757 .3686 .3662 .3617 .3548 0.3514 .3480 .3443 .3380 0.3313 .3280 0.3313 .3280 0.3118 .3086 .3044 .3023 .2990 0.2928 .2897 .2866	67° 30′ 67° 20′ 67° 10′ 67° 0′ 66° 50′ 66° 20′ 66° 0′ 65° 50′ 65° 20′ 65° 20′ 64° 50′ 64° 40′ 64° 20′ 64° 20′ 64° 20′ 64° 30′ 64° 20′ 64° 30′ 64° 20′ 63° 50′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 63° 20′ 62° 50′ 60′ 20′ 50′ 50′ 50′ 50′ 50′ 50′ 50′ 50′ 50′ 5
27° 30′ 27° 40′ 27° 50′ 28° 0′ 28° 10′ 28° 30′ 28° 40′ 28° 50′ 29° 10′ 29° 20′ 29° 30′ 29° 30′ 29° 50′	.6644 .6668 .6692 9.6716 .6740 .6763 .6787 .6810 .6833 9.6856 .6978 .6901 .6923 .6946 .6968	2.4 2.4 2.4 2.4 2.3 2.3 2.3 2.3 2.3 2.2 2.3 2.2 2.2 2.3 2.2 2.2	.9479 .9473 .9466 9.9459 .9453 .9446 .9432 .9425 9.9418 .9411 .9404 .9397 .9390 .9383	.7 .6 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7 .7	7165 7196 7226 9.7257 7287 .7317 .7348 .7348 .7408 9.7438 .7467 .7497 .7556 .7556 .7585	3.1 3.0 3.1 3.0 3.0 3.0 3.0 3.0 3.0 2.9 3.0 2.9 3.0 2.9	.2835 .2804 .2774 0.2743 .2713 .2683 .2652 .2592 0.2562 .2592 0.2562 .2533 .2503 .2474 .2444 .2415	62° 30′ 62° 20′ 62° 10′ 62° 0′ 61° 50′ 61° 30′ 61° 20′ 61° 10′ 60° 50′ 60° 40′ 60° 30′ 60° 20′ 60° 10′
30° 0′	9.6990 Cos.	D. 1'	9.9375 Sin.	D. 1'	9.7614 Cot.	D. 1'	0.2386 Tan.	Angle

LOGARITHMIC SINES, COSINES,

Angle	Sin.	D. 1'	Cos.	D. 1'	Tan.	D. 1'	Cot.	
30° 0 0 30° 10 30° 20 30° 30° 30° 30° 31° 10 31° 30° 31° 31° 40 31° 32° 20 32° 32° 32° 32° 32° 32° 33° 33° 30° 33° 33	9.6990 7012 7033 7055 7076 7076 7077 9.7118 7139 7160 7181 7201 7282 7382 7382 7382 7382 7382 7382 7382	2.2 2.1 2.2 2.1 2.1 2.1 2.1 2.0 2.0 2.0 2.0 2.0 2.0 1.9 1.9 1.9 1.8 1.8 1.8 1.8 1.8 1.7 1.7 1.7 1.7 1.7 1.7 1.7	9.9375 .9368 .9361 .9363 .9346 .9338 .9323 .9315 .9328 .9308 .9300 .9292 .9292 .9294 .9276 .9268 .9269 .9252 .9211 .9211 .9203 .9194 .9116 .9161 .9142 .9186 .9170 .9088 .9089 .9080 .9080 .9070 .9061 .9062 .9033 .9033 .9033 .9014 .9003	.7 .7 .8 .7 .8 .8 .8 .8 .8 .8 .8 .8 .8 .8 .8 .8 .8	9.7614 7644 7673 7701 77730 7759 9.7788 7816 7816 7846 7816 7847 7902 7930 9.7958 7986 8014 8042 8070 9.8125 8153 8180 8208 8208 8208 8208 8235 8263 9.8290 8317 8344 8371 8398 8425 9.8459 8506 8533 8559 8566 9.8138 8666 9.8613 8639 8666 9.8613	3.0 2.9 2.8 2.9 2.9 2.8 2.9 2.8 2.8 2.8 2.8 2.7 2.8 2.8 2.7 2.8 2.8 2.7 2.8 2.8 2.7 2.8 2.8 2.7 2.8 2.8 2.7 2.8 2.7 2.8 2.7 2.7 2.7 2.7 2.7 2.7 2.7 2.7 2.7 2.7	0.2386 2356 2327 2327 2327 2227 22184 2155 2127 2098 2070 0.2042 2014 1.986 1.958 1.930 0.1875 1.847 0.1710 1.683 1.666 1.629 1.602 1.575 0.1548 1.521 1.494 1.414 1.414 0.1387 1.334 1.314 1.1384 1.1384 1.1384 1.1385 1.282 1.292 1.203 1.176	60° 0′ 59° 50′ 59° 40′ 59° 20′ 59° 20′ 59° 10′ 58° 50′ 58° 40′ 58° 0′ 58° 0′ 57° 50′ 57° 20′ 57° 20′ 56° 30′ 56° 10′ 5
	Cos.	D. 1'	Sin.	D. 1'	Cot.	D. 1'	Tan.	Angle

TANGENTS, AND COTANGENTS

		•	80	58	57	55	54	52	51	20	48	47	45	44	45	41	40	38	32	300	33.3	322	30
	0	L Tan	8.94 19	8.94 34	94	94	95	95	95	8.95 63	8.95 77	96	96	96	96	96	8.97 01	97	97	97	76	8.98 09	8.98 36
S	20	L Sin	94 03	94 17	94 46	94 60	94 89	95 03	95 31	.95 45	95 59	95 87	96 00	96 28	96 42	69 96	.96 82	96 96	97 23	97 50	97 63	98 03	98 16
SMALL ANGLES	0	L Tan	8.84 46 8	8.84 8.84 8.84 8.85 8.80 8.80 8.80 8.80 8.80 8.80 8.80	85 01	25.00	85 54	85 72	20 98	8.86 24 8	8.86 42 8	86 76	86 93	87 28	87 45	87 78	.87 95 8	.88 12 88 29	88 45	88 78	88 95	89 27 89 44 88 88	8 09 68
MALL	4°	L Sin	84 36	84 54	84 90	85 07	85 43	85 60	85 95	8.86 13	8.86 30 8 86 47	86 64	86 82	87 16	87 32	99 28	87 83 8	99	33	65	200	8.89 14 8.89 30 8.89 30	46
OF	0	L Tan	8.71 94 8	8.72 18 8 8 72 42 8	72 66	72 90	73 37	73 60	74 06	8.74 29 8	8.74 52 8	74 97	75 20	75 64	75 87	76 31	8.76 52 8	76 74	77 17	77 60	77 81	8.78 23 8.78 23 8.78 23 8.78 23	78 65
TANGENTS	အ	L Sin	8.71 88	8.72 12 8	72 60	72 83	73 30	73 53	74 00	8.74 23	8.74 45	74 91	75 13	75 57	75 79	76 23	8.76 45	76 67	77 10	77 52	77 73	28.78.15	78 57
AND TA	0	L Tan	8.54 31	8.54 67 8	55 38	55 73	56 43	56 77	57 45	8.57 79	8.58 12	58 78	59 10	59 75	70 09	02 09	8.61 01 8	61 32	61 93	62 23	62 83	8.63 43	64 01
SINES A	çı	L Sin	8.54 28	8.54 64	55	55	56	56	57	8.57 76	8.58 09	58	59	59	09	000	8.60 97	61	61	622	62	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	63
	0	L Tan	8.24 19	8.24 91	26	27	28	28	30	8.30 89	8.31 50	32	233	34	35	36	8.36 69	37	300	30	39	8.40 83	41
OGARITHMIC	1	L Sin	8.24 19	8.24 90	26	26	28	28	30	8.30 88	8.31 49	32	233	34	35	36	8.36 68	37	300	39	39	8.40 82	41
ro	00	L Tan	:::	6.46 37	94	90	24	30		7.46 37	7.50 51						7.76 48	-	8.5	86	.87	7.91 09	
	0	L Sin	:::	6.46 37	100	90			-	7.46 37	7.50 51				-		7.76 48	80 80	85	98	87	7.92 61	
			0	-0	m.	4.10	9	r-0	00	10	11	13	45	16	17	19	20	21 22	23	25	26	28	30

22222222	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		
00000000000000000000000000000000000000	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	L Cot	4.
80000000000000000000000000000000000000	8	T Cos	84
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88.64430 88.64453 88.654433 88.65715 88.66599 8665743	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	C	870
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8.8.8.4.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	CC	880
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88.00220 00.00000 00.0000000000000000000	8.8.06.58 8.8.04 8.8.04 8.8.04 8.8.11		OC
000000000000000000000000000000000000000	44444444444444444444444444444444444444		

NATURAL SINES AND COSINES

A.	Sin.	Cos.		A.	Sin.	Cos.		A.	Sin.	Cos.	
0°	.000000	1.0000	90°	30'	.1305	.9914	30'	15°	.2588	.9659	75°
10'	.002909		50'	40'	.1334	.9911	20'	10'	.2616	.9652	50'
20'	.005818		40'	50'	.1363	.9907	10'	20'	.2644	.9644	40'
30'	.008727	1.0000	30'	8°	.1392	.9903	82°	30'	.2672	.9636	30'
40′ 50′	.011035	.9999	10'	10'	.1421	.9899	50'	50'	.2728	.9628	10'
10	.017452	.9998	89°	20' 30'	.1449	.9894	30'	16°	.2756	.9613	74°
10'	.02036	.9998	50'	40'	.1507	.9886	20'	10'	.2784	.9605	50'
20'	.02030	.9998	40'	50'	.1536	.9881	10'	20'	.2812	.9596	40'
30'	.02618	.9997	30'	90	.1564	.9877	81°	30'	.2840	.9588	30'
40'	.02908	.9996	20'	10'	.1593	.9872	50'	40'	.2868	.9580	20'
50'	.03199	.9995	10'	20'	.1622	.9868	40'	50'	.2896	.9572	10'
2°	.03490	.9994	880	30'	.1650	.9863	30'	17°	.2924	.9563	73°
10'	.03781	.9993	50'	40' 50'	.1679	.9858	20'	10'	.2952	.9555	50'
20'	.04071	.9992	40'		-	.9853		20'	.2979	.9546	40'
30' 40'	.04362	.9990	30' 20'	10°	.1736	.9848	80°	30'	.3007	.9537 .9528	30'
50'	.04943	.9988	10'	10' 20'	.1765	.9843	50'	50'	.3062	.9520	10'
3°	.05234	.9986	87°	30'	.1794	.9838	30'	18°	.3090	.9511	72°
10'	.05524	.9985	50'	40'	.1851	.9827	20'	10'	.3118	.9502	50'
20'	.05814	.9983	40'	50'	.1880	.9822	10'	20'	.3145	.9492	40'
30'	.06105	.9981	30'	11°	.1908	.9816	79°	30'	.3173	.9483	30'
40'	.06395	.9980	20'	10'	.1937	.9811	50'	40'	.3201	.9474	20'
50'	.06685	.9978	10'	20'	.1965	.9805	40'	50'	.3228	.9465	10'
40	.06976	.9976	860	30'	.1994	.9799	30'	19°	.3256	.9455	71°
10'	.07266	.9974	50'	40' 50'	.2022	.9793	20'	10'	.3283	.9446	50'
20'	.07556	.9971	40'	-		-		20'	.3311	.9436	40' 30'
30' 40'	.07846	.9969	30' 20'	12°	.2079	.9781	78°	30'	.3338	.9426	20'
50'	.08426	.9964	10'	10' 20'	.2108	.9775	50'	50'	.3393	.9407	10'
50	.08716	.9962	85°	30'	.2164	.9763	30'	20°	.3420	.9397	70°
10'	.09005	.9959	50'	40'	.2193	.9757	20'	10'	3448	.9387	50'
20'	.09295	.9957	40'	50'	.2221	.9750	10'	20'	.3475	.9377	40'
30'	.09585	.9954	30'	13°	.2250	.9744	77°	30'	.3502	.9367	30'
40'	.09874	.9951	20'	10'	.2278	.9737	50'	40'	.3529	.9356	20'
50'	.10164	.9948	10'	20'	.2306	.9730	40'	50'	.3557	.9346	10'
6°	.10453	.9945	84°	30'	.2334	.9724	30'	21°	.3584	.9336	69°
10'	.10742	.9942	50'	40' 50'	.2363	.9717	20' 10'	10'	.3611	.9325	50'
20' 30'	.11031	.9939	40' 30'	14°	.2419	.9703	76°	20' 30'	.3638	.9315	40' 30'
40'	.11320	.9936	20'					40'	.3692	.9293	20'
50'	.11898	.9929	10'	10' 20'	.2447	.9696	50'	50'	.3719	.9283	10'
70	.12187	.9925	83°	30'	.2504	.9681	30'	22°	.3746	.9272	68°
10'	.12476	.9922	50'	40'	.2532	.9674	20'	10'	3773	.9261	50'
20'	.12764	.9918	40'	50'	.2560	.9667	10'	20'	.3800	.9250	40'
30'	.13053	.9914	30'	15°	.2588	.9659	75°	30'	.3827	.9239	30'
	Cos.	Sin.	A.		Cos.	Sin.	A.		Cos.	Sin.	A.

NATURAL SINES AND COSINES - continued

A.	Sin.	Cos.		A.	Sin.	Cos.		A.	Sin.	Cos.	
30'	.3827	.9239	30'	30°	.5000	.8660	60°	30'	.6088	.7934	30'
40'	.3854	.9228	20'	10'	.5025	.8646	50'	40'	.6111	.7916	20'
50'	.3881	.9216	10'	20'	.5050	.8631	40'	50'	.6134	.7898	10'
23°	.3907	.9205	67°	30' 40'	.5075	.8616	30' 20'	38°	.6157	.7880	52°
10'	.3934	.9194	50' 40'	50'	.5125	.8587	10'	10' 20'	.6180	.7862	50'
30'	.3987	.9171	30'	31°	.5150	.8572	59°	30'	.6225	.7826	30'
40'	.4014	.9159	20'	10'	.5175	.8557	50'	40'	.6248	.7808 .7790	20'
50'	.4041	.9147	10'	20'	.5200	.8542	40'	50'	.6271		10'
24°	.4067	.9135	66°	30'	.5225	.8526	30'	39°	.6293	.7771	51°
10'	.4094	.9124	50'	40′ 50′	.5250	.8511	20'	10'	.6316	.7753	50'
20'	.4120	.9112	40' 30'	32°	.5299	-		20'	.6338	.7735 .7716	40' 30'
30' 40'	.4147	.9100	20'		-	.8480	58°	30'	.6361	7698	20'
50'	.4200	.9075	10'	10' 20'	.5324	.8465	50'	50'	.6406	.7679	10'
25°	.4226	.9063	65°	30'	.5373	.8434	30'	40°	.6428	.7660	50°
10'	.4253	.9051	50'	40'	.5398	.8418	20'	10'	.6450	.7642	50'
20'	.4279	.9038	40'	50'	.5422	.8403	10'	20'	.6472	.7623	40'
30'	.4305	.9026	30'	33°	.5446	.8387	57°	30'	.6494	.7604	30'
40'	.4331	.9013	20'	10'	.5471	.8371	50'	40'	.6517	.7585	20'
50'	.4358	.9001		20'	.5495	.8355	40'	50'		.7566	49°
26°	.4384	.8988	64°	30' 40'	.5519	.8339	30'	41°	.6561	.7547	
10' 20'	.4410	.8975	50' 40'	50'	.5568	.8307	10'	10'	.6583	.7528 .7509	50'
30'	.4462	.8949	30'	34°	.5592	.8290	56°	30'	.6626	.7490	30'
40'	.4488	.8936	20'	10'	.5616	.8274	50'	40'	.6648	.7470	20'
50'	.4514	.8923	10'	20'	.5640	.8258	40'	50'	.6670	.7451	10'
27°	.4540	.8910	63°	30'	.5664	.8241	30'	42°	.6691	.7431	48°
10'	.4566	.8897	50'	40'	.5688	.8225	20'	10'	.6713	.7412	50'
20'	.4592	.8884	40'	50'	.5712	.8208	10'	20'	.6734	.7392	40'
30'	.4617	.8870	30'	35°	.5736	.8192	55°	30'	.6756	.7373	30'
40' 50'	.4669	.8857	10'	10'	.5760 .5783	.8175	50'	50'	.6799	.7333	10'
28°	.4695	.8829	62°	20' 30'	.5807	.8158	40' 30'	43°	.6820	.7314	470
10'	.4720	.8816	50'	40'	.5831	.8124	20'	10'	.6841	.7294	50'
20'	.4746	.8802	40'	50'	.5854	.8107	10'	20'	.6862	.7274	40'
30'	.4772	.8788	30'	36°	.5878	.8090	54°	30'	.6884	.7254	30'
40'	.4797	.8774	20'	10'	.5901	.8073	50'	40'	.6905	.7234	20'
50'	.4823	.8760	10'	20'	.5925	.8056	40'	50'	.6926	.7214	10'
29°	.4848	.8746	61°	30'	.5948	.8039	30'	44°	.6947	.7193	46°
10'	.4874	.8732	50'	40' 50'	.5972	.8021	20'	10'	.6967	.7173	50'
20' 30'	.4899	.8718	40' 30'	37°	.6018	.7986	53°	20' 30'	.6988	.7153 .7133	40' 30'
40'	.4924	.8689	20'		.6041		50'	40'	.7030	.7112	20'
50'	.4975	.8675	10'	10'	.6065	.7969	40'	50'	.7050	.7092	10'
30°	.5000	.8660	60°	30'	.6088	.7934	30'	45°	.7071	.7071	45°
	Cos.	Sin.	A.		Cos.	Sin.	A.		Cos.	Sin.	A.

NATURAL TANGENTS AND COTANGENTS

A.	Tan.	Cot.		A.	Tan.	Cot.		A.	Tan.	Cot.	
0°	.000000	00	90°	30'	.1317	7.5958	30'	15°	.2679	3.7321	75°
10'	.002909	343.7737	50'	40'	.1346	7.4287	20'	10'	.2711	3.6891	50'
20'	.005818		40'	50'	.1376	7.2687	10'	20'	.2742	3.6470	40'
30'	.008727	114.5887	30'	8°	.1405	7.1154	82°	30'	.2773		30'
40'	.011636	85.9398	20'	10'		6.9682	50'	40'		3.5656	20'
50'	.014545	68.7501	10' 89°	20'		6.8269	40'	50'		3.5261	74°
1°	.017455	57.2900		30′ 40′		6.6912 6.5606	30'	16°	.2867	3.4874	
10'	.02036	49.1039 42.9641	50' 40'	50'		6.4348	10'	10'	.2899	3.4495 3.4124	50'
20' 30'	.02528	38.1885	30'	9°	.1584	6.3138	81°	30'	.2962		30'
40'	.02910	34.3678	20'	10'	.1614	$\frac{6.0130}{6.1970}$	50'	40'		3.3402	20'
50'	.03201	31.2416	10'	20'	.1644		40'	50'	.3026	3.3052	10'
20	.03492	28.6363	88°	30'	.1673	5.9758	30'	17°	.3057	3.2709	73°
10'	.03783	26.4316	50'	40'	.1703	5.8708	20'	10'	.3089		50'
20'	.04075	24.5418	40'	50'	.1733	5.7694	10'	20'	.3121	3.2041	40'
30'	.04366	22.9038	30'	10°	.1763	5.6713	80°	30'	.3153	3.1716	30'
40'	.04658	21.4704	20'	10'	.1793	5.5764	50'	40'	.3185		20'
50'	.04949	20.2056	10'	20'	.1823	5.4845	40'	50'	.3217	3.1084	10'
3°	.05241	19.0811	87°	30'	.1853	5.3955	30'	18°	-	3.0777	72°
10'	.05533	18.0750	50'	40' 50'	.1883	5.3093 5.2257	20'	10'	.3281	3.0475	50'
20'	.05824	17.1693	40' 30'	11°	1944	-	79°	20' 30'	.3314	3.0178 2.9887	40' 30'
30' 40'	.06116	16.3499 15.6048	20'			5.1446		40'	3346	2.9600	20'
50'	.06700	14.9244	10'	10' 20'	.1974	5.0658 4.9894	50'	50'	.3411		10'
40	.06993	14.3007	86°	30'		4.9094 4.9152	30'	19°	.3443	_	71°
10'	.07285	13.7267	50'	40'	.2065	4.8430	20'	10'	.3476	2.8770	50'
20'	.07578	13.1969	40'	50'	.2095	4.7729	10'	20'	.3508		40'
30'	.07870	12.7062	30'	12°	.2126	4.7046	78°	30'	.3541	2.8239	30'
40'	.08163	12.2505	20'	10'	.2156	4.6382	50'	40'	.3574	2.7980	20'
50'	.08456	11.8262	10'	20'	.2186	4.5736	40'	50'	.3607	2.7725	10'
5°	.08749	11.4301	85°	30'	.2217	4.5107	30'	20°	.3640	2.7475	70°
10'	.09042	11.0594	50'	40' 50'	.2247	4.4494 4.3897	20'	10'	.3673	2.7228	50'
20'	.09335	10.7119	40'	13°	-	-	77°	20'	.3706	2.6985	40'
30'	.09629	10.38 5 4 10.0780	30′		.2309	4.3315		30'	.3739	2.6746 2.6511	30'
50'	.10216	9.7882	10'	10' 20'	.2339	4.2747 4.2193	50'	50'	.3805		10'
60	.10510	9.5144	84°	30'	.2401	4.2193	30'	21°		2.6051	69°
10'	.10805	9.2553	50'	40'	.2432	4.1126	20'	10'	.3872	2.5826	50'
	.11099	9.0098	40'	50'		4.0611	10'	20'	.3906	2.5605	40'
30'	.11394	8.7769	30'	14°	.2493	4.0108	76°	30'	.3939	2 5386	30'
	.11688	8.5555	20'	10'		3.9617	50'	40'	.3973	2.5172	20'
50'	.11983	8.3450	10'	20'		3.9136	.40'	50'	_	2.4960	10'
7°	.12278	8.1443	83°	30'		3.8667	30'	22°	.4040	2.4751	68°
10'	.12574	7.9530	50'	40'		3.8208	20'	10'		2.4545	50'
	.12869	7.7704	40'	50'	_	4.7760	10'	20'		2.4342	40'
30'	.13165	7.5958	30'	15°	.2679	3.7321	75°	30'	.4142	2.4142	30'
	Cot.	Tan.	A.		Cot.	Tan.	A.		Cot.	Tan.	A .

NATURAL TANGENTS AND COTANGENTS

A.	Tan.	Cot.		A.	Tan.	Cot.		A.	Tan.	Cot.	
30'	.4142	2.4142	30'	30°	.5774	1.7321	60°	30'	.7673	1.3032	30'
40' 50'	.4176	2.3945 2.3750	20'	10'	.5812	1.7205	50'	40' 50'	.7720	1.2954	20'
23°	.4245	2.3559	67°	20' 30'	.5851	1.7090 1.6977	40' 30'	38°	.7813	$\frac{1.2876}{1.2799}$	10' 52°
10'	.4279	2.3369	50'	40'	.5930	1.6864	20'	10'	.7860	1.2723	50'
20'	.4314	2.3183	40'	50'	.5969	1.6753	10'	20'	.7907	1.2647	40'
30'	.4348	2.2998 2.2817	30' 20'	31°	.6009	1.6643	59°	30'	.7954	1.2572	30'
50'	.4383	2.2637	10'	10'	.6048	1.6534 1.6426	50'	40' 50'	.8002	1.2497 1.2423	20'
24°	.4452	2.2460	66°	30'	.6129	1.6319	30'	39°	.8098	1.2349	51°
10'	.4487	2.2286	50'	40'	.6168	1.6212	20'	10'	.8146	1.2276	50'
20'	.4522	2.2113	40'	50'	.6208	1.6107	10'	20'	.8195	1.2203	40'
30'	.4557	2.1943 2.1775	30'	32°	.6249	1.6003	58°	30'	.8243	1.2131	30'
50'	.4628	2.1609	10'	10'	.6289	1.5900	50'	50'	.8342	1.1988	10'
25°	.4663	2.1445	65°	30'	.6371	1.5697	30'	40°	.8391	1.1918	50°
10'	.4699	2.1283	50'	40'	.6412	1.5597	20'	10'	.8441	1.1847	50'
20'	.4734	2.1123 2.0965	40' 30'	50′ 33°	.6453	1.5497	57°	20' 30'	.8491	1.1778	40'
40'	.4806	2.0903	20'	10'	.6536	1.5399	50'	40'	.8541	1.1708	30'
50'	.4841	2.0655	10'	20'	.6577	1.5204	40'	50'	.8642	1.1571	10'
26°	.4877	2.0503	64°	30'	.6619	1.5108	30'	41°	.8693	1.1504	49°
10'	.4913	2.0353	50'	40' 50'	.6661	1.5013	20'	10'	.8744	1.1436	50'
20' 30'	4950	2.0204 2.0057	40' 30'	340	.6745	1.4826	56°	20' 30'	.8796	1.1369	40' 30'
40'	.5022	1.9912	20'	10'	.6787	1.4733	50'	40'	.8899	1.1237	20'
50'	.5059	1.9768	10'	20'	.6830	1.4641	40'	50'	.8952	1.1171	10'
27°	.5095	1.9626	63°	30′	.6873	1.4550	30'	42°	.9004	1.1106	48°
10'	.5132	1.9486	50'	50'	.6916	1.4460 1.4370	20'	10'	.9057	1.1041	50'
20'	.5169	1.9347 1.9210	40' 30'	35°	.7002	1.4281	55°	30'	.9110	1.0977	40' 30'
40'	.5243	1.9074	20'	10'	.7046	1.4193	50'	40'	.9217	1.0850	20'
50'	.5280	1.8940	10'	20'	.7089	1.4106	40'	50'	.9271	1.0786	10'
28°	.5317	1.8807	62°	30'	.7133	1.4019	30'	43°	.9325	1.0724	47°
10'	.5354	1.8676 1.8546	50'	50'	.7221	1.3848	10'	10'	.9380	1.0661	50'
30'	.5430	1.8418	30'	36°	.7265	1.3764	54°	30'	.9490	1.0538	30'
40'	.5467	1.8291	20'	10'	.7310	1.3680	50'	40'	.9545	1.0477	20'
50'	.5505	1.8165	10'	20'	.7355	1.3597	40'	50'	.9601	1.0416	10'
29°	.5543	1.8040	61°	30'	.7400	1.3514	30'	440	.9657	1.0355	46°
10'	.5581	1.7917 1.7796	50'	50'	.7490	1.3351	10'	10'	.9713	1.0295 1.0235	50'
30'	.5658	1.7675	30'	37°	.7536	1.3270	53°	30'	.9827	1.0176	30'
40'	.5696	1.7556	20'	10'	.7581	1.3190	50'	40'	.9884	1.0117	20'
50′ 30°	.5735	1.7437	10' 60°	20'	.7627	1.3111	40' 30'	50' 45°	1.0000	1.0058	10'
30	.5//4	1.7321	00	30'	.7673	1.3032	30	40	1.0000	1.0000	40
	Cot.	Tan.	A.		Cot.	Tan.	A.		Cot.	Tan.	A.

A B C D E F G H I J K L M N O P Q R S T U V W X Y Z I 234567890 & I I I II I I V V I X Etc. ABCDEFGHIJKLMNOPGRSTUVWXYZ ABCDEFGHIJKLMNOPGRSTUVWXYZ Condensed Style, & 1234567890 Extended abcdefghijkImnopgrstuvwxyz Inclined Lettering, Used for Waters.

LETTERING SUGGESTED FOR USE ON A WOODSMAN'S MAP

Upright Lettering, for General Use.

SECTION II

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CONTENTS OF CYLINDERS IN CUBIC FEET

(Gives also area of any number of circles)

	25	6.00110.002 4.0
	24	2222 2222 2222 2222 2222 2222 2223 2223 2223 223 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 223 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 223 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 223 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 223 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 223 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 223 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 223 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 2233 223
	23	25.9 25.9 25.0 25.0 25.0 25.0 25.0 25.0 25.0 25.0
	22	2.6 2.6 2.6 2.6 2.6 2.6 2.6 2.6 2.6 2.6
	21	4847.00.211.00.024.88.1.00.04.89.1.00.04.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.89.1.00.04.1.00.00.04.1.00.04.1.00.04.1.00.04.1.00.04.1.00.04.1.00.04.1.00.04.1.0
	8	22.4 6.5.5 1.0.0 1.0
	19	25.00.00.00.00.00.00.00.00.00.00.00.00.00
	18	1.82 1.1. 1.1. 1.1. 1.1. 1.1. 1.1. 1.1.
	17	11.6 11.6
	16	482407482344624 48240748244624 482400482460 482460482466 4824664
ches	15	247.0.14.0.8.0.1.1.4.0.8.0.1.1.0.8.0.1.1.1.0.8.0.1.1.1.1.1.1
Diameter in Inche	14	11.00.00.00.00.00.00.00.00.00.00.00.00.0
neter	13	0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Dia	12	0.00
	11	900000000000000000000000000000000000000
	10 1	
		0.05 1.09 1.09 1.09 1.09 1.09 1.09 1.09 1.09
	6	4.88.00.00.00.00.00.00.00.00.00.00.00.00.
	00	0.35 0.70 0.70 0.70 0.70 0.70 0.70 0.70 0.7
	2	0.53 0.53 0.53 0.53 0.53 1.07 1.87 2.41 2.41 1.87 8.3 8.3 111 110 110 110 110
	9	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0
	10	00000000000000000000000000000000000000
	4	011288440017 01288440017 001288420017 0012888420017 0012884200000000000000000000000000000000
-	60	200703232884 20070323884 10070323884 112884799777
gth	Ineal I ni	1984001000000000000000000000000000000000

Example. A log 47 ft. long has a mid diameter of 17 inches. Contents of cylinder 7 ft. long is found in the table under 17 inches to be 11 eu. ft. Cylinder 40 ft. long of same diameter has 63 cu. ft. contents. 63 + 11 = 74 cu. ft. total contents of log.

CONTENTS OF CYLINDERS IN CUBIC FEET (continued) (Gives also area of any number of circles)

	48	1006 1006 1006 1006 1006 1006 1006 1006
	47	108448211088438450 10848211088438450 1084831108843845
	46	0.000 0.000
	45	11284430119887483211 10084430119887483211 100844301119887483211
	44	1128448821-88448821-1128448821-1128448821-1128488488488488488488848884888848
	43	0.0000000000000000000000000000000000000
	42	0.000 8 4 7 0 1 0 1 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0
	41	12844051.80481284414 987.04044129118044414 2887.0404412911807.8094814
	40	12884200078 - 198848085 - 19884200078 - 19884500 - 1988400 - 19884500 - 19884500 - 19884500 - 19884500 - 19884500 - 19884
	39	80021447000000000000000000000000000000000
	000	25.55.55.55.55.55.55.55.55.55.55.55.55.5
ches	37	24 24 25 25 25 25 25 25 25 25 25 25 25 25 25
Diameter in Inches	36	11.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1
meter	35	1.401.4-004-00
Dia	-	1 waavaan41-0
	34	0.0.0.0.0.0.0.0.0.4 0.0.0.0.0.0.0.0.0.0.
	33	028800011-80 0112228440011122864440
	32	22222222222222222222222222222222222222
	31	1150 1150 1150 1150 1150 1150 1150 1150
	30	401104228844 40110424844 4014042464687 6014044484 6014044484
	59	4.0 8.2 8.2 8.2 8.2 8.2 8.2 8.2 8.2 8.2 8.2
	28	4.825-1-0.0488.4891-1-0.0488.4891-1-0.0488.68-1-4-0.0488.68-1-0.0488.08-1-0.0488.68-1-0.0488.68-1-0.0488.68-1-0.0488.68-1-0.0488.68-1-0.0488.68-1-0.0488.68-1-0.0488.68-1-0.0488.68-1-0.0488.68-1-0.04
	27	47-1111 88 25 25 25 25 25 25 25 25 25 25 25 25 25
	26	8.5.114183888888888888888888888888888888888
	Ten F	1984400 1088400000000000000000000000000000

Example. In calipering over a piece of timber 23 trees are found whose diameter is 13 inches at breast high. What is the area of the cruses taken together, or, in other words, the total basal area?

In the column under 13 inches and opposite 20 in the column marked "Length in Feet" IS is found. In the same column opposite 3 feet is 2.77. 18 + 2.77 = 20.77, or with sufficient accuracy for this purpose 21. 21 square feet, therefore, is the answer desired.

AREA OF CIRCLES OR BASAL AREAS

(Gives also Contents of Cylinders one foot long)

Diameter Inches	Sq. ft. Diameter Inches	Area Sq. ft.	Diameter	Area Sq. ft.	Diameter	Area Sq. ft.	Diameter Inches	Area Sq. ft.
1.5 2.5 3.5 4.0 4.5 5.0 5.5 6.0 6.5 7.5 8.5 9.5 10.0 11.5	005 13.0 012 13.5 122 14.0 0334 14.5 049 15.0 067 15.5 087 16.0 111 16.5 17.5 186 17.5 196 18.0 307 19.5 349 20.0 394 20.5 394 20.5 442 21.0 21.5 545 22.0 301 22.5 360 23.5 360 23.5 3785 24.0 385 24.0	0.92 0.99 1.07 1.15 1.23 1.31 1.40 1.48 1.58 1.67 1.77 2.18 2.29 2.41 2.52 2.64 2.76 2.89 3.01 3.14 3.27	25.0 25.5 26.5 27.0 27.5 28.5 29.5 30.0 31.5 32.5 33.0 34.5 34.0 34.5 35.5 36.5	3.41 3.55 3.69 3.88 4.12 4.28 4.43 4.57 5.49 5.59 6.69 6.68 6.87 7.07	37.0 37.5 38.0 39.0 39.5 40.0 40.5 41.0 42.0 42.5 44.0 45.5 44.0 45.5 46.0 46.5 47.0 48.5	7.47 7.67 7.88 8.08 8.30 8.51 8.73 8.95 9.17 9.39 9.62 9.85 10.08 10.30 11.04 11.29 11.79 12.05 12.26 12.57	49.0 49.5 50.0 51.0 51.5 52.5 53.0 54.0 54.5 55.5 56.5 57.5 58.0 58.5 59.5 60.5	13.10 13.37 13.64 13.91 14.19 14.47 15.03 15.59 15.90 16.20 16.50 17.10 17.41 17.72 18.03 18.67 18.95 19.31 19.63

(see page 130) CORD A MAKE 128 DIMENSIONS. GIVEN OF LOGS FEET CUBIC STACKED

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6 8 0883747472883108444435551580 17 803222218 16 O 15 14 \$1-4-0 \\ \text{25}\) \text{25}\) \text{25}\) \text{26}\) \text{27 Middle-Inches 12 Diameter at 41-4 0010000 0 220000-4250-750000-2264420000 6 **80545801058** 00 41-4-4-0000000000000000 1 9 C4C-086088444444466 C4C-08608086680 401:24822323232 0001000 3 00000 Length Feet in

	25	29 28 39 39 39 39 39 39 39 39 39 39 39 39 39
	24	9: 81: 142 : 65: 65: 65: 65: 65: 65: 65: 65: 65: 6
	23	25 : : : : : : : : : : : : : : : : : : :
	22	25: 30: 36: 44: 45: 36: 37: 77: 77: 74: 45: 46: 46: 46: 46: 46: 46: 46: 46: 46: 46
	21	12 : : : : : : : : : : : : : : : : : : :
	20	: 25: : 25: : 36: : : 36: : : 25: : 25: :
	19	: :8 : : :8 : :8 : :8 : :4 : :8 : :8 :
	18	71
	17	4 4 4
er	16	11111111111111111111111111111111111111
Inches in Diameter	15	111111111111111111111111111111111111111
in Di	14	38888: 2882: 4882: 12098: 1202: 1109
ches	13	8 : 65: 12: 12: 12: 12: 12: 12: 12: 12: 12: 12
II	12	7 : 8 : 6 : 0 : 111 : 12
	11	.o. :r. :8: :0: :1: :2: :2: :1: :0: :0: :0: :0: :0: :0: :0: :0: :0
	10	19: 55: 14: : 35: 12: 12: 15: 6: 6: 7: 6: 65: 65: 7: 15: 15: 15: 15: 15: 15: 15: 15: 15: 15
	6	:4 :70: :20: :12: :20: :20: :20: :20: :20: :2
	00	w:::4:::rv:::6:::1.::00:::1.00:::00:::00:::00:::00
	-1	:::::\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
	9	::ex ::::::::::::::::::::::::::::::::::
	20	H:::::::::::::::::::::::::::::::::::::
	4	::::=::::::::::::::::::::::::::::::::::
	Feet	00000000000000000000000000000000000000

	1	1
	40	21222222222222222222222222222222222222
	39	820 820 820 820 820 820 820 820 820 820
	38	2820288825544455544555688885554555545555555555
	37	40000000000000000000000000000000000000
	36	2011 2011 2011 2011 2011 2011 2011 2011
ie.	35	72727272888138282111100000000000000000000000000
Diamete	34	488877288887728777777777777777777777777
Inches in Diameter	33	1.25048888999999999999999999999999999999999
Inc	32	828282828282828282828838883888888888888
	31	44449999999999999999999999999999999999
	30	294466666666666666666666666666666666666
	29	8444466666675777777777777777777777777777
	28	7.044444655555555555555555555555555555555
	27	47.6444477008688888888888888888888888888888
	26	32. 3.7. 4.5. 5.0. 5
ni t dty		284000-8900-89000-8900-8900-8900-8900-890

NEW YORK STANDARD, DIMICK, OR. GLENN'S FALLS RULE

нтн	DIAMETER IN INCHES												
LENGTH	3	4	5	6	7	В	9	10	11	12	13	14	15
ft.													
5	.009	.01	.02	.03	.04	.06	.07	.09	.10	.12	.14	.17	.19
6	.01	.02	.03	.05	.06	.08	.10	.13	.16	.18	.22	.25	.29
7	.02	.02	.04	.05	.08	.10	.12	.15	.18	.22	.25	.29	.33
В	.02	.02	.04	.06	.09	.11	.14	.17	.21	.25	.29	.33	.38
9	.02	.03	.05	.07	.10	.12	.15	.19	.24	.28	.33	.37	.43
10	.02	.03	.05	.08	.11	.14	.17	.22	.26	.31	.36	.42	.48
11	.03	.03	.06	.08	.12	.15	.19	.24	.29	.34	.40	.46	.52
12	.03	.04	.06	.09	.13	.17	.20	.26	.31	.37	.43	.50	.57
13	.03	.04	.07	.10	.14	.18	.22	.28	.34	.40	.47	.54	.62
	16	17	18	19	20	21	22	23	24	25	26	27	28
4	.22	.25	.28	.31	.34	.38	.41	.45	.49	.53	.58	.62	.67
5	.27	.31	.35	.38	.43		.52		.62	.67	.72	.78	.83
6	.33	.37	.42	.46	.51	.56	.62	.68	.74	.80	.86	.93	1.00
7	.38	.43	.48	.54	.60	.66	.72	.79	.86	.93	1.01	1.09	1.17
8	.44	.49	.55	.62	.68	:75	.82	.90	.98	1.06	1.15	1.24	1.34
9	.49	.55	.62	.69	.77	.84		1.02		1.20	1.29	1.40	1.50
10	.55	.62	.69	.77	.85		1.03			1.33	1.44	1.55	1.67
11	.60	.68	.76	.85		1.03				1.46	1.58	1.71	1.84
12	.66	.74	.83			1.13				1.60	1.73	1.86	2.00 2.17
13	.71	.80	.90	1.00	1.11	1.22	1.34	1.47	1.00	1.73	1.01	2.02	2.11
	29	30	31	32	33	34	35	36	37	38	39	40	
4	.72	.77	.82	.87	.93	00	1.04	1 10	1.17	1 92	1.30	1.36	
5	.72								1.46		1.62	1.70	
6	1.08			1.31						1.85	1.94	2.04	
7	1.25								2.04	2.15	2.27	2.39	
8	1.43			1.75						2.46	2.59	2.73	-
9	1.61								2.62	2.77	2.91	3.07	
10	1,79								2.92	3.08	3.24	3.41	
11	1.97	2.11		2.40							3.56	3.75	
12	2.15								3.50	_	3.89	4.09	
13	2.33	2.49	2.66	2.84	3.02	3.20	3.39	3.59	3.79	4.00	4.21	4,43	

SCRIBNER LOG RULE Legal Rule in Minnesota

	29	457 600 685 761 914		90	1296 1512 1728 1944 2160 2376 2592
	28	509 509 509 582 582 654 800 873		74	1242 1449 1862 2070 2276 2484
	27	411 479 548 616 684 753 821			-
	26	375 439 500 500 502 625 688 750		46	11388 1388 1587 1785 1983 2180 2380
	25	344 450 450 516 631 688		45	11329 1329 1518 1707 1898 2088 2278
	24	303 404 404 454 555 505 606		44	1110 1295 1480 1480 1665 850 850 850 8220
	23	283 3330 377 474 470 518 566		43	046 222 396 571 745 992
	22	251 292 292 292 334 418 60 601		4,	
	21	2228 2266 3304 342 380 418 418		42	1007 1175 1343 1511 1679 1848 2014
	50	2210 2245 2880 3315 3350 4420	. 1	4	954 11113 1272 1431 1590 1748 1908
	19	180 240 240 330 330 360		40	903 204 205 505 656 656 806
es	18	160 213 240 267 267 293 320	Diameter Inches	39	840 120 120 120 120 120 120 120 120 120 12
Diameter	17	139 1852 208 203 203 203 203 203 203 203 203 203 203			
	91	238 238 238 238 238 238 238		38	801 9344 1068 1201 1335 1468 1468
	15	107 125 142 160 160 178 214		37	772 901 1029 1158 1287 1416 1544
	14	86 100 1114 129 143 172		36	692 807 923 1038 1152 1268 1380
	13	73 85 97 109 1122 1134 1146		35	657 766 876 985 985 314
	12	259 669 79 888 1108 118		34	200000000000000000000000000000000000000
	11	50 255 20 20 20 20 20 20			
	10	440 555 70 70 70		33	588 686 686 784 784 980 980 1078 1176
	6	30 440 550 550		32	552 644 736 828 920 1012 1104
	00	48220444		31	532 622 710 799 888 976 976
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	9	22228422		30	493 657 657 739 821 904 986
ngth Feet	Lei	118 118 120 220 24 24	ngth Feet	a.I.	24588824

that from an extension of the latter made FOREST SERVICE to 70"; above alues in tens. Rule the Spauld Columbia OF the from DECIMAL OD up to 48"; Rule Scribner Derived from

	72	193 204 204 204 204 204 204 204 204 204 204	06	315 393 393 393 393 393 393 393 393 393 39
	71	2240 2880 2881 2881 2881 2881 2881 2881 288	- 68	3885 4682 4682 4683 6693 6693 677 770 670 671 672 872 873 874 874 874 874 874 874 874 874 874 874
	20	186 2325 2325 2325 2419 465 605 605 605 744	00	
	69	180 221 271 371 361 406 452 497 542 5542 632 677 723	88	301 377 452 527 527 603 678 753 829 904 979 979 1130
	89	175 202 202 3306 3306 4437 4437 6118 6018 609	87	295 3368 516 516 5589 663 737 737 737 737 737 737 737 737 737
	29	2012 2122 2544 2974 3399 4423 466 550 635 677		1
	99	164 2206 2206 2206 2200 2320 2320 2320 2320	86	2845 5705 646 646 1003 1003 1149
	65	1159 2239 2239 2239 2398 2398 2398 2398 239	53	281 491 561 631 772 842 912 912 982 1123
	64	1154 1193 2232 2232 2270 3848 3874 4655 563 561 619	25	27.5 27.5 27.5 27.5 27.5 27.5 27.5 27.5
	63	149 187 187 224 229 229 336 444 448 448 5523 560	-	
es	62	145 1181 1181 2253 2253 3325 3325 3325 542 570 570	000	868 868 868 868 868 868 869 1000 1000 1000 1000
Diameter Inches	19	140 175 175 175 175 175 175 175 175 175 175	82	261 3326 3326 3326 3326 717 717 717 717 717 717 717 717 717 71
	09	135 169 237 237 237 237 2406 439 4439 507 507	81	254 444 444 699 699 699 699 699 699 699 69
	29	131 163 196 196 229 229 4425 4457 4457 523	00	
	200	1126 1158 1158 1158 1158 1158 1158 1158 115	80	247 3719 3719 3719 3719 3719 4742 4742 866 866 987 987 989
	22	1122 1152 1152 1183 1274 1274 133 133 147 147 147 147 147 147 147 147 147 147	79	240 3011 3011 3011 3011 3011 442 472 472 472 472 472 472 472 902 902 903
	56	1118 1176 2206 2235 2235 3323 344 4411 44411 44411	78	234 234 234 234 252 252 264 268 268 268 268 268 268 268 268 268 268
	55	1113 1170 1170 1198 1198 12227 2227 2223 3340 4425 4425 4425	22	222 222 223 230 24 25 25 25 25 25 25 25 25 25 25 25 25 25
	54	109 1137 1164 1164 1164 1164 1164 1164 1164 116	92	221 2221 2221 2221 2221 2221 2221 2221
-	.53	105 1132 1132 1158 1158 1158 1158 1158 1158 1158 115	75	215 2215 2215 3223 3223 323 323 323 323 323 323 323
	52	101 1152 1152 1152 1152 1153 1153 1153 115	74	200 200 3161 3164 317 523 523 628 628 628 683 7783 837
	51	97 1122 1146 1170 1195 222 222 2268 3215 3341 3365	73	203 203 305 305 305 450 610 610 813 813
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	26	2022 2022 2022 2022 2022 2022 2022 202
	25	2222 22222 22222 22222 22222 22222 22222
	4.61	04040404040404040404040404040404040404
	60	880 881 882 883 883 883 883 883 883 883 883 883
	69	1162 2222 2222 22222 22222 22222 2222 2
	21	11633 11633
	20	104491 10
63	19	11127 11127 11127 11127 11127 11127 11127 1127
n Inch	18	8012101010101010101010101010101010101010
Diameter in Inches	17	100 80 80 80 80 80 80 80 80 80 80 80 80 8
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	10	000 000 000 000 000 000 000 000 000 00
	14	2525
	13	66 66 66 66 66 66 66 66 66 66 66 66 66
	12	2500444460000000000000000000000000000000
	11	488888844466668446666846666646666666666
	10	8888824744428832832848848848888888888888
		24-61-093331880888888844444444444444444444444444
	00	800000000000000000000000000000000000000

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ngth			
	44	800 1000 11100 1100 1100	390
	43	845 25 25 25 25 25 25 25 25 25 25 25 25 25	3707
	42	84288888888888888888888888888888888888	3520
	41	0.000 0.000	3337
	40	104 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	3159
	68	6612 7689 7689 9100 9100 9100 9100 9100 9100 9100 91	2986
	38	6578 6578	2818
Inches	37	6544 68134 68136 6	2654
n.	36	\$2000 \$2000	2496
Diameter	900	488 661 661 661 661 661 661 661 661 661 6	2342
A	34	6450 6560 6560 6600 6600 6600 6600 6600	2194
	800	44466644466644466644646464646464646464	2050
	85	398 444 444 444 444 444 444 444 4	1911
	31	### ### ##############################	1778
	30	88844466 888446666666666666666666666666	1648
	29	23152 2445 2455 2455 2455 2455 2455 2455 2	1523
	138	288 288 288 288 288 288 288 288 288 288	1404
	74	28 28 28 28 28 28 28 28 28 28 28 28 28 2	1289
ngth Feet	ni in	000-100-100-1000-1000-1000-10000-10000-10000-10000-1000-	300

MAINE, HOLLAND, OR BANGOR LOG RULE

	24	22 22 23 23 24 24 24 24 24 24 24 24 24 24 24 24 24
	23	250 2740 3300 3326 3326 3326 4401 4401 4401 4401 4401 4401 4401 440
	22	22222222222222222222222222222222222222
	21	22222220 22222222222222222222222222222
	20	1189 2222 2227 2227 2227 2227 2227 2227 22
	19	1189 1184 1184 1184 1184 1184 1184 1184
	18	1145 1165 11740 11899 1189 1189 11899 11899 11899 11899 11899 11899 11899 11899 11899 11899 1189 1189
	17	1128 11674 1
ches	16	1111 1233 1145 1157 1168 1179 1179 1179 1179 1179 1179 1179 117
r in In	15	101 1121 1131 1131 1131 1141 1151 1161 1161 1161 1161 1161 116
Diameter in Inches	14	89 989 1115 1115 1115 1115 1115 1115 111
D	13	2218 2218 2218 2218 2218 2218 2218
	12	65 70 70 70 70 70 70 70 70 70 70 70 70 70
	11	11229 11229 11229 11229 11229 11229 11229 11229 11229 11229
	10	244 46 66 66 66 66 66 66 66 66 66 66 66 6
	6	8888444477776404777777777777777777777777
	00	23.33.30.00.00.00.00.00.00.00.00.00.00.00
	1-	01034438666664444444666666
	9	822222222222222222222222222222222222222
gth	d ni	011111111111111111111111111111111111111

MAINE, HOLLAND, OR BANGOR LOG RULE - continued

-		F-20004-1004-00F-400F-00OF-
	42	877 1052 1052 1152 1153 1153 1153 1153 1153 1153 11
	41	834 1081 1082 11085 11188 1118
	40	2888 867 867 867 867 867 867 867 867 867
	39	756 831 907 907 907 907 907 907 907 907 907 907
	98	700 9022 9022 1006 1112 1112 1112 1112 1112 1112 1
	37	681 7491 7491 7491 7491 7492 7493 7493 7493 7493 7493 7493 7493 7493
	36	64 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
33	35	659 771122 771122 8837122 111326 111326 11146
Diameter in Inches	34	5662 6682 78775 7877
neter i	33	5530 6636 6636 6636 6636 6636 7483 1122 1122 1132 1132
Dian	32	64494964646666466666666666666666666666
	31	472 5617 6614 6614 708 708 803 708 803 803 1108 803 1118 1118 1118 1118
	30	444 4921 6618 6618 6618 6618 6618 7061 7066 7066 7066 7066 7066 7066 7066
	29	4410 4514 4514 6614 6698 7739 6698 7739 6698 7739 10027 1100 11100 11100 11100 11100 11100 11100 11100
	28	388 4489 4489 6537 6537 6653 6653 6653 6653 6653 6653
	27	18004874666000000000000000000000000000000
	26	69888888888888888888888888888888888888
	25	00000444400000000000000000000000000000
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PROVINCE OF QUEBEC

Table of Contents of Saw Logs, Boom and Dimension Timber in Feet Board Measure

HLE	-						D	IAMI	ETER	IN	Inc	HES					
LENGTH	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
ft.																	
10	6	9	10	15	20	28	37	42	50	62	75	83	100	117	133	154	175
11	7	10	11	16	22	31	40	46	55	69	82	92	110	128	147	170	192
12	8	11	12	18	24	34	44	50	60	75	90	100	120	140	160	185	210
13	9	12	13	19	26	37	48	54	65	81	97	108	130	152	173	200	227
1		13			28	40	51	58	70	87	105	117	140	163	187	216	245
15	11	14	15	22	30	42	55	62	75	94	112	125	150	175	200	231	262
16	12	15	16	24	32	45	59	67	80	100	120	133	160	187	213	247	280
17			17	25	34	48	62	71					170				297
18			18	27	36	51	66	75	90	112	135	150	180	210	240	277	315
19			19	28	38	54	70	79					190				
20			20	30	40	57	73	83	100	125	150	167	200	233	267	308	350
21			21	31	42	59	77	87	105	131	157	175	210	245	280	324	367
22			22	33	44	62	81	92	110	137	165	183	220	257	293	339	385
23			23	34	46	65	84	96	115	144	172	192	230	268	307	355	402
24				36	-	68	88	100	120	150	180	200	240	280	320	370	420
25			25	37	50	71	92	104	125	156	187	208	250	292	333	385	437
26			26	39	52	74	95	108	130	162	195	217	260	303	347	401	455
27			27	40	54	76	99	112	135	169	202	225	270	315	360	416	472
28			28	42	56	79	103	117	140	175	210	233	280	327	373	432	490
29			29	43	58	82	106	121	145	181	217	242	290	338	387	447	507
30			30	45	60	85	110	125	150	187	225	250	300	350	400	462	525
31			31	46	62	88	114	129	155	194	232	258	310	362	413	478	542
32			32	48	64		117								_		
33			33	49	66	93	121	137	165	206	247	275	330	385	440	509	577
34			34	51	68	96	125	142	170	212	225	283	340	397	453	524	595
35			35	52	70	99	128	146	175	219	262	292	350	408	467	540	612
36			36	54	72	102	132	150	180	225	270	300	360	420	480	555	630
37			37	55	74	105	136	154	185	231	277	308	370	432	493	570	647
38			38	56	76	108	139	158	190	237	285	317	380	443	507	586	665
39			39	57	78	111	143	162	195	244	292	325	390	455	520	601	682
40			40	60	80	114	147	167	200	250	300	333	400	467	533	617	700

TABLES RELATING TO PARTS III AND IV 251

PROVINCE OF QUEBEC

Table of Contents of Saw Logs, Boom and Dimension Timber in Feet Board Measure

				DIA	METEI	R IN	Inchi	es				TH
21	22	23	24	25	26	27	28	29	30	31	32	LENGTH
192	217	240	262	283	317	333	362	392	421	450	475	ft.
102	-1.	-10	202	200	011	000	002	002		200	2.0	-
211	238	264	289	312	348	367	399	431	463	495	522	11
230	260	288	315	340	380	400	435	470	505	540	570	12
249	282	312	341	368	412	433	471	509	547	585	617	
268	303	336	367	397	443	467	507	548	589	630	665	1
287	325	360	394	425	475	500	544	587	631	675	712	15
307	347	384	420	453	507	533	580	627	673	720	760	16
326	368	408	446	482	538	567	616	666	715	765	807	17
345	390	432	472	510	570	600	652	705	757	810	855	18
364	412	456	499	538	602	633	689	744	800	855	902	19
383	433	480	525	567	633	667	725	783	842	900	950	20
402	455	504	551	595	665	700	761	822	884	945	997	21
422	477	528	577	623	697	733	797	862	926	990	1045	22
441	498	552	604	652	728	767	834	901	968	1035	1092	23
460	520	576	630	680	760	800	870	940	1010	1080	1140	24
479	542	600	656	708	792	833	906	979	1052	1125	1187	25
498	563	624	682	737	823	867	942	1018	1094	1170	1235	26
517	585	648	709	765	855	900	979	1057	1136	1215	1282	27
537	607	672	735	793	887	933	1015	1097	1178	1260	1330	28
556	628	696	761	822	918	967	1051	1136	1220	1305	1377	29
575	650-	720	787	850	950	1000	1087	1175	1262	1350	1425	30
594	672	744	814	878	982	1033	1124	1214	1305	1395	1472	31
613	_	768	840	907	1013			1253		1440		32
632	715	792	866	935	1045	1100	1196	1292	1389	1485	1567	33
652	737	816	892	963	1077	1133	1232	1332	1431	1530	1615	34
671	758	840	919	992	1108	1167	1269	1371	1473	1575	1662	35
690	780	864	945	1020	1140	1200	1305	1410	1515	1620	1710	36
709	802		971	1048			1341	1449	1557	1665	1757	
728	823		997	1077	_	1267	1377	1488	1599		1805	
747	845		1024	1105	1235	1300	1414	1527	1641	1755	1852	39
767	867	960	1050	1133	1267	1333	1450	1567	1683	1800	1900	40

PROVINCE OF QUEBEC

Table of Contents of Saw Logs, Boom and Dimension Timber in Feet Board Measure

TH				Dr	AMETI	ER IN	INCH	ES			
LENGTH	33	34	35	36	37	38	39	40	41	42	43
ft.				1							
10	525	542	567	592	617	655	692	733	758	792	83
11	577	596	623	651	678	715	761	807	834	871	91
12	630	650	680	710	740	780	830	880	910	950	100
13	682	704	737	769	802	845	899	953	986	1029	108
14	735	758	793	828	863	910	968	1027	1062	1108	117
15	787	812	850	887	925	975	1037	1100	1137	1187	125
16	840	867	907	947	987	1040	1107	1173	1213	1267	133
17	892	921	963	1006	1048	1105	1176	1247	1289	1346	141
18	945	975	1020	1065	1110	1170	1245	1320	1365	1425	150
19	997	1029	1077	1124	1172	1235	1314	1393	1441	1504	158
20	1050	1083	1133	1183	1233	1300	1383	1467	1517	1583	166
21	1102	1137	1190	1242	1295	1365	1452	1540	1592	1662	175
22	1155	1192	1247	1302	1357	1430	1522	1613	1668	1742	183
23	1207	1246	1303	1361	1418	1495	1591	1687	1744	1821	191
24	1260	1300	1360	1420	1480	1550	1660	1760	1820	1900	200
25	1312	1354	1417	1479	1542	1625	1728	1833	1896	1979	208
26	1365	1408	1473	1538	1603	1690	1796	1907	1972	2058	216
27	1417	1462	1530	1597	1665	1755	1867	1980	2047	2137	225
28	1470	1517	1587	1657	1727	1820	1937	2053	2123	2217	233
29	1522	1571	1643	1716	1788	1885	2006	2127	2199	2296	241
30	1575	1625	1700	1775	1850	1950	2075	2200	2275	2375	250
31	1627	1679	1757	1834	1912	2015	2144	2273	2351	2454	258
32	1680	1733	1813	1893	1973	2080	2213	2347	2427	2533	266
33	1732	1787	1870	1952	2035	2145	2282	2420	2502	2612	275
34	1785	1842	1927	2012	2097	2210	2352	2493	2578	2692	283
35	1837	1896	1983	2071	2158	2275	2421	2567	2654	2771	291
36	1890	1950	2040	2130	2220	2340	2490		2730		
37	1942	2004	2097	2189	2282	2405	2559		2806		
38		2058	2153	2248	2343	2470	2628	2787	2882	-	
39	2047	2112	2210	2307	2405	2535	2697	2860	2957	3087	
40	2100	2167	2267	2367	2467	2600	2767	2933	3033	3167	333

NEW BRUNSWICK LOG RULE

gtlı Ft.					D	iam	eter	at To	p in l	Inche	3			
Length in Ft.	11	12	13	14	15	16	17	18	19	20	21	22	23	24
12	80	72	84	98	112	128	149	172	196	225	247	272	297	324
14	70	84	98	114	131	149	174	200	228	262	288	317	336	380
16	80	96	112	130	150	170	198	229	261	300	327	362	376	432
18	90	108	126	147	168	192	223	258	294	337	370	408	445	486
20	100	120	140	163	187	213	248	286	326	375	411	453	495	540
21	105	126	147	171	196	223	261	301	343	393	432	476	519	569
22	110	132	154	179	205	234	275	315	359	412	453	498	544	594
24	120	144	168	196	224	256	298	344	392	450	494	544	594	648
26	142	168	196	226	259	298	346	396	453	509	560	614	560	730
28	154	182	212	245	280	323	374	428	490	550	605	653	716	788
30	164	194	226	261	299	344	398	457	523	588	644	698	756	840
32	176	208	242	280	320	368	427	490	561	627	689	738	808	898
34	186	220	256	297	336	390	452	519	594	664	732	784	877	952
36	198	234	273	315	360	415	481	552	631	707	778	853	931	1011
38	208	246	287	331	379	436	506	580	663	745	829	898	981	1065
40	220	260	303	350	400	461	534	612	701	786	864	948	1035	1123
42	231	273	318	367	419	484	562	644	736	825	908	995	1088	1181
44	242	286	333	384	439	509	590	674	771	865	951	1042	1138	1235
46	252	298	347	401	458	531	613	703	804	903	992	1088	1188	1289
48	264	312	364	420	480	554	642	736	842	944	1038	1138	1242	1348
50	286	336	392	450	515	596	690	788	903	1003	1104	1208	1308	1430

UNDERSIZED LOGS

A log measuring 7 inches at the top contains twice as many superficial feet as its own length.

A log measuring 9 inches, 2½ times its length.
A log measuring 9 inches, 3 times its length.
A log measuring 10 inches, 4 times its length.

CLARK'S INTERNATIONAL LOG RULE

Diameter						Leng	th —	Feet					
Dian	8	9	10	11	12	13	14	15	16	17	18	19	20
Ins.					Vo	lume	— Во	ard F	`eet				
6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 32 4 26 27 28 29 30 31 32 33 34 43 44 44 44 44 44 44 44 44 44 44	10 15 20 25 30 40 40 95 10 10 11 10 11 13 15 15 20 15 20 15 20 15 20 15 20 15 20 31 40 40 40 40 40 40 40 40 40 40 40 40 40	945 990 1035	1005 1055 1100 1150	1010 1060 1110 1160 1215 1270	1050 1100 1155 1215 1270 1330 1390	975 1030 1085 1140 1200 1260 1320 1380 1445 1510	1000 1055 1110 1170 1230 1295 1360 1425 1490 1560 1630	400 500 655 800 1200 1200 1200 1200 1200 1200 1200	105 1300 175 2000 225 255 255 255 255 255 255 2900 3200 3200 355 3390 4300 510 555 600 645 8500 905 1210 1210 1210 1210 1210 1210 1210 12	1030 1095 1160 1225 1295 1365 1435 1510 1585 1665 1745 1825 1910	1030 1095 1160 1230 1300 1375 1450 1525 1605 1685 1770 1855 1940 2030 2120	1025 1090 1160 1230 1305 1380 1455 1535 1615 1700 1785 1870 2050 2145 2240	1375 1455 1535 1620 1705 1795 1885 1975 2070 2165 2265 2365

SPAULDING LOG RULE OF COLUMBIA RIVER

GTE					Di	AMET	ER	IN I	NCI	TE8				
LENGTH	10	11	12	13	14	15	16	6	17	18	19	20	21	22
ft. 12 14 16 18 20	38 44 50 57 63	47 55 63 70 78	58 67 77 87 96	71 82 94 106 118	86 100 114 129 143	103 120 137 154 171	12 14 16 18 20		141 164 188 211 235	16: 18: 21: 24: 27:	$ \begin{array}{c c} 214 \\ 5 245 \\ 3 276 \end{array} $	241 276 310	269 308 346	256 298 341 384 426
22 24 26 28 30	89 76 82 88 94	86 94 101 109 117	106 116 125 134 144	130 142 153 164 176	157 172 186 200 214	188 206 223 240 257	22 24 26 28 30	2 3 2 3	158 182 305 328 352	29' 32' 35 37' 40'	4 368 1 398 8 428	414 448 482	423 462 500 538 577	469 512 554 596 639
32 34 36 38 40	101 107 113 120 126	125 132 140 148 156	154 164 174 183 192	188 200 212 224 236	228 243 258 272 286	274 291 308 325 342	32 34 36 38 40	2 3 4 2 4	376 399 122 146 170	43: 45: 48: 51: 54:	521 552 3 582	586 620 655	616 654 602 731 770	682 725 768 810 852
42 44 46 48 50	132 138 145 151 157	164 172 179 187 195	202 212 222 232 241	248 260 272 284 295	300 314 329 344 358	359 376 394 412 429	42 44 46 48 50	2 3 5 4 5	193 516 540 564 587	563 594 623 648 673	674 1 705 8 736	758 793 828	808 846 885 924 962	895 938 981 1024 1066
	23	24	25	26	27	2	В	29	1	30	31	32	33	34
ft. 12 14 16 18 20	282 329 376 423 470	309 360 412 463 515	393 449 505	42	7 46 8 52 9 59	52 4 28 5 94 6	27 98 69 40	45 53 61 68 76	5 2 8	492 574 656 738 820	526 613 701 789 876	561 654 748 841 935	597 696 796 895 995	634 739 845 951 1056
22 24 26 28 30	517 564 611 658 705	566 618 669 720 772	617 674 730 786 842	67: 73: 79: 85: 91:	2 79 3 85 4 92	2 8 8 9 4 9	82 54 25 96 67	84 913 99- 107- 114	8 1 1 0 1	902 984 066 148 230	964 1052 1139 1226 1314	1028 1122 1215 1308 1402	1094 1194 1293 1392 1492	1162 1268 1373 1478 1584
32 34 36 38 40	752 799 846 893 940	824 875 926 978 1030	898 954 1010 1066 1122	103 109 115	7 112 8 118 9 128	22 12 38 12 54 13	09 80 51	122- 130- 137- 145- 153-	0 1 6 1 3 1	312 394 476 558 640	1402 1490 1578 1665 1752	1496 1589 1682 1776 1870	1592 1691 1790 1890 1990	1690 1796 1902 2007 2112
42 44 46 48 50	987 1034 1081 1128 1175	1081 1132 1184 1236 1287	1178 1234 1291 1348 1404	134 140 146	2 145 3 15 4 158	52 15 18 16 34 17	64 36 08	160 168 175 183 191	2 1 1 6 1	722 804 886 968 050	1840 1928 2016 2104 2191	1963 2056 2150 2244 2337	2089 2188 2288 2388 2487	2218 2324 2430 2536 2641

SPAULDING LOG RULE - continued

GTH					DIAM	ETER	IN IN	снка				
LENGTH	35	36	37	38	39	40	41	42	43	44	45	46
ft. 12 14 16 18 20	673 785 897 1009 1121	713 831 950 1069 1188	755 880 1006 1132 1258	798 931 1064 1197 1330	843 983 1124 1264 1405	889 1037 1185 1333 1481	936 1092 1248 1404 1560	984 1148 1312 1476 1640	1033 1205 1377 1549 1721	1086 1267 1448 1629 1810		1186 1383 1581 1779 1976
22 24 26 28 30	1233 1346 1458 1570 1682	1307 1426 1544 1662 1781	1384 1510 1635 1760 1886	1463 1596 1729 1862 1995	1545 1686 1826 1966 2107	1629 1778 1926 2074 2222	1716 1872 2028 2184 2340	1804 1968 2132 2296 2460	1893 2066 2238 2410 2582	1991 2172 2353 2534 2715	2646	2174 2372 2569 2766 2964
32 34 36 38 40	1794 1906 2018 2130 2242	1900 2019 2138 2257 2376	2012 2138 2264 2390 2516	2128 2261 2394 2527 2660	2248 2388 2528 2669 2810	2370 2518 2666 2814 2962	2496 2652 2808 2964 3120	2624 2788 2952 3116 3280	2754 2926 3098 3270 3442	2896 3077 3258 3439 3620	3213 3402	3162 3360 3558 3755 3952
42 44 46 48 50	2354 2466 2579 2692 2804	2495 2614 2733 2852 2970	2642 2768 2894 3020 3145	2793 2926 3059 3192 3325	2950 3090 3231 3372 3512	3110 3258 3407 3556 3704	3276 3432 3588 3744 3900	3444 3608 3772 3936 4100	3614 3786 3959 4132 4304	3801 3982 4163 4344 4525	3969 4158 4347 4536 4725	4150 4348 4546 4744 4941
	47	48	49	50	51	52	53 -	54	55	56	57	58
ft. 12 14 16 18 20	1239 1445 1652 1858 2065	1293 1508 1724 1939 2155	1348 1572 1797 2022 2246	1404 1638 1872 2106 2340	1461 1704 1948 2191 2435	1519 1772 2025 2278 2531	4578 1841 2104 2367 2630	1638 1911 2184 2457 2730	1700 1983 2266 2550 2833	1763 2056 2350 2644 2938	1827 2131 2436 2740 3045	1893 2208 2524 2839 3155
22 24 26 28 30	2271 2478 2684 2890 3097	2370 2586 2801 3016 3232	2470 2696 2920 3144 3369	2574 2808 3042 3276 3510	2678 2922 3165 3408 3652	2784 3038 3291 3544 3797	2893 3156 3419 3682 3945	3003 3276 3549 3822 4095	3116 3400 3683 3966 4249	3232 3526 3819 4112 4406	3349 3654 3958 4262 4567	3470 3786 4101 4416 4732
32 34 36 38 40	3304 3510 3716 3923 4130	3448 3663 3878 4094 4310	3594 3819 4044 4268 4492	3744 3978 4212 4446 4680	3896 4139 4382 4626 4870	4050 4303 4556 4809 5062	4208 4471 4734 4997 5260	4368 4641 4914 5187 5460	4532 4816 5100 5383 5666	4700 4994 5288 5582 5876	4872 5176 5480 5785 6090	5048 5363 5678 5994 6310
42 44 46 48 50	4336 4542 4749 4956 5162	4525 4740 4956 5172 5387	4716 4940 5166 5392 5616	4914 5148 5382 5616 5850	5113 5356 5600 5844 6087	5315 5568 5822 6076 6329	5523 5786 6049 6312 6575	5733 6006 6279 6552 6825	5949 6232 6516 6800 7083	6170 6464 6758 7052 7345	6394 6698 7003 7304 7612	6625 6940 7256 7572 7887

TABLES RELATING TO PARTS III AND IV 257

SPAULDING LOG RULE - continued

LENGTH					DIAM	ETER	IN IN	CHES				
LEN	59	50	61	62	63	64	65	56	67	68	69	70
ft. 12 14 16 18 20	1960 2286 2613 2940 3266	2028 2366 2704 3042 3380		2530 2892 3253	2241 2614 2988 3361 3735	2315 2700 3086 3472 3858		2878 3289 3700		2625 3062 3500 3937 4375	2706 3157 3608 4059 4510	4183
22 24 26 28 30	3592 3920 4246 4572 4899	4056 4394 4732	4196 4545 4894	4338 4699 5060	4108 4482 4855 5228 5602	4244 4630 5015 5400 5786	4780	5345		5250 5687 6124	4961 5412 5863 6314 6765	6042
32 34 36 38 40	5226 5553 5880 6206 6532	5408 5746 6084 6422 6760	5944	6145 6506	5976 6349 6722 7096 7470	6558	6771 7170	6989 7400 7811	6786 7210 7634 8058 8482	7437 7874	7216 7667 8118 8569 9020	7901 8366
42 44 46 48 50	6858 7184 7512 7840 8166	7436 7774	7692 8042	7952 8314	7843 8216 8590 8964 9337	8102 8488 8874 9260 9645			8906 9330 9755	9187 9624	9471 9922	9761

BRITISH COLUMBIA LOG SCALE

Established by the government, and derived from the following rule: — Deduct 1½ inches from the mean diameter of the log at the small end; square the result and multiply by .7854; deduct ¾; divide by 12; multiply by the length of the log in feet.

Logs more than 40 and not over 50 feet long to be scaled as two logs of equal length, the butt log taken as 1 inch larger than the top. Logs over 50 and not over 60 feet long to be treated similarly, but with 2 inches rise allowed to the butt log; and so on, 1 inch of rise being added for each 10 feet or part thereof over 40 feet.

GTH						Di	AME'	PER I	IN I	NCHE	18					
LENGTH	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
ft. 1 10	3 34	43	5 53		7 74	9 87			13 130					22 220		26 263
12 14 16		52 60 69	63 73 84		104	104 121 139	140	160	181	204	228	253	280	308	337	315 368 421
18 20	62	77	94	113	134	156 173	180 200	206 229	233 259	262 292	293 326	326 362	360 400	396	434	473 526
22 24 26	76 83 89	103 112	126 136	164	178 193	208 226	$\frac{240}{260}$	$\begin{array}{c} 274 \\ 297 \end{array}$	337	$\frac{350}{379}$	391 424	434 471	480 520	528 572	578 626	631 683
30	103	120 129	157	189	208	260	300		389	437		543	600	•	723	736 789
34 36	117 124 131	146 155	178 189	227	238 253 268 283	$\frac{295}{312}$	340 360	389 412	415 441 466 492	$\frac{496}{525}$	554 586	$\begin{array}{c} 615 \\ 652 \end{array}$	720	748 792	819 867	841 894 946 999
	138				297				518							

BRITISH COLUMBIA LOG SCALE - continued

нде					DIAM	ETER	IN IN	CHES				
LENGTH	26	27	28	29	30	31	32	33	34	35	36	37
ft. 1 10	29 286	31 309	33 334	36 360	39 387	41 414	44 443	47 472	50 503	534 534	57 567	600
12 14 16 18	343 400 457 514	371 433 495 557	401 468 535 602	432 504 576 648	464 541 619 696	497 580 663 746	531 620 708 797	567 661 756 850	603 704 804	641 748 855 961	680 793 906 1020	720 840 960 T080
20	571	619	668	720	773	828	886	945	1005	1068	1133	1200
22 24 26 28 30	629 686 743 800 857	681 743 805 867 928	735 802 869 936 1003	791 864 936 1008 1080	850 928 1005 1082 1160	911 994 1077 1160 1243	974 1063 1151 1240 1328	1039 1133 1228 1322 1417	1106 1207 1307 1408 1508	1175 1282 1389 1496 1602	1246 1360 1473 1586 1700	1320 1440 1560 1679 1799
32 34 36 38 40	914 971 1028 1086 1143	990 1052 1114 1176 1238	1070 1136 1203 1270 1337	1152 1224 1296 1368 1440	1237 1314 1392 1469 1546	1325 1408 1491 1574 1657	1417 1505 1594 1682 1771	1511 1606 1700 1795 1889	1609 1709 1810 1910 2011	1709 1816 1923 2030 2137	1813 1926 2039 2153 2266	1919 2039 2159 2279 2399
	38	39	40	41	42	43	44	45	46	47	48	49
ft. 1 10	63 634	67 669	71 705	74 743	78 781	82 820	86 860	90 901	94 943	99 985	103 1029	107 1074
12 14 16 18 20	761 888 1015 1141 1268	803 937 1071 1205 1339	847 988 1129 1270 1411	891 1040 1188 1337 1485	937 1093 1249 1405 1561	984 1148 1312 1475 1639	1032 1204 1376 1547 1719	1081 1261 1441 1621 1801	1131 1320 1508 1697 1885	1182 1379 1577 1774 1971	1235 1441 1647 1852 2058	1289 1503 1718 1933 2148
22 24 26 28 30	1395 1522 1649 1775 1902	1472 1606 1740 1874 2008	1552 1693 1834 1975 2116	1634 1782 1931 2079 2228	1717 1874 2030 2186 2342	1803 1967 2131 2295 2459	1891 2063 2235 2407 2579	1981 2161 2342 2522 2702	2074 2262 2451 2639 2828	2168 2365 2562 2759 2956	2264 2470 2676 2882 3087	2362 2577 2792 3007 3222
32 34 36 38 40	2029 2156 2283 2410 2536	2142 2276 2410 2543 2677	2258 2399 2540 2681 2822	2376 2525 2673 2822 2970	2498 2654 2810 2967 3123	2623 2787 2951 3115 3279	2751 2923 3095 3267 3439	2882 3062 3242 3422 3602	3016 3205 3393 3582 3770	3153 3350 3547 3744 3941	3293 3499 3705 3911 4117	3436 3651 3866 4081 4295

BRITISH COLUMBIA LOG SCALE - continued

LENGTH					DIAM	ETER	IN INC	CHES				
LEN	50	51	52	53	54	55	56	57	58	59	GO	61
ft.	112	117	121	126	131	136	141	147	152	157	163	168
1	1120	1166	1214	1262	1312	1362	1414	1466	1519	1574	1629	1685
12	1343	1399	1457	1515	1574	1635	1696	1759	1823	1888	1955	202
14	1567	1633	1699	1767	1837	1907	1979	2052	2127	2203	2280	235
16	1791	1866	1942	2020	2099	2180	2262	2346	2431	2518	2606	269
18	2015	2099	2185	2272	2361	2452	2545	2639	2735	2832	2932	303
20	2239	2332	2428	2525	2624	2725	2827	2932	3039	3147	3258	337
22	2463	2566	2670	2777	2886	2997	3110	3225	3343	3462	3583	370°
24	2687	2799	2913	3030	3148	3269	3393	3519	3646	3777	3909	404°
26	2911	3032	3156	3282	3411	3542	3676	3812	3950	4091	4235	438°
28	3135	3265	3399	3535	3673	3814	3958	4105	4254	4406	4561	471°
30	3359	3499	3641	3787	3936	4087	4241	4398	4558	4721	4886	505°
32	3583	3732	3884	4039	4198	4359	4524	4691	4862	5036	5212	539:
34	3807	3965	4127	4292	4460	4632	4807	4985	5166	5350	5538	572:
36	4030	4198	4370	4544	4723	4904	5089	5278	5470	5665	5864	606:
38	4254	4432	4612	4797	4985	5177	5372	5571	5774	5980	6190	640:
40	4478	4665	4855	5049	5247	5449	5655	5864	6077	6294	6515	674:
	62	63	64	65	66	67	68	69	70	71	72	73
ft.	174	180	186	192	198	204	210	217	223	230	237	24
1	1742	1800	1859	1919	1980	2042	2105	2169	2233	2299	2366	243
12	2091	2160	2231	2303	2376	2450	2526	2602	2689	2759	2839	292
14	2439	2520	2603	2687	2772	2859	2947	3036	3127	3219	3312	340
16	2787	2880	2975	3071	3168	3267	3368	3470	3573	3678	3785	389
18	3136	3240	3347	3454	3564	3676	3789	3903	4020	4138	4258	438
20	3484	3600	3718	3838	3960	4084	4210	4337	4467	4598	4731	486
22	3833	3960	4090	4222	4356	4492	4631	4771	4913	5058	5204	535
24	4181	4320	4462	4606	4752	4901	5051	5205	5360	5518	5677	584
26	4529	4680	4834	4990	5148	5309	5472	5638	5807	5977	6151	632
28	4878	5040	5206	5374	5444	5717	5893	6072	6253	6437	6624	681
30	5226	5401	5578	5757	5950	6126	6314	6506	6700	6897	7097	730
32 34 36 38	5575 5923 6272 6620 6968	5761 6121 6481 6841 7201	5949 6321 6693 7065 7437	6141 6525 6909 7293 7677	6336 6732 7128 7524 7920	6534 6943 7351 7759 8168	6735 7156 7577 7998 8419	6939 7373 7807 8240 8674	7146 7593 8040 8486 8933	7357 7816 8276 8736 9196	7570 8043 8516 8989 9462	778 827 876 924 973

VOLUME TABLE No. 1. WHITE PINE BY THE SCRIBNER RULE

Breast Diam.				Total	Heigh	at of T	ree — l	Feet		
Inches	60	70	80	90	100	110	120	130	140	150
10	60	70	80	95						
11	75	85	100	115						
12	90	100	115	135						
13	100	115	135	155	180					
14	120	135	155	180	210					
15	140	160	180	200	230	270				
16	160	185	210	240	270	310				
17	0	210	240	270	310	350				
18		240	270	310	350	390	440			
19		270	310	350	390	440	490			
20			350	390	440	490	550			
21			390	430	480	540	600	680		
22			440	480	540	600	670	750		
23			490	540	600	660	740	830	940	
24			540	600	660	730	810	910	1020	
25				660	720 790	800	890	990 1070	1100	1000
26 27				720		870 940	970	1150	1190	1320
28					850 920	1020	1130	1240	1370	1420
29					990	1100	1210	1330	1470	1640
30						1180	1300	1420	1580	1750
31						1270	1400	1520	1690	1860
32		111		1		1360	1500	1630	1800	1980
33						1450	1600	1750	1920	2100
34						1550	1700	1870	2040	2220
35						1650	1800	1980	2170	2360
36						1750	1900	2100	2300	2500

Based on 3000 trees cut in New York, the Lake States, and Canada, cut as a rule into 16-foot logs. These scaled with due allowance for crook and breakage, but not for decay. Original.

VOLUME TABLE No. 2. RED PINE, IN BOARD FEET, BY THE MINNESOTA SCRIBNER RULE

(Trees under 130 Years Old)

Diameter Breast High		Tota	al Height in	Feet	
Inches	60	70	80	90	100
7 8 9 10 11 12 13 14 15 16 17 18	17 29 44 61 80 100 120 140	24 38 53 72 92 114 138 164 190	50 68 88 110 136 160 189 220 252	81 104 130 159 189 222 257 296 334 372	94 119 148 180 214 250 292 340 394 450

VOLUME TABLE No. 3. RED PINE, IN BOARD FEET, BY THE MINNESOTA SCRIBNER RULE

(Trees over 200 Years Old)

Diameter Breast High		Total Hei	ght in Feet	
Inches	70 .	80	90	100
10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27	85 102 122 144 168 193 220 250 282 317 355 396	105 126 150 176 208 240 275 311 349 390 433 480 530 584	147 177 210 246 284 323 370 417 468 523 582 646 715 790 867 951	383 435 490 551 616 685 755 830 905 986 1075 1166

The preceding tables from Minnesota timber cut into 16-foot logs and scaled straight and sound. By H. H. Chapman.

VOLUME TABLE No 4. WHITE PINE IN FEET-BOARD MEASURE

(From State Forester of Massachusetts)

Diameter Breast High			Total 1	Height (of Tree	— Feet		
Inches	30	40	50	60	70	80	90	100
56 77 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26	10 15 20 25 30 40 	20 30 35 45 55 65 75 85 100 115	30 40 50 60 75 90 105 120 140 180 	50 65 80 95 115 135 155 175 200 230 260 295 335 385	65 85 105 125 145 165 190 215 245 275 310 350 390 435 480 526 600 645	115 145 170 200 235 265 360 335 370 410 455 505 595 590 690 740	200 230 280 340 340 380 425 475 530 680 635 680 780 780 886 885	230 260 265 335 335 375 420 470 530 600 600 720 780 835 890 940 995

Gives yield of trees from $\frac{1}{2}$ foot stump to 4 inches in the top as sawed into round or waney-edged, or both round and square-edged, lumber. In the smallest sizes of trees appreciably more may be obtained by cutting to a smaller size in the top.

VOLUME TABLE No. 5. WHITE PINE IN CORDS
(From State Forester of Massachusetts)

Diameter Breast High		7	rotal Hei	ght of T	ree — Fe	et	
Inches	30	40	50	60	70	80	90
5 6 7 8 9 10 11 12 13 14 15	.03 .03 .04 .05 .07	.04 .05 .07 .09 .11 .13 .15 .17	.05 .07 .09 .11 .13 .16 .19 .22 .25 .28	 .09 .11 .13 .16 .19 .22 .26 .30 .34	 .13 .16 .19 .23 .27 .31 .34 .40	 .22 .26 .31 .36 .41	

Includes volume of tree above $\frac{1}{2}$ foot from ground and up to 4 inches diameter in the top.

VOLUME TABLE No. 6. SPRUCE IN CUBIC FEET

Breast Diam- eter		Total Height of Tree — Feet									
Inches	40	45	50	55	60	65	70	75	80	90	
6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24	4.9 6.3 7.8 9.8 12.0	5.3 6.9 8.6 10.8 13.5 16.0 18.5 22.	5.8 7.6 9.5 12.0 15.0 18.0 21. 24. 28. 31.	6.5 8.5 10.6 13.4 16.5 19.7 23. 27. 30. 34. 38. 43. 47. 52. 56. 	9.6 12.0 15.0 18.2 22. 25. 29. 33. 37. 41. 46. 50. 55. 60.	114 17 20 23 27 31 36 40 44 49 54 59 65 72 79 96	21 25 29 34 38 43 47 52 58 64 70 777 84 92 100	27 32 36 41 46 51 56 62 69 76 82 88 88 95	34 39 44 49 55 61 67 74 81 87 93 100	63 70 77 85 93 98 105 114 123	

Table No. 6 gives volume of tree from ground to tip exclusive of branches. Includes bark, which is about 123 per cent of the total volume. Based on 2500 trees cut in Maine, New Hampshire, and New York, calipered each 4 feet, computed separately, and averaged. Original.

This table may without great modification be applied to other soft wood species, regard being had to the remarks on tree form on pages 159-165 of this volume. Balsam fir. however, is believed to be pretty uniformly somewhat slimmer than spruce, having, as would appear from the results of a study on fir made by Mr. Zon of the United States Forest Service, 8 per cent less volume for the same breast diameter and height.

VOLUME TABLE No. 7. SPRUCE IN FEET, BOARD MEASURE

Breast Diam- eter		Total Height of Tree — Feet								
Inches	40	45	50	55	60	65	70	75	80	90
7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24	20 20 30 40	20 25 35 45 55 65 75 	20 30 40 50 65 75 90 105 120	25 35 45 60 70 85 100 120 135 155 170 185 205 235	25 40 50 65 80 100 115 135 175 170 210 235 265 300 330 400	45 55 70 90 110 125 150 170 235 260 295 330 360 440	80 105 120 140 165 190 205 230 255 290 325 360 395 435 480	115 135 135 155 180 205 225 225 280 320 320 430 470 515	150 170 195 220 250 275 310 350 385 425 465 510 555	315 350 390 430 470 550 650

Based on 2500 trees scaled in 16-foot log lengths up to 6 inches in diameter by the Maine rule and discounted from 5 to 10 per cent. Purports to give the yield in edged lumber of average spruce trees in economical woods and mill practice.

VOLUME TABLE No. 8. SPRUCE IN CORDS

Breast Diameter		Total Height of Tree — Feet							
Inches	40	45	50	55	60	65	70	75	80
6 7 8 9 10 11 12 13 14 15 16 17 17 18 19 20	.04 .06 .07 .09 .11	.05 .06 .08 .10 .12 .15 .18 .21	.05 .07 .09 .12 .14 .17 .20 .23 .26	.06 .08 .10 .13 .16 .19 .22 .25 .29 .32 .36 .40 .45 .49	.09 .12 .14 .17 .20 .24 .27 .31 .35 .39 .48 .52 .57	 .13 .16 .19 .22 .26 .30 .34 .38 .42 .46 .50 .56 .62	 .20 .24 .28 .32 .36 .40 .45 .50 .55 .60	 .22 .26 .30 .34 .39 .43 .48 .54 .65 .72	 .28 .32 .37 .42 .47 .52 .59 .64 .70

Table No. 8 derived from Table No. 6 by deducting a fair allowance for waste in stump, also volume of top above 4 inches diameter, and dividing by 96, usual number of cubic feet, solid wood, in a piled cord. The values in this table are very closely confirmed by a table for second growth spruce based on 711 trees that was made up in 1903 by Mr. T. S. Woolsey of the United States Forest Service.

This table may be used for balsam fir, but in general with some deduction. For the amount of this deduction see the preceding page.

YIELD OF HEMLOCK BARK

Where the tanbark industry is large and well organized, 2240 lbs. of dried bark constitute one cord. One thousand feet of hemlock timber, log scale, yields \(\frac{3}{4}\) cord usually, up to a cord in some cases. Small, thrifty hemlock, if closely utilized at the saw, as in parts of New England, yields about \(\frac{1}{6}\) cord per M.

VOLUME TABLE No. 9. HEMLOCK, BY THE SCRIBNER RULE

(From Bulletin No. 152, U. S. Dept. Agriculture, by E. H. Frothingham)

Diam- eter			Total I	Height	of Tree	— Feet	t		Diam- eter inside	
breast- high,	30	40	50	60	70	80	90	100	bark of top	
Inches		Feet Board Measure								
8 9 10 111 122 133 144 155 166 177 188 199 200 211 222 233 244 256 277 288 299 30	5 8 12 16 20 	7 14 22 29 37 46 56 5	13 22 32 42 53 65 77 90 110 120 140 180 200 220 	20 29 40 51 64 78 95 110 130 150 200 230 260 230 260 330 360 430 470 500 540 570	25 35 47 60 76 94 110 130 160 1210 2210 2210 2310 350 380 460 550 550 640 680	40 52 67 84 100 130 150 210 240 280 350 390 440 490 530 580 640 690 750 800	75 93 110 140 160 190 220 340 340 480 480 660 660 660 720 780 850 920	200 240 280 320 360 410 520 580 650 720 790 870 940 1030	6 6 6 7 7 7 7 8 8 8 8 9 9 10 10 10 11 11 11 11 11	

Based on 534 trees cut in the Lake States and scaled from a 2-foot stump to diameter given in 16.3 foot log lengths. Crook, breakage, and defect not allowed for.

VOLUME TABLE No. 10. HEMLOCK IN BOARD FEET
(From Report N. H. Forest Commission for 1906-7)

Diameter Breast High		Total H	eight of Tre	e — Feet	
Inches	30	40	50	60	70
6 7 8 9 10 11 12 13 14 15 16 17	5 10 17 26 36 47 60	20 28 36 46 58 72 88 107 126 148	30 39 49 59 72 86 104 125 148 171 197	42 50 60 71 86 103 124 147 172 200 233	86 103 123 148 173 204 240 281

Based on 317 second growth trees grown in New Hampshire, cut with good economy $(4\frac{1}{2} \text{ to } 6\frac{1}{2} \text{ inches in the top})$ and sawed into edged boards and scantling. Figures derived from actual tally of the sawed lumber.

VOLUME TABLE No. 11. PAPER BIRCH IN CORDS (Adapted from Report of N. H. Forest Commission for 1906-7)

Diameter Breast High		Used Le	ngth of Tree	e — Feet	
Inches	10	20	30	40	50
6 7 8 9	.02 .03 .04 .05	.04 .05 .07 .08	.05 .07 .09	.07 .08 .11	.08 .10 .13 .16
10 11 12 13	.05 .07 .08	.10 .12 .14 .17	.13 .16 .19 .22	.16 .19 .22 .26	.19 .22 .26 .30
14 15	**	.19 .22	.25 .29	.30 .34	.34

Based on 427 trees cut to be sawed. Volumes given are of used portion of tree only. Original figures by Forest Service men in cubic feet converted into cords at the ratio of 96 cubic feet solid per cord.

VOLUME TABLE No. 12. RED OAK IN BOARD FEET (From Report of N. H. Forest Commission for 1906-7)

Diameter Breast High		Used L	ength of Tre	e — Feet	
Inches	10	20	30	40	50
5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20	7 9 14 18 25 31 37 44 54 65	15 22 30 40 50 63 78 93 109 124 143 163 181 202 223	29 39 48 60 74 89 107 126 149 173 201 232 265 300	34 43 58 73 90 110 132 160 190 225 262 308 356 405	99 118 143 174 208 243 288 330

Based on about 700 trees tallied through saw mills by members of United States Forest Service. Trees from 50 to 80 years of age, cut off at from 5 to 9 inches at the top. Lumber sawed round or waney-edged; 85 per cent of the product 1½-inch boards surveyed as 1 inch; balance 1½-inch plank.

Table may be used for other second growth hard wood species when similarly cut and manufactured.

VOLUME TABLE No. 13. PEELED POPLAR IN CORDS (Adapted from Report of N. H. Forest Commission for 1906-7)

Diameter Breast High		Total Height o	of Tree — Feet	,
Inches	50	60	70	80
5 6 7 8 9 10 11 12 13	.02 .03 .05 .06 .08	.02 .04 .06 .08 .11 .13	.05 .07 .10 .13 .16 .20 .25 .30	 .08 .12 .15 .18 .24

Based on 289 trees cut for pulp wood. All diameter measures except diameter breast high taken on the wood surface after peeling off the bark. Original figures in cubic feet, converted into cords at the ratio of 90 cubic feet solid wood per cord.

TABLE 14. SECOND GROWTH HARD WOODS IN CORDS

		Total Height of Tree — Feet												
Diam. Breast High	30	35	40	45	50	55	60	65						
Inches		N	lumber	Trees p	er Cord	- ''								
3-5 5-7	61	47	38 24	33 20	31 17	15	14	9						
3-5 5-7 7-9		100		33 20	31 17 12	15 11	14 19							

From study by Harvard Forest School on oak thinnings. Wood used up to 2 inches in diameter. 80 cubic feet solid wood per cord.

The study showed that when the bolts from the trees 3 to 5 inches in breast diameter were piled by themselves, there were 250 bolts and 67 cubic feet in a cord; wood from the 5- to 7-inch trees piled together gave 173 bolts and $79\frac{1}{2}$ cubic feet; from the 7- to 9-inch trees, 133 bolts and 91 cubic feet.

FORM HEIGHT FACTORS FOR SECOND GROWTH HARD WOODS IN CORDS

(Utilized to 1 inch in diameter; 80 cubic feet solid wood per cord.) Sectional Area Breast High × F. H. F. = Cords of 128 Cubic Feet of Wood

Diameter	Basal	Total Height in Feet						
Breast High	Area	40	50	60				
Inches	Sq. Ft.	Form Height Factors						
6 7 8 9 10 11 12	.196 .267 .349 .442 .545 .660	.26 .26 .27	.31 .31 .32 .33 .35 .37	.37 .38 .38 .40 .43 .45				

SAME FOR CHESTNUT EXTRACT WOOD

(Smaller trees used to 5 inches; 90 cubic feet solid wood per cord.) Sectional Area Breast High × F. H. F. = Cords of 128 Cubic Feet of Wood

Diameter		Total Height of Tree in Feet											
Breast High	40	40 50 60 70 80 90											
Inches		Form Height Factors											
6 9 12 15 18 21 24 27 30 36 45	.20 .18 .18 .17 	.23 .21 .21 .20 .19 .19 .18 .18	.28 .25 .23 .22 .22 .21 .21 .21	.30 .27 .26 .25 .24 .24 .24 .23 .22	 .31 .29 .28 .27 .27 .27 .26 .25 .26	 .34 .32 .31 .30 .30 .29 .28 .28	 .38 .36 .34 .33 .32 .31 .31						

Above tables from "Biltmore Timber Tables," by Howard Krinbill, copyrighted.

To use, caliper or estimate the breast diameter of the tree or stand and get the total height. Then multiply the basal area in square feet (see table on page 238) by the proper factor in the table above. The product gives the result in cords. Considerable stands of timber should be divided into diameter groups.

Example 1. A 10-inch tree is 50 feet high. How much cordwood is in it? .545 (basal area) \times .35 (form height factor) = .19 cord; or $1 \div .19 = 5\frac{1}{4}$, number of such trees required for a cord if closely utilized.

Example 2. A bunch of chestnut averaging 80 feet tall and running 13 to 17 inches in diameter, to be cut into extract wood, proves after calipering to have a total basal area of 95 square feet. $95 \times .29$ (form height factor in second table above) = 27.55, number of cords in the stand.

VOLUME TABLE No. 16. HARD WOODS, IN BOARD FEET, BY THE SCRIBNER RULE

(From	R.	A.	Brotherton,	Negaunee,	Mich.)
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Stump Diameter	Number of Sixteen-Foot Logs									
Inches	1	2	3	4						
10	30	50	90							
12 14	55 80	95 140	130 180							
16	110	180	250							
18	140	250	340	390						
20	190	320	440	540						
22	240	400	550	650						
24	300	470	640	750						
26	360	560	740	900						
28	420	680	900	1100						
30	500	820	1100	1350						

Stumps average about 3 feet high. One and two log trees may either be short trees, or those that above a certain height are faulty or defective.

Elm in the sizes above 18 inches yields about 10 per cent more than the above figures.

VOLUME TABLE No. 17. NORTHERN HARD WOODS (BIRCH, BEECH AND MAPLE) BY THE SCRIBNER RULE

(Adapted from Bulletin No. 285, U. S. Forest Service. by E. H. Frothingham)

Diameter breast-		N	lumber	of 16-f	oot Lo	gs		Diameter inside
high	1	1 1/3	2	21/2	3	31/2	4	bark of top
Inches			Volume	— Bos	rd Fee	t		Inches
0 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24	20 20 25 25 30 30 	30 35 40 50 55 65 70 80	45 50 60 70 80 95 110 120 140 160	70 80 95 110 130 140 160 190 210 240 270 300 340 380 420	100 120 140 160 180 210 240 270 300 340 380 430 530	140 170 190 220 250 280 320 410 460 520 580 640	230 260 290 320 380 430 490 550 620 690 770	6 6 7 7 7 7 8 8 9 9 10 10 11 12 12 13

Based on 800 trees cut in the Lake States scaled from taper measures in logs 16.3 feet long from a stump 1 foot high to top diameters found in actual logging: figures evened by curves. As no allowance was made for crook and defect, considerable discount is necessary in most timber.

Note. Comparison between the values in this table and the preceding NOTE. Comparison between the values in this table and the preceding shows striking differences, and the text indicates how these arose, from differences in tree form and soundness, lumbering practice, and methods of recording and computing. The cruiser is under obligation before he applies either in practice to understand these points, and he will do well to check the table he uses with local practice and on local timber. That done, however, the tables will apply throughout the distribution of the species.

VOLUME TABLE No. 18. LONGLEAF PINE, IN BOARD FEET, BY THE SCRIBNER RULE

Diam- eter		Total Height of Trees — Feet											
breast- high	40	50	60	70	80	90	100	110	120	inside bark of top			
Inches					Volum	е				Inches			
7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36	5 10 20 25 35	10 20 30 40 50 65 80 80 95 115 	15 25 400 90 91 110 130 175 2250 2255 2250	50 70 90 115 160 220 2250 2250 330 440 440	110 135 165 200 260 280 295 370 470 520 580 640 780 840	195 230 350 350 350 350 360 440 490 550 610 670 740 820 890 1050 1140	310 350 450 450 560 620 770 850 1010 1180 1180 1180 1180 1180	450 500 560 630 780 950 1130 1130 1410 1410 1630 1630 1740 1860 1980 2210 2230	620 700 860 950 1140 1240 1340 1470 2030 2160 2230 2340	6 6 6 6 6 6 6 7 7 7 7 7 7 7 8 8 8 8 8 9 9 100 111 111 112 122 123 13 134 145 16 17 18			

Based on 614 trees cut in Alabama scaled as a rule in 16-foot logs. Height of stump equal diameter breasthigh. By Franklin B. Reed of the U. S. Forest Service. Shortleaf pine, as shown by other work of the Service, follows Longleaf closely.

VOLUME TABLE No. 19. LOBLOLLY PINE, BY THE SCRIBNER RULE

(Ashe in Bulletin No. 24, N. C. Geological and Economic Survey)

Diam- eter		Total Height of Tree — Feet											
breast- high	40	40 50 00 70 80 90 100 110 120 130 140											
Inches		Contents — Board Feet											
8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 29 30 31 32 33 34 35	5 12 18 25 32 40 	13 22 30 40 50 60 70 	211 322 422 544 666 811 97 110 120 	27 42 55 68 83 100 120 140 160 190 220 	52 65 81 199 120 150 170 200 230 330 	93 110 140 180 220 270 310 360 410 460 550 620	130 160 200 230 270 310 360 410 470 660 730 810 890 970 1150	140 170 220 300 350 460 460 520 590 660 740 990 1180 1280 1380 1500 1720 1840	1090 1190 1290 1400 1510 1630 1750 1870 2000 2130	530 610 690 780 870 960 1060 1170 1280 1390 1500 1620 1750 1880 2010	1020 1130 11350 1470 1860 2130 2250 2250 2250 2510	5 6 6 6 7 7 7 7 7 8 8 8 9 9 9 10 10 11 11 11 12 13 13 13 14 14 15 15 15	

Based on measurement of about 3000 trees scaled in 16.3 foot log lengths (with some shorter logs to avoid waste) from a stump 1 or 1.5 foot high to top diameters stated. Allowance made for normal but not excessive crook, and not for defect or breakage. With the same outside dimensions younger trees yield slightly less than old ones: 40 to 45 year old trees yield about 10% less than above figures.

NOTES ON WESTERN VOLUME TABLES

The tables which follow are representative and the most reliable in existence; all are in use in work of importance. No one, however, either East or West, should harbor the idea that such tables will work his salvation.

Few will require caution as to the difference between log scale and saw product. It is well understood that defect has to be specially allowed for. The big part breakage plays in the yield of Coast timber was emphasized in earlier pages.

The fact that trees may have been scaled for a volume table by a scale rule different from the one by which timber in question is actually to be scaled will be considered of consequence only if the two rules vary enough to signify among the inevitable errors of estimating. If that is the case a comparison should be worked out, not a difficult undertaking. Then varying practice in application of the scale rule itself might make noticeable difference. The general conclusion is that, before trusting any volume table on responsible work, the cruiser had better test it to see how it fits his timber and practice.

Further, it is indispensable, when such tables are relied on, that the exact nature of the table itself should be understood and field practice governed accordingly. Three

different kinds of tables are, in fact, represented.

In No. 23, for lodgepole pine, total height of the tree is used as the basis of height classification. Some men will find it strange to work in that dimension; it is habitual with others, however. The general reliability of tables of this kind was discussed on pages 170 and 171, and it is necessary here to add only a suggestion on the head of timber utilization. When the table in question was made up, the logs were scaled to a diameter of 6 inches at the top. If actual utilization in a given locality falls short of that, a very few measurements on down trees will enable a man to make proper deduction. If, for instance, actual utilization of lodgepole pine should fall one log length lower than the standard, a 6-inch 16-foot log,

scaling 18 feet by the Scribner rule, may be deducted from the tabular values. It is not a large percentage of sizable timber. If logs are cut and scaled in longer lengths than 16 feet, adjustment may be made on somewhat the same plan, as explained on pages 172 and 173. This last adjustment may be made in any kind of table.

In most of the western tables total height is neglected and the trees are classified by number of merchantable log lengths. That follows the usual practice in western cruising, practice connected apparently with the great height of the timber. There are, however, two types of tables in this class — those in which the timber is scaled up to a single fixed diameter and those in which the top diameter varies with actual utilization. Nos. 28 and 22, tables for Washington hemlock and for yellow pine of the Southwest, illustrate these two types.

The chances of error in connection with tables of the type of No. 22 (leaving out of account now individual variation of form) may be illustrated as follows: A tree 31 inches in breast diameter with five 16-foot logs is given a volume of 1410 feet and the figure is based (see table 21) on utilization to a 13-inch top limit. If very close utilization should secure another log length above that, the fact would not greatly concern an estimator because it would be so small in volume proportionally. Even if one less log were taken out than the table contemplates, it would amount to but 97 feet, 7 per cent of the tabular volume. What is of more importance, however, is that the height at which the tree reaches 13 inches diameter be estimated correctly. Should this height be set a log length too low and the tree scored down as of four logs instead of five, the value derived from the table would be 1230 feet instead of 1410, 13 per cent too little. An error of equal amount results if the tree is scored a log too long.

Tables of the type of No. 28, scaling the logs up to a small diameter uniform in all sizes of timber, present an appearance of greater accuracy, but as a matter of fact much larger errors than the above may arise from care-

less use of such tables. A chief reason is that men tend strongly to tally timber as yielding the log lengths to which they are accustomed in practice, which in the case of large trees departs widely from the theoretical utilization. Thus, a 36-inch 5-log hemlock is given in table 28 as having 3430 feet of timber. In logging, however, somewhere about 128 feet in log lengths would be got out of it. If, then, a cruiser tallied it as a 4-log tree, his table would give him 2530 feet, over 26 per cent less than the true volume. That might indeed in a given case just about make due breakage and defect allowance, but such a result accidentally arrived at is no justification of the practice.

The user of these tables, then, of whatever description, must realize their exact nature and govern his field work accordingly. Judgment also must supplement their use,

	Diameter I	Breast High		Diam	eter a	t To	p	Contents by
Tree No.	Outside Bark	Inside Bark			of Log 2 Fee			Decimal Rule
	Inches	Inches	1	2	3	4	5	Feet
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16	27 38 53 84 26 39 46 51	23 32 45 74 23 23 24 36 43 48 48 48 30 74 73	19 26 36 62 18 20 20 31 36 41 43 40 27 25 63 54	16 23 32 57 15 18 17 28 31 37 39 37 25 23 60 48	13 20 27 51 11 16 14 24 26 32 32 31 19 46 45	10 15 21 46 i5 8 17 19 24 25 21 12 12 41 40	36 12 10 12 11 11 	1,110 2,590 5,030 19,570 850 1,750 1,290 2,760 4,870 7,040 7,690 6,760 2,790 2,310 17,090 13,280

and some men, having arrived at direct, first-hand grasp of timber quantity, find tables of use only incidentally.

On pages 196 to 197 volume tables produced by scaling logs decreasing by a regular taper, as if trees were conical in form, were referred to as in wide use in Oregon

and Washington. In the application of these to standing timber somewhat the same difficulties are met as above. while others arise due to the fact that only a very unusual tree throughout its merchantable length has a true taper. Normal and also unusual relations in northwestern trees are illustrated above. The inference is easy that tables of the kind mentioned are best left to the use of experts.

The first four of the above sets of figures, for Douglas fir, represent normal form. The body of the tree is seen to have less taper than either the butt log or the top; the larger the tree's diameter the faster the taper normally, and that shows in the butt log particularly. On this last fact rests the practice of cruisers of taking base diameter pretty high usually and frequently discounting the diameter ascertained by measure. Their effort really is to line the basal diameter with that at the top of the first log and those above it.

Trees No. 5 and 6 are representative of quick and slow taper, or what amounts to the same thing, of short and tall timber. On the same base diameter one tree has twice the contents of the other. No. 6 is a tree of very

unusual taper, however.

Other northwestern species, with the exception of cedar, have form in general similar to fir, but a much thinner bark, as Nos. 7 to 10, for hemlock and noble fir. illustrate. Very heavy taper high up in the trees is also shown here. The bearing of this last fact on the applicability of a straight-taper volume table is illustrated below from tree No. 10 in the series. (See also discussion on pages 196 and 197.) The error in one case is 3 per cent, the other 15 per cent. This last error is seen to be incurred by inclusion in the reckoning of a log that contains only 2 per cent of the volume of the tree, and that likely to be broken up in felling. The practice of commercial cruisers in neglecting the contents of trees above a diameter equal about half the base diameter is thus rationalized.

Contents of 4 lower logs, actual taper							6880 feet
Contents of 4 lower logs, regular taper							6660 feet
Contents of 5 logs, actual taper							7040 feet
Contents of 5 logs, regular taper							5960 feet
Contents of fifth log				0			160 feet

The remaining figures illustrate variation of form and irregularity. Nos. 11 and 12, having the same diameter breast high and also at the top of the logs used, are yet 13 per cent apart in contents, while the second pair of matched trees differ by 19 per cent, of the average value in each case. The taper of the body of these trees is regular, however; the variation is in the butt and top log sections, the former being far more significant. Trees Nos. 15 and 16 show some real irregularity, though nothing extreme. Much wider departures from type than any of these could in fact be chosen.

In conclusion, a contrast will be drawn between present commercial methods and the use of volume tables. In the construction of these it is customary to throw out swell butt and other abnormality of form, and, that done, the tables derive strength from the law of averages. Single trees may depart from the type and a certain amount of variation goes with age, but the table, based on a large number of trees and applied to large numbers, if that is done in the same way the measures behind the table were taken, gives results that are trustworthy within reasonable limits. Present-day commercial estimates may be equally correct, but that depends on a different thing—on the ability of the cruiser to size up each tree as seen, on the basis of his training of every description.

VOLUME TABLE No. 20. WESTERN WHITE PINE, IN BOARD FEET, BY THE SCRIBNER RULE

(From Bulletin No. 36, U.S. Forest Service)

high Inches	2	3	4	5	6	7	0			Basis
					ì	1	8	9	10	
			V	olume	— Bo	ard Fe	et			Trees
59 101 112 133 144 155 161 177 188 199 201 212 223 224 225 226 227 228 230 311 323 333 344 355 366	40 45 55 65 75	000 70 85 95 95 1110 125 185 145 185	856 955 1100 125 145 145 165 2215 2255 2275 2255 2275 2295 320 	105 120 140 160 200 230 230 230 320 350 410 480 480 510	165 190 215 240 310 340 420 460 550 650 660 770 760 810	245 280 320 360 400 450 500 500 650 710 760 820 890 1010 1080 1150 11220	360 400 450 510 570 630 690 760 830 910 1140 1220 1300 1470 1550 1710	570 640 720 720 790 870 1050 1050 1140 1240 1330 1730 1830 2030 2030 2140 2250 2360	880 980 1080 1190 1410 1520 2110 2230 2230 2490 2490 22630 2770	77 400 655 76 6104 76 107 86 80 104 111 117 115 103 94 83 81 80 64 65 40 23 28 14 6 6 6 6 6 6 6

From timber grown in northern Idaho.

Trees scaled to a top diameter inside bark of 6 to 8 inches. Height of stump — 2 to 3 feet. All trees scaled as though sound. Loss by breakage was 4 per cent. Loss due to invisible rot was 5 per cent.

VOLUME TABLE No. 21. WESTERN YELLOW PINE IN BOARD FEET, BY THE SCRIBNER RULE

(From Bulletin No. 36, U. S. Forest Service)

Diam- eter breast-			Diam- eter of top in- side	Basis							
high Inches	40	50	60	70	80	90	100	110	120	bark Inches	Trees
12 13 14 16 17 18 19 20 21 22 23 24 25 26 29 30 31 32 33 34 35 36 37 38 39 40	50 60 70 90 110 1130 160 2210 2280 	60 80 90 110 130 160 180 2250 280 330 430 470	70 90 110 130 160 210 250 2280 320 450 610 660	80 100 120 150 210 220 230 370 440 470 520 580 630 760 820 940 1010	140 170 230 230 310 360 410 470 520 590 650 790 860 930 1000 1150 1230 1470	150 180 220 260 350 400 520 590 660 730 800 880 1130 1220 1310 1410 1620 1810 1720 1810	190 230 280 320 380 440 570 640 720 890 980 1170 1270 1380 1490 1610 1870 2120 2250 2390 2530	240 290 350 410 470 780 780 980 1080 1190 1420 2110 2260 2410 2550 2840	310 370 430 580 670 760 850 950 1070 1190 11310 11440 2020 2180 22300 2660 2820 2820 2820 2820 3150	9.6 9.9 10.1 10.4 10.6 11.1 11.3 11.6 11.9 12.1 12.7 12.9 13.2 13.5 14.7 15.2 15.8 16.4	23 48 91 117 142 136 135 104 127 135 85 83 63 63 63 221 22 22 22 22 21 13 64 45 55 11

Measurements by T. S. Woolsey, Jr., in Arizona.

Trees scaled to 8-inch top inside bark - straight and sound. Allow 3 to 15 per cent for defects. The so-called "black jack" variety requires a further reduction of about 12 per cent, having a smaller volume than the older " vellow pine."

VOLUME TABLE No. 22. WESTERN YELLOW PINE, BY THE SCRIBNER RULE

Same trees classified by 16-foot log lengths

Diam- eter			Number	of 16-foot	Logs			
breast- high	1	2	3	4.	5	6	Basis	
Inches			Volume -	- Board	Feet		Trees	
13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 30 31 32 33 34 35 36 37 38 39 40	50 60 70 80 100 120 140 160	80 100 120 140 160 190 220 250 290 330 380 420 470 530 630	140 160 180 210 240 360 410 360 410 460 520 580 640 710 790 1050 1140 1240 1240 1340	190 210 240 270 310 400 450 500 500 780 800 950 1040 1130 1230 1340 1460 1580 1710 1830 1950 2060 2260	380 430 430 490 550 610 680 920 1010 1100 1200 1300 1410 1530 1660 2040 2160 2280 2400 2520	1000 1150 1250 1360 1470 1590 1710 1830 2090 2220 2340 2450 2670	22 47 93 119 142 140 138 108 128 136 101 108 86 95 54 43 25 28 21 14 12 28 21 14 12 28 28 21 21 21 21 21 21 21 21 21 21 21 21 21	
							1844	

The values in this table are materially higher than those of other Forest Service tables for the same species made in California and Oregon.

VOLUME TABLE No. 23. LODGEPOLE PINE, IN BOARD FEET, BY THE SCRIBNER RULE

(From Bulletin No. 36, U.S. Forest Service)

Diam- eter breast-		Total	al Height	of Tree -	— Feet		Basis
high Inches	50	60	70	80	90	100	Trees
10 11 12 13 14 15 16 17 18 19 20	50 60 75 90 105	65 75 90 105 125 140 160	75 90 105 125 145 170 195 225 250 275 300	90 105 125 145 170 200 230 260 290 320 345	105 125 150 180 215 250 285 315 350 380 415	125 155 185 215 220 285 315 350 385 420 460	495 478 296 146 120 113 60 44 25 17

Figures by Tower and Redington from trees cut in Gallatin County, Montana. Trees scaled in logs 10 to 16 feet long up to 6 inches in top.

YIELD OF LODGEPOLE PINE IN RAILROAD TIES (From Study by Students of University of Washington)

Diam-		Average Number Obtained per Tree											
eter breast- high		Hewn Ties	3	Sawed Ties									
Inches	Tall over 80'	Medium 60-80'	Short under 60'	Tall over 80'	Medium 60-80'	Short under 60							
10 11 12 13 14 15 16 17 18 19 20	1.7 3.0 4.0 4.9 5.5 6.0 6.4 6.7 6.9 7.1 7.2	1.5 2.7 3.5 4.0 4.4 4.7 5.0 5.0 5.0	1.1 1.8 2.2 2.5 2.7 2.9	0.9 1.9 3.0 3.9 4.6 5.1 5.5 5.9 6.1 6.3	0.8 1.7 2.6 3.3 3.8 4.2 4.2	0.7 1.2 1.8 2.2 2.5							

Results from 267 trees cut in eastern Oregon: Hewn ties from timber not less than 8½ inches in diameter, made 7 inches thick; sawed ties, 6 by 8 inches; both kinds, 8 feet long. Average height of 10-inch trees, 68 feet; of 15-inch trees, 85 feet; of 20-inch trees, 93 feet.

VOLUME TABLE No. 24. WESTERN LARCH, IN BOARD FEET, BY THE SCRIBNER RULE

(From Bulletin No. 36, U.S. Forest Service)

Diam- eter breast-		Numl	Diam- eter of top inside	Basis				
high	3	4	5	G	7	8	bark Inches	Trees
11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 30 31 32 33 34 35 36 37 38 39 40	95 105 120 135 155 175 195 220 240 265 	140 155 165 185 205 230 260 285 345 345 380 415 455 525 565 605 6	220 240 270 295 325 365 400 435 520 605 655 655 805 805 805 805	380 415 455 490 535 585 635 745 805 805 1060 1130 11205 11280 1140 11525 1600 1685 1770 1830	645 705 775 840 905 975 1055 1130 1210 1295 1465 1560 1745 1945 2040 2145 2240	1005 1085 1180 1275 1375 1470 1565 1670 1770 1875 2085 2190 2295 2395 2395 2505 2610 2715	7.3 7.4 7.5 7.6 7.7 8.0 8.1 8.1 8.2 8.2 8.2 8.2 8.3 8.4 8.5 9.0 9.2 9.4 9.6 9.8 10.0	3 15 31 193 114 119 128 100 93 127 89 89 89 52 32 32 127 210 4 81 15 3 3 2 10 10 11 11 11 11 11 11 11 11 11 11 11

Above table by L. Margolin from timber cut in Flathead County, Montana. Trees scaled without allowance for breakage and defect, which in this timber amounted to 5 per cent. In addition 5 per cent or more should be allowed for "butts" left if logs are driven.

VOLUME TABLE No. 25. ENGELMANN SPRUCE, IN BOARD FEET, BY THE SCRIBNER RULE

(From Bulletin No. 36, U.S. Forest Service)

Diam- eter breast- high			Hei	ght o	f Tre	e — I	eet			Diam- eter of top inside	Basis
Inches	40	50	60	70	80	90	100	1,10	120	bark Inches	Trees
8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26	15 15 20 25 30 40 50 60 70	20 25 30 40 50 60 70 80 95 110 125	30 35 45 55 65 80 95 110 125 140 160 205 230 250	50 60 70 85 100 120 140 160 225 250 280 310 340 370	70 80 90 110 130 150 190 220 250 310 340 370 400 470 500	135 160 185 210 240 270 300 330 360	220 250 280 320 320 400 440 480 520 560 600 650 700	340 380	600 650 700 760 820 880 950	7.4 7.4	8 19 19 35 45 44 51 37 61 57 55 45 43 41 29 21 21 10 11

From trees cut in Colorado and Utah measured by H. D. Foster. Stump height $1\frac{1}{2}$ -3 feet.

VOLUME TABLE No. 26. DOUGLAS FIR OF THE COAST BY THE SCRIBNER DECIMAL RULE (U. S. Forest Service)

Diameter at Stump	Average			1	lum	ber o	of T	hirty	-tw	o-Fo	ot Lo	gs	
Outside Bark	Average	1 1	2	2 1	3	31	4	41	5	51	6	64	7
Inches			V	olur	ne –	- Bo	ard	Fee	t in	Ten	В		
18 20	40 50	28 32	34	41 47	50 56	58 65							
22 24 26 28 30	62 77 91 105 125		44 49 55 61 66	53 60 68 76	66 75 84 95 106	98 110		136					
32 34 36 38 40	145 169 195 228 270			100	115 125 138	149 164	176 192 212	203	247 278				
42 44 46 48 50	312 365 425 480 535						268 286	332	374 403 433		462	592 044	
52 54 56 58 60	588 645 705 765 830		.X					450 480 	530	566 595 630 668 711		680 722 771 821 872	730 774 830 888 942
62 64 66 68 70	900 972 1048 1133 1226									760 808 864	953	926 985 1066 1147 1225	1009 1082 1171 1261 1345
72 74 76	1310 1413 1515										1285	1312 1390 1465	1420 1486 1556

Based on 1394 trees measured in logging operations in Lane County, Oregon. Diameters, taken outside bark, on the stump, which was ordinarily about 4 feet high, are closely comparable with the diameter at breast height. Trees scaled without deduction for defect or breakage, to a point 10 inches in diameter at the top, unless unmerchantable to this point. The majority of the logs were 24 feet long, though the length varied from 16 to 36 feet.

VOLUME TABLE No. 27. DOUGLAS FIR OF THE INTERIOR IN BOARD FEET, BY THE SCRIBNER RULE

(From Bulletin No. 36, U.S. Forest Service)

Diam- eter breast-		Total I	Height (of Tree	— Feet		Diam- eter of top inside	Basis	
Inches	60	70	80	90	100	110	bark Inches	Trees	
8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29	20 30 40 60 70 90 120 140 150 170 230 250 	30 40 60 70 90 110 130 150 170 220 240 270 330 360 390 420 450 520	60 70 90 110 130 150 220 230 280 320 350 380 420 450 450 620 670	110 130 160 180 210 240 270 300 330 370 410 450 630 680 730 790	190 220 250 290 320 360 400 480 530 690 630 690 870 940	400 450 500 500 600 650 710 770 830 900 970	6.2 6.3 6.5 6.6 6.7 6.9 7.0 7.2 7.3 7.4 7.5 8.0 8.2 8.3 8.5 8.8 8.9	1 7 4 23 53 57 51 55 59 51 64 57 55 57 55 50 45 40 38 31 22 12 9	

From timber cut in Wyoming and Idaho measured by Messr. Redington and Peters.

VOLUME TABLE No. 28. WASHINGTON HEMLOCK BY THE SCRIBNER DECIMAL RULE

(By E. J. Hanzlik of U. S. Forest Service)

Diameter Breast,	Average			Numb	er of T	hirty-	two-Fo	ot Log	ÇB	
High Outside Bark	Average	11	2	21	3	31	4	44	5	51
Inches			Vol	lume –	- Boar	d Feet	in Te	ns		
12 13 14 15	14 20 26 32	16 17 18 19	21 23 26 29	28 31 35	32 37 42	44 49				
16 17 18 19 20	39 46 53 62 70	21 23 26	32 35 39 42 46	39 43 47 52 57	47 52 58 64 71	55 61 68 76 84	78 87 96			
21 22 23 24 25	80 90 100 111 122		50 54 57	62 67 73 80 86	77 84 90 96 104	91 100 108 116 124	104 112 122 130 139	140 148 156 165		
26 27 28 29 30	134 146 158 170 183			92 100 106 113 121	112 120 128 139 147	133 141 149 158 168	148 158 167 177 186	174 184 193 204 214	226 237 248	
31 32 33 34 35	197 212 228 245 264				156 165 173 181 190	177 186 195 204 213	197 208 219 229 242	226 238 250 263 278	260 274 288 305 323	353 376
36 37 38 39 40	284 304 326 346 368					222 231 240 250 259	253 266 280 294 308	293 310 330 351 378	343 366 393 424 460	404 436 477 519 561

Based on 1440 trees, in both pure and mixed stands, measured at logging operations at various points in western Washington. A stump height equal breast diameter allowed. Trees scaled in 16-foot log lengths (with trimming allowance) to a diameter inside bark of 8 inches. No deduction for defect or breakage.

Actual utilization a little over 80 per cent of above figures.

The true firs are formed very nearly like hemlock.

VOLUME TABLES No. 29. WASHINGTON RED CEDAR BY THE SCRIBNER DECIMAL RULE

TALL TIMBER

Diameter Breast	Fir	st 32'	Log	Seco	ond 32'	Log	Log m.	Log n.	Total
High Outside Bark	Top Diam.	Scale	% of Total	Top Diam.	Scale	% of Total	Third Lo	Fourth L Diam.	Feet
16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50	28 29 30	140 160 190 230 320 370 430 480 610 670 750 810 920 1100 1120 11210 11210	70 70 61 62 67 59 55 53 56 51 50 48 47 49 46 44 42 42	7 8 10 11 12 13 14 15 16 17 18 19 20 21 23 24 25 26	60 70 120 140 160 190 230 320 370 430 480 610 750 810 920 1000	30 30 39 38 33 30 31 29 28 28 29 27 29 29	11(1) 10 11 12 13 14 15 16 17 18 19 20 21	11(1/2) 12(1/2) 10 11 11 11 12 13 14 15	200 230 310 370 480 630 780 900 1300 1490 1490 1940 2220 2500 2700 3000 3300

The above and following table are based on field measurements of about 1200 sound and normal trees grown in fully stocked mixed stands in the Puget Sound region, at elevations from 200 to 1000 feet, by A. G. Jackson of the U.S. Forest Service. Scaled from taper measurements in 32-foot logs to diameters stated. Data arranged to promote timber grading.

Cedar scaled in short lengths, if at the same time it is sound, of good form, and fully utilized, will yield more than these values. On the other hand the tree is so largely subject to swell butt, rot and breakage, that tables must be used with great caution and often discarded altogether.

CITO	DTPD	TIMBER	
SHU	B. I E. R.	I LIVED F. IS.	

Diam- eter Breast	Fir	st 32'	Log	Seco	nd 32'	Log	Log m.	Log m.	Total Volume
High Outside Bark	Top Diam.	Scale	% of Total	Top Diam.	Scale	% of Total	Third Lo	Fourth I Diam.	Feet
16 18 20 22 24 26 28 30 32 34 36 28 40 42 44 46 48 50	10 11 12 13 14 15 17 18 19 20 22 23 24 25 27 28 29 30	120 140 160 190 210 280 370 430 480 560 670 750 810 920 1100 1160 1220 1310	70 70 70 88 69 67 70 63 61 58 57 55 55 54 48 47 45	6 7 8 9 10 11 12 13 14 15 17 18 19 20 21 23 24 25	50 60 70 90 120 140 160 190 230 280 370 430 480 560 610 750 810 920	30 30 30 32 31 33 30 28 29 M2 31 33 32 31 32 31 32 31 32 31 33 32 31 31 31 31 31 31 31 31 31 31 31 31 31	10(1) 12(1) 10 11 12 13 15 16 17 18	 	170 200 230 250 330 420 530 650 790 960 1180 1340 1480 2110 2420 2620 2900

The trees in this table are really of good length. Measurements on short mountain timber are not available.

Cedar Shingle Bolts. Very defective trees, the breakage of logging operations, and sometimes the whole usable contents of trees above about 20 inches in breast diameter are largely utilized in this form. The bolts are cut 52 inches long and the larger pieces split; they are then piled and measured in the cord 8×4 feet. In present practice from 18 to 25 bolts make a cord which careful measurement has shown to contain of solid wood about 70 per cent of its outside contents. A cord is equivalent to from 500 to 700 feet log scale, less in the smaller sizes of timber.

VOLUME TABLE No. 30. SUGAR PINE IN CALIFORNIA BY THE SCRIBNER DECIMAL RULE

(U. S. Forest Service)

Inches	9 10	3	4	5 Volu	6 me -	7	8	9	10	11	12	Diameter Inside Bark of Top	Basis
12		15	,	Volu	me -	D.		1	1			T	ME
		15				- Do	ard I	eet in	n Ten	8		Inches	
			22									8	
	10	17 19	24 27	39								8	1
	13	20	30	43	::	111		1			1	9	2
20	17	25	37	50	65	79				1		9	28
22		31	43	57	74	89						9	23
880		40 50	53 64	67	83 96	100	122					9	35
200		63	78	92	110	128	136 152					10	35
00		80	94	108		144	170	189				10	53
32			113		145	163	192	218				10	50
		٠.		149		187	217	247				10	38
70.00	• •	• •	$\frac{160}{183}$		$\frac{191}{220}$	213 245	246 278	279 313	310			11	36
40		::	210	229	253	280	313	349	386			11	40
40			240	261	288	319	354	390	427	463		îî	43
			271	295	325	359	398	435	473	515		12	39
40			303	330		401	445	482	523	567		12	31
EO	• •	• •	337	366 401	405	446 493	493 544	532 586	575 630	623 681	749	12 12	43
20.00			::	438	489	544	598	642	686	740	818	12	56
54				472	532	597	653	698	742	801	885	13	36
					575	652	711	756	800	862	953	13	25
		٠.			619	709	769	814	860	923		13	25
00					704	764 820	829 886	872 930	921 983	987 1051	1090 1159	14	28 25
O.A.						876	943	990		1116	1227	14	27
66 .						933		1053	1109	1181	1297	14	11
						989	1058	1115		1250	1000	15	9
70	• •					1048	$\frac{1117}{1176}$	$\frac{1177}{1240}$	$\frac{1239}{1305}$	1319 1388	1434	15 15	17
74							1235	1303				16	6
70							1296	1368				16	6
78							1358		1500			16	4
80							1420	1497	1565	1659	1778	16	8

Average stump heights 1.3 to 3.1 feet. Logs scaled in commercial lengths as cut.

SECTION III

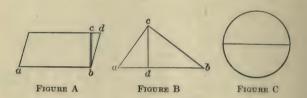
MISCELLANEOUS TABLES AND INFORMATION

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RULES FOR AREA AND VOLUME OF DIFFERENT FIGURES

Area of Square. Multiply the length of side by itself, or, as is said, "square" it.

Area of Rectangle. Multiply the base by the altitude.



Area of Parallelogram. (Figure A.) Multiply base $a \ b$ by altitude $b \ c$, not by $b \ d$. If $b \ d$ and the angle at d are known, $b \ c$ may be found by the formula

 $bc = bd \times \text{sine of angle at } d.$

Area of Triangle. (Figure B.) Multiply base a b by

altitude c d and divide by 2.

Area of Triangle with 3 Sides Given. (Figure B.) Add the 3 sides together and divide the sum by 2. From this half sum take each side in succession. Multiply the half sum and the remainders all together and take the square root. The formula is

$$\sqrt{\frac{1}{2}} s (\frac{1}{2} s - a) (\frac{1}{2} s - b) (\frac{1}{2} s - c)$$

Circle. Circumference equals diameter × 3.1416.

Area of Circle. (Figure C.) Square the diameter, multiply by 3.1416, and divide by 4.

Right-Angled Triangle. The square of the hypothenuse of a right-angled triangle equals the sum of the squares on the other two sides, or, in the figure,

$$AB^2 + AC^2 = BC^2,$$

$$O + N = M$$

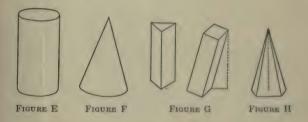
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By means of this rule, when any two sides of a right-angled triangle are given, the third can be found



Volume of Cylinder. (Figure E.) Multiply the area of the base by the altitude.

Volume of Cone. (Figure F.) Multiply the area of the base by one-third of the height.



Volume of Prism whether Right or Oblique. (Figure G.) Multiply area of base by the vertical height.

Volume of Pyramid. (Figure H.) Multiply base by

one-third of the height.

To Measure the Contents of a Box or Solid with Sides at Right Angles to One Another. Multiply length by breadth by height. If the dimensions are in feet the result will be the contents in cubic feet.

WEIGHT OF MATERIALS

A	cubic foot	of	water weighs									62½ lbs.
A	cubic foot	of	cast iron weighs	abou	ıt							
A	cubic foot	of	wrought iron or	steel	we	ighs	ab	out			٠	480 lbs.

Woods when thoroughly seasoned weigh per cubic foot about as follows. Absolute drying in a kiln will lessen these figures about 10 per cent. Green wood is from 50 to 80 per cent heavier.

White pine, white spruc	e,	bal	sam	fir	as	pen			 	27	lbs.
Red spruce, bemlock, po	go	ar							 	30	lbs.
Pitch pine, Norway pine										1 - 35	lbs.
White birch, red maple,											
red oak											
Beech, sugar maple	**							 ,0	abou	at 48	lbs.
White oak, black birch									a.bo1	at 52	lbs.

A cord of green spruce pulp wood weighs about 4500 lbs.; fir and white pine a little more. A cord of dry spruce pulp wood weighs 3000 to 3500 lbs. Pine, fir, and poplar are somewhat lighter if in exactly the same moisture condition.

Green hard wood by the cord varies greatly in weight. A cord of white birch spool-wood weighs 6000 to 7000 lbs.; sugar maple and yellow birch are 10 per cent heavier; soft maple, ash, basswood, and poplar are somewhat lighter than white birch. For green split cord wood 4000 to 6000 lbs. are the usual limits of weight. Medium dry birch, beech, and maple, split, 66 per cent solid in the pile, weighs about 3000 lbs. to the cord.

A thousand feet of old growth spruce logs, Androscoggin scale, weighs about 6000 lbs., and this is probably the lower limit for green soft-wood lumber, while southern yellow pine at 8000 to 10,000 lbs. is the limit in the other direction. Between these limits there is wide variation by reason of scale and quality.

Seasoning decreases the weight of timber by 30 to 50 per cent as a rule, and at the same time increases its strength by 50 to 100 per cent.

HANDY EQUIVALENTS

There are 160 square rods in an acre.

A square acre is 208.71 feet on a side.

118 feet is approximately the radius of a circular acre. 83 feet of a half acre, and 59 feet of a quarter acre.

There are 5280 feet in a mile

A meter contains 39.37 inches: a kilometer is .62 mile.

A liter contains 61 cubic inches, - nearly the contents of a quart.

A hectare contains 2.47 acres.

A gram weighs 15.432 grains, Troy weight.

A kilogram or kilo contains 2.2 lbs avoirdupois.

There are 231 cubic inches in a U. S. liquid gallon.

There are 2150.42 cubic inches in a U. S. struck bushel. A horsepower is the work done in lifting 33,000 pounds 1 foot in 1 minute. A flow of 528 cubic feet of water per

minute with 1 foot fall generates one horsepower.

A miner's inch is the flow of water through an orifice 1 inch square under a head (in some States) of 6 inches. In California 50 miner's inches equal 1 cubic foot per second, equal 1.9835 acre feet per day, nearly an inch an hour. In some States 40 miner's inches equal this flow.

NO. OF PLANTS PER ACRE WITH DIFFERENT SPACING

Spacing	No.	
3 × 3 ft.	4840	
4×4	2720	
5 × 5	1740	
6 × 6	1210	
7 × 7	890	
8 × 8	680	
9 × 9	538	
10 × 10	436	

COMPOUND INTEREST TABLE

Amount of \$1 principal after any number of years and at given rates percent

Yrs.	2%	21/2%	3%	31/2%	4%	41/2%	5%	51/2%	6%
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19	1.020 1.040 1.061 1.082 1.104 1.126 1.149 1.172 1.243 1.264 1.320 1.320 1.340 1.373 1.400	1.025 1.025 1.077 1.104 1.131 1.160 1.218 1.218 1.249 1.312 1.312 1.342 1.379 1.413 1.448 1.485 1.522 1.590	1.030 1.061 1.093 1.126 1.159 1.194 1.230 1.267 1.304 1.344 1.384 1.469 1.513 1.553 1.605 1.653 1.754	1.035 1.071 1.109 1.148 1.188 1.229 1.317 1.363 1.411 1.460 1.514 1.619 1.675 1.734 1.795 1.858 1.928	1.040 1.082 1.125 1.170 1.217 1.265 1.316 1.423 1.480 1.540 1.601 1.665 1.732 1.801 1.873 1.948 2.107	1.045 1.092 1.141 1.193 1.246 1.302 1.361 1.422 1.486 1.553 1.623 1.696 1.772 1.852 2.022 2.113 2.209 2.308	1.050 1.103 1.158 1.216 1.216 1.340 1.407 1.478 1.551 1.629 1.710 1.796 1.886 1.980 2.079 2.183 2.292 2.407 2.527	1.055 1.113 1.174 1.239 1.307 1.379 1.455 1.535 1.619 1.708 1.802 1.901 2.006 2.116 2.233 2.355 2.485 2.622 2.766	1.060 1.124 1.191 1.262 1.338 1.419 1.504 1.594 1.690 1.791 1.898 2.012 2.213 2.261 2.397 2.540 2.693 2.854 3.026
20 25 30 35 40 45	1.486 1.641 1.811 2.000 2.208 2.438	1.639 1.854 2.098 2.373 2.685 3.038	1.806 2.094 2.427 2.814 3.262 3.782	1.990 2.363 2.807 3.334 3.959 4.702	2.191 2.666 3.243 3.946 4.801 5.841	2.412 3.005 3.745 4.667 5.816 7.248	2.653 3.386 4.322 5.516 7.040 8.985	2.918 3.813 4.984 6.514 8.513 11.127	3.207 4.292 5.744 7.686 10.286 13.765
50	2.692	3.437	4.384	5.585	7.107	9.033	11.467	14.542	18.420

TIME IN WHICH A SUM WILL DOUBLE

Rate Per cent	Simple Interest	Compound Interest
2 2 3 3 3 4 4 4 5 5 5 6	50 years 40 years 33 years 4 months 28 years 7 months 25 years 22 years 2½ months 20 years 18 years 7 months 16 years 8 months	35 years 28 years 1 month 23 years 5½ months 20 years 2½ months 17 years 8 months 15 years 0 months 14 years 1½ months 12 years 11½ months 11 years 11½ months

Note in above tables that a sum at compound interest doubles when rate of interest × number of years equals (very nearly) 71. With this remembered many problems in compound interest can be solved mentally.

TABLE OF WAGES, AT GIVEN RATES PER MONTH OF TWENTY-SIX DAYS

			1		DAIS		
D	\$15	\$16	\$17	\$18	\$19	820	\$21
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 22 24 25 26	0.58 1.15 1.73 2.31 2.83 3.46 4.04 4.62 5.19 5.75 6.92 7.50 8.08 8.65 9.23 9.81 10.38 10.96 11.54 12.12 12.69 13.27 13.85 14.42	0.62 1.23 1.85 2.46 3.08 4.31 4.92 5.54 6.15 7.7 7.38 8.00 8.62 9.23 10.46 11.69 12.31 12.92 13.54 14.17 15.38 16.00	0.66 1.31 1.96 2.62 3.27 4.58 5.23 5.88 6.54 7.19 7.85 8.50 9.15 9.81 11.12 11.77 12.42 13.08 13.73 14.38 15.04 15.69 16.35 17.00	0.69 1.38 2.08 2.77 3.46 4.15 5.54 4.85 5.54 6.23 6.92 7.62 8.31 9.60 9.60 10.38 11.77 12.46 13.15 13.85 14.54 15.22 16.62 17.31 18.00	0.73 1.46 2.19 2.92 3.65 4.38 5.12 5.85 6.58 7.31 8.77 9.50 10.23 10.96 11.69 12.42 13.15 13.15 14.62 15.35 16.08 17.54 18.27 19.00	0.77 1.54 2.31 3.08 3.85 4.62 5.38 6.16 6.92 7.69 9.23 10.00 10.77 11.54 12.31 13.08 14.62 15.38 6.16 6.16 19.23 20.00	0.81 1.62 2.42 3.23 4.04 4.85 5.65 6.46 7.27 8.08 9.69 10.50 11.31 12.12 13.73 14.54 16.96 17.77 17.77 19.38 20.19 21.00
D	\$22	\$23	\$24	\$25	\$26	\$27	\$28
1 2 3 4 4 5 6 6 7 8 9 10 111 112 13 144 15 16 117 18 19 20 21 22 23 24 25 25 26	0.85 1.70 2.54 3.38 4.23 5.08 4.23 6.76 7.61 8.46 9.30 10.15 11.00 11.84 14.38 15.23 16.07 16.92 17.76 18.61 19.46 20.30 21.15 22.00	0.88 1.77 2.65 3.53 4.42 5.30 6.19 7.96 8.85 9.93 10.62 11.50 12.38 13.27 14.15 10.76 10.7	0.92 1.85 2.77 3.69 4.62 5.54 6.46 7.38 8.31 9.23 10.15 11.08 12.00 12.92 13.85 14.77 16.62 17.54 19.23 10.15 12.00 12.0	0.96 1.92 2.89 3.84 4.81 5.77 6.73 7.69 8.65 9.61 10.57 11.54 12.50 13.46 14.42 15.38 16.34 17.31 18.27 19.23 20.19 21.15 22.11 23.08 24.04 25.00	1.00 2.00 3.00 4.00 5.00 6.00 9.00 10.00 11.00 12.00 13.00 14.00 14.00 15.00 16.00 19.00 20.00 21.00 22.00 22.00 23.00 24.00 25.00 26.00	1.04 2.07 3.11 4.15 5.19 6.23 7.27 8.30 9.34 10.38 11.42 12.46 13.50 14.54 15.58 16.61 17.65 18.68 19.72 20.76 21.80 22.84 23.88 24.91 25.95 27.00	1.08 2.15 3.23 4.31 5.38 6.46 7.54 8.62 9.69 10.77 11.89 14.00 15.08 16.15 17.23 19.38 20.46 21.54 22.61 23.69 24.75 25.89 22.81 20.92 24.70 25.89 26.92 28.00

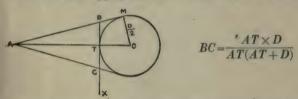
TABLE OF WAGES AT GIVEN RATES PER MONTH OF TWENTY-SIX DAYS—continued

D	\$29	\$30	\$31	\$32	\$35	\$40	\$45
1 2 3 4 4 5 6 6 7 8 8 9 10 11 12 13 14 15 16 17 18 19 20 20 22 23 24 25 26	1.12 2.23 3.34.46 5.58 6.69 7.80 8.92 10.04 11.15 12.27 13.38 14.50 15.61 16.73 17.84 20.07 21.19 22.30 22.4.53 25.65 26.76 27.88 29.00	1.15 2.30 3.46 4.62 5.77 6.92 8.08 9.23 10.38 11.54 12.69 13.85 15.00 16.15 17.31 18.46 19.62 20.77 21.92 23.08 24.23 25.38 24.23 25.38 26.54 27.69 28.85 30.00	1.19 1.19 2.38 3.38 4.77 1.19 2.19 2.19 2.19 2.19 2.19 2.19 2.19	9 1.23 8 2.469 7 4.92 5 7.38 8 9.85 6 8 11.08 8 11.08 1 12.31 1 12.	1.35 2.69 4.04 5.38 6.73 8.07 9.42 10.77 12.11 13.46 14.81 16.15 17.50 19.21 22.88 24.23 25.57 26.92 28.27 29.61 30.96 31.33 33.65 35.00	1.54 3.08 4.62 6.15 7.69 9.23 10.77 12.31 13.84 15.38 16.92 12.54 62 10.00 21.54 24.61 29.23 30.77 24.61 29.23 30.77 32.31 33.84 35.38 46.92 29.23 30.77 32.84 46.92 30.84 46.94 46.	1.73 3.46 5.19 6.02 8.65 10.39 12.12 13.85 17.31 19.04 20.77 22.596 24.23 25.96 27.70 29.43 31.16 33.89 34.62 36.35 38.08 39.81 41.54 43.27 45.00
D	\$50	\$60	\$70	\$ 75	\$80	\$90	\$100
1 2 3 4 5 6 7 8 9 10 11 11 12 13 14 15 16 17 18 19 20 20 21 22 23 24 25 26 26 26 26 27 28 28 28 28 28 28 28 28 28 28 28 28 28	1.92 3.85 5.77 7.69 9.61 11.54 13.46 15.38 17.31 19.23 21.15 23.08 25.00 26.92 28.85 30.77 32.69 34.61 38.46 40.38 42.31 44.23 44.08 50.00	2.31 4.62 6.92 9.23 11.54 13.85 16.15 18.46 20.77 23.08 25.38 27.69 30.00 32.31 34.61 36.92 39.23 41.54 446.15 48.46 77 53.08 55.38 57.69 60.00	2.69 5.38 8.08 10.77 13.46 16.15 18.84 21.54 24.23 26.92 29.61 32.31 35.00 40.38 43.08 45.77 48.46 51.15 55.28 61.92 64.62 67.31 70.00	2.88 5.77 8.65 11.54 14.42 17.11 19.19 23.08 25.96 28.85 31.73 40.38 40.38 43.27 46.15 54.81 57.69 60.58 66.35 69.23 72.12 75.00	3.08 6.15 9.23 12.31 15.38 18.46 24.62 27.69 30.77 33.89 40.00 43.08 46.15 49.23 55.38 61.54 64.61 67.69 70.77 73.85 80.00	3.46 6.92 10.38 13.85 17.31 20.77 24.23 27.69 31.16 34.62 38.08 41.54 45.00 48.46 51.92 55.85 62.31 65.77 69.23 72.69 79.61 83.08 86.54 90.00	3.85 7.69 11.54 15.38 19.23 23.08 26.09 30.77 34.61 38.46 42.21 46.15 50.00 61.54 65.38 69.23 76.92 80.77 84.61 88.46 92.31 96.15 100.00

THE BILTMORE STICK

This implement, employed to ascertain the diameter of standing timber when held at arm's length tangent to the trees to be measured, was briefly described on page 163. Relations between tree, stick, and eye when the stick is in use are made clear in the figure, the circle representing a section of a tree breast high, B X the Biltmore stick, A T the distance from the stick to the eye, and O M a radius vertical to the line of sight passing on one side of the tree. With this for a pattern it is clear how the woodsman, after having determined A T as a matter of practice, can plot circles of different diameters, draw tangents to them from A, and ascertain by measurement in each case B C, the proper stick graduation.

The geometry of the matter is that of similar rightangled triangles, and consideration will show the soundness of the formula appended, from which may be derived



the value of B C for circles of any size and for any arm reach. When the latter, A T, has been determined by trial, the formula becomes simpler. Thus with A T = 25 inches

$$B~C = \frac{25~D}{\sqrt{25~(25~+D)}}$$
 or, for $D=10$ inches
$$\frac{250}{\sqrt{625~+250}} = \frac{250}{29.58} = 8.45$$
 inches.

Values of B C for tree diameters from 6 to 60 inches and distances of 23 to 27 inches have been worked out and are published in the "Proceedings of the Society of American Foresters" for 1914, page 48.

The Forest Service has employed the Biltmore stick in measuring large timber on the Pacific Coast and elsewhere, and the tests applied have shown reasonable accuracy. A careful analysis of sources of error ¹ has developed the following:

(a) Tilting the stick and holding it other than vertical to the line of sight to the trees' center are practices to be guarded against, but if reasonable care is used in manipula-

tion, errors are negligible.

(b) In applying values derived from plots or tables to the stick itself, regard must be had to its thickness. The stick may well be beveled, or a steel spline may be inserted into it to carry the graduations.

(c) Errors arising from measuring a tree the narrow or the wide way are greater than with the caliper; hence

cross measures are the more desirable.

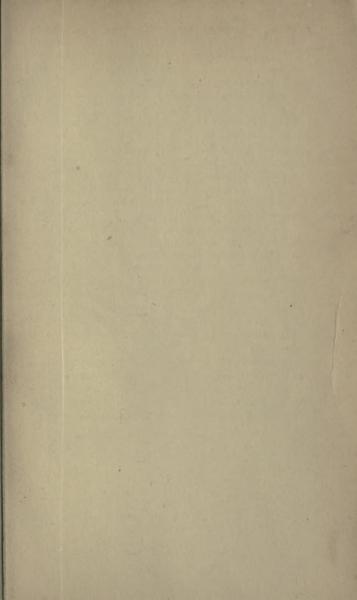
(d) It is very easy in practice to vary the distance between the stick and the eye, and this introduces error that is material, though in continued work successive errors tend to balance.

(e) Men of ordinary height have a constant tendency to measure tree diameter not breast high, but higher, near

the eye level.

To conclude, the Biltmore stick requires to be practically tested before use and constant care in application. More liable to error than the caliper, in ordinary timber it works less rapidly as well. While serviceable in its field, its general use is not to be recommended.

¹ Bruce at previous reference.



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